

TRAFFIC IMPACT STUDY

For

WPT Industrial REIT Proposed Warehouse

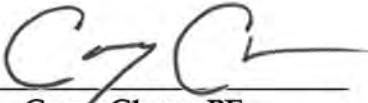
Property Located at:

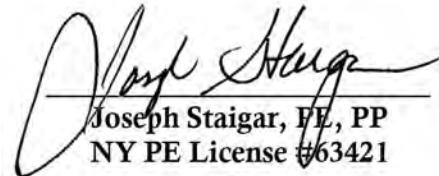
13 Mountainview Avenue
Section 74.07 – Block 1 – Lots 33 & 36
Town of Orangetown, Rockland County, NY

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INTRODUCTION

It is proposed to construct a warehouse on a parcel of land currently developed with a church, located along the westbound side of Mountainview Avenue to the west of its intersection with Route 303 in the Town of Orangetown, Rockland County, New York, see Figure 1 in Appendix A. The site is designated as Section 74.07, Block 1, Lots 33 & 36. The existing use consists of an approximately 106,000 SF vacated church. It is proposed to raze the existing site and construct a 175,760 SF warehouse building (The Project). Access to the site is currently provided via one (1) full movement driveway along Mountainview Avenue. It is proposed to construct a right turn in/right turn out driveway along Route 303 and to reconstruct a full movement driveway along Mountainview Avenue. Parking will be provided via one hundred fifty (150) on-site parking spaces (plus thirty (30) land banked parking spaces) as well as thirty (30) trailer storage spaces.

Dynamic Traffic, LLC has been retained to prepare this study to assess the traffic impact associated with the construction of The Project on the adjacent roadway network. This study documents the methodology, analyses, findings and conclusions of our study and includes:

- A detailed field inspection was conducted to obtain an inventory of existing roadway geometry, traffic control, and location and geometry of existing driveways and intersections.
- Existing traffic data was collected by Creighton Manning via manual turning movement (MTM) counts during the weekday AM and weekday PM peak periods at the following intersections:
 - Route 303 & Mountainview Avenue
 - Route 303 & Orangeburg Road (CR 20)/Chase Bank Driveway
 - Route 303 & Route 340/South Greenbush Road
 - Route 303 & Glenshaw Street
- Projections of traffic to be generated by the proposed development were prepared utilizing trip generation data as published by the Institute of Transportation Engineers. Site traffic was then assigned to the adjacent street system based upon the anticipated directional distribution.
- Capacity analyses were conducted for the Existing, No Build, Build, Design Horizon Year No Build, and Design Horizon Year Build conditions for the study intersections.
- The proposed point of ingress and egress was inspected for adequacy of geometric design, spacing and/or alignment to streets and driveways on the opposite side of the street, relationship to other driveways adjacent to the development, and conformance with accepted design standards.
- The site plan as designed was reviewed for sufficiency in accommodating large wheel base vehicles such as delivery trucks, refuse trucks, and emergency vehicles.
- The parking layout and supply was assessed based on accepted design standards, local requirements, and demand experienced at similar developments.

EXISTING CONDITIONS

A review of the existing roadway conditions near the proposed site was conducted to provide the basis for assessing the traffic impact of the development. This included field investigations of the surrounding roadways and intersections, collection of traffic volume data, and extensive analyses.

Existing Roadway Conditions

The following are descriptions of the roadways in the study area:

Route 303 is a Principal Arterial roadway under the jurisdiction of the New York State Department of Transportation (NYSDOT). In the vicinity of the site the posted speed limit is 40 MPH and the roadway provides two travel lanes in each direction with a general north/south orientation. On-street parking is not permitted along either side of the roadway while curb is provided along portions of the west side of the roadway and sidewalk is not provided along either side of the roadway. Route 303 provides a straight horizontal alignment and a relatively flat vertical alignment. The land uses along Route 303 in the vicinity of The Project are primarily commercial.

Route 340 is a Minor Arterial roadway under the jurisdiction of the NYSDOT. In the vicinity of the site the posted speed limit is 40 MPH and the roadway provides one travel lane in each direction with a general east/west orientation. On-street parking is not permitted along either of the roadway, while curb and sidewalk are provided along the southern side of the roadway. Route 340 provides a curved horizontal alignment approximately 300' east of its intersection with Route 303 and a relatively flat vertical alignment. The land uses along Route 340 in the vicinity of The Project are primarily residential.

Orangeburg Road (CR 20) is a Minor Arterial roadway under the jurisdiction of Rockland County. In the vicinity of the site the posted speed limit is 30 MPH and the roadway provides two travel lanes in each direction with a general east/west orientation. On-street parking is not permitted along either side of the roadway. Curb is provided along both sides of the roadway, while sidewalk is provided along the southern side of the roadway. Orangeburg Road provides a straight horizontal alignment and a downgrade from west to east towards the intersection with Route 303. The land uses along Orangeburg Road in the vicinity of The Project are primarily institutional (church and college).

Mountainview Avenue is a local roadway under the jurisdiction of the Town of Orangetown. In the vicinity of the site the posted speed limit is 30 MPH and the roadway provides one travel lane in each direction with a general east/west orientation. On-street parking is not permitted along either side of the roadway while curb and sidewalk are provided along the north side of the roadway and portions of the south side of the roadway. Mountainview Avenue provides a curved horizontal alignment and a hilly vertical alignment. The land uses along Mountainview Avenue in the vicinity of The Project are a mixture of commercial and residential.

South Greenbush Road is a local roadway under the jurisdiction of the NYSDOT immediately adjacent to the intersection with Route 303. In the vicinity of the site the posted speed limit is 30 MPH and the roadway provides one travel lane in each direction with a general east/west orientation. It should be noted that South Greenbush Road provides a 90° horizontal curve to the south approximately 220' west of its intersection with Route 303 to continue as a generally north/south roadway. On-street parking is not permitted along either side of the roadway. Curb is provided along both sides of the roadway, while sidewalk is provided along the southern side of the roadway. South

Greenbush Road provides a curved horizontal alignment and a relatively flat vertical alignment. The land uses along South Greenbush Road in the vicinity of The Project are a mixture of commercial, institutional (library), and residential.

Glenshaw Street is a local roadway under the jurisdiction of the Town of Orangetown. In the vicinity of the site the posted speed limit is 30 MPH and the roadway provides one travel lane in each direction with a general east/west orientation. On-street parking is not permitted along either side of the roadway. Curb is provided along both sides of the roadway, while sidewalk is not provided along either side. Glenshaw Street provides a straight horizontal alignment and a relatively flat vertical alignment. The land uses along Glenshaw Street in the vicinity of The Project are primarily industrial.

Existing Traffic Volumes

Manual turning movement (MTM) counts were conducted by Creighton Manning on Thursday, June 2, 2022 from 7:00 to 9:00 AM and from 4:00 to 6:00 PM at the following intersections:

- Route 303 & Mountainview Avenue
- Route 303 & Orangeburg Road (CR 20)/Chase Bank Driveway
- Route 303 & Route 340/South Greenbush Road
- Route 303 & Glenshaw Street

Review of the collected traffic data reveals that the weekday morning peak street hour (PSH) occurs between 7:45 - 8:45 AM and the weekday evening PSH occurs between 4:15 - 5:15 PM. It should be noted that traffic impacts associated with the COVID-19 pandemic were in effect as of the time of the traffic counts. As a result, current traffic volumes on the surrounding roadways are atypically low at this time and may not be representative of “typical” traffic conditions. Consistent with data published by Creighton Manning within the *Traffic Impact Study for Proposed WPT Industrial REIT Warehouse Development*, dated September 1, 2022, an adjustment factor was applied to the weekday evening peak hour volumes to account for this effect. The current weekday morning peak hour volumes were found to be higher than the historical traffic volumes conducted along Route 303 and as such, no adjustment factor was applied. Figure 2, located in Appendix A, shows the existing peak hour traffic volumes at the study intersections. All traffic counts are contained in Appendix B.

Existing Capacity Analysis

The methodology utilized in the capacity analyses is described in the *Highway Capacity Manual*, published by the Transportation Research Board. In general, the term Level of Service (LOS) is used to provide a “qualitative” evaluation of capacity based upon certain “quantitative” calculations related to empirical values, such as traffic volume and intersection control.

At signalized intersections, factors that affect the various approach capacities include width of approach, number of lanes, signal “green time”, turning percentages, truck volumes, etc. However, delays cannot be related to capacity in a simple one-to-one fashion. For example, it is possible to have delays in the Level of Service “F” range without exceeding roadway capacity. Substantial delays can exist without exceeding capacity if one or more of the following conditions exist: long signal cycle lengths; a particular traffic movement experiences a long red time; or progressive movement for a particular lane group is poor. Table I describes the level of service ranges for signalized intersections.

An unsignalized (STOP sign controlled) driveway or side street along a through route is seldom critical from an overall capacity standpoint, however, it may be of great significance to the capacity of the minor cross-route, and it may influence the quality of traffic flow on both. When analyzing an unsignalized intersection, it is assumed that both the major street through and right turn movements are unimpeded and have the right-of-way over all side street traffic and left turns from the major street. All other turning movements in the intersection cross, merge with, or are otherwise impeded by major street movements. Traffic delays at unsignalized intersections are determined by sequentially processing these impeded movements. Table II describes the level of service ranges for unsignalized (stop controlled) intersections.

Table I
**Level of Service Criteria
for Signalized Intersections**

Level of Service	Average Control Delay (seconds per vehicle)
A	0.0 to 10.0
B	10.1 to 20.0
C	20.1 to 35.0
D	35.1 to 55.0
E	55.1 to 80.0
F	greater than 80.0

Table II
**Level of Service Criteria
for Unsignalized Intersections**

Level of Service	Average Control Delay (seconds per vehicle)
a	0.0 to 10.0
b	10.1 to 15.0
c	15.1 to 25.0
d	25.1 to 35.0
e	35.1 to 50.0
f	greater than 50.0

It should be noted that the analyses within the *Highway Capacity Manual* assume a random arrival for all the movements, which may not be the case if an adjacent traffic signal is present that platoons vehicles, as there are multiple signalized intersections present along the Route 303 corridor.

All capacity analyses were performed utilizing Synchro 11 software. It should be noted that the existing percentage of trucks and peak hour factors were used in the existing analysis. Table III summarizes the existing levels of service (LOS) and delays. All capacity analysis calculation worksheets are contained in Appendix C.

Table III
Existing Levels of Service

Intersection	Direction/ Movement		AM PSH	PM PSH
Route 303 & Mountainview Avenue	EB	LTR	E (59)	D (37)
	WB	LTR	F (86)	F (84)
	NB	LTR	D (49)	E (74)
	SB	LTR	C (31)	D (42)
	Overall		D (46)	E (59)
Route 303 & Orangeburg Road (CR 20)/ Driveway	EB	LT	E (61)	E (69)
		R	A (6)	A (5)
	WB	LT	E (57)	E (70)
		R	A (0)	A (0)
	NB	LTR	B (16)	C (28)
	SB	LTR	C (22)	D (36)
Overall		C (22)	C (35)	
Route 303 & Route 340/ South Greenbush Road	EB	LTR	D (35)	C (25)
	WB	LTR	D (47)	E (56)
	NB	LTR	B (16)	C (24)
	SB	LTR	A (9)	B (18)
	Overall		B (19)	C (29)
Route 303 & Glenshaw Street	EB	LR	c (19)	c (22)
	NB	L	a (9)	b (13)

a (#) - Unsignalized Intersection Level of Service (seconds of delay per vehicle)

A (#) - Signalized Intersection Level of Service (seconds of delay per vehicle)

The following are discussions pertaining to each of the existing intersections analyzed.

Route 303 & Mountainview Avenue

Mountainview Avenue intersects Route 303 to form a four-leg intersection controlled by a traffic signal. The signal timing directive was obtained from NYSDOT which indicates that a three-phase cycle is utilized with a 110-second background cycle length. It should be noted that the Route 303 approaches operate in a split-phased format. The traffic signal timing directive is included in Appendix B. The northbound and southbound approaches of Route 303 each provide a shared left turn/through lane and a shared through/right turn lane. The eastbound and westbound approaches of Mountainview Avenue each provide a shared left turn/through/right turn lane.

A review of the existing analysis reveals that the intersection operates at levels of service “E” or better and all movements operate at levels of service “E” or better during the analyzed peak periods, with the exception of the westbound approach which operates at level of service “F” during both peak hours. See Table III for the individual movement levels of service and delays.

Route 303 & Orangeburg Road (CR 20)/Driveway

Orangeburg Road intersects Route 303 opposite a full movement driveway to a Chase Bank located at 333 NY-303, Orangeburg, NY, to form a four-leg intersection controlled by a traffic signal. The signal timing directive was obtained from NYSDOT which indicates that a four-phase 110-second background cycle is utilized (the traffic signal timing directive is included in Appendix B).

Both the northbound and southbound approaches of Route 303 provide a shared left turn/through lane and a shared through/right turn lane. The eastbound approach of Orangeburg Road provides a shared left turn/through lane and a dedicated right turn lane. The westbound approach of the driveway provides a shared left turn/through lane and a dedicated right turn lane.

A review of the existing analysis reveals that the intersection operates at levels of service “C” and all movements operate at levels of service “E” or better during the analyzed peak periods. See Table III for the individual movement levels of service and delays.

Route 303 & Route 340/South Greenbush Road

Route 340 intersects Route 303 opposite South Greenbush Road to form a four-leg intersection controlled by a traffic signal. The signal timing directive was obtained from NYSDOT which indicates that a three-phase 110 second background cycle is utilized (the traffic signal timing directive is included in Appendix B).

Both the northbound and southbound approaches of Route 303 provide a shared left turn/through lane and a shared through/right turn lane. The eastbound approach of South Greenbush Road and westbound approach of Route 340 both provide a shared left turn/through/right turn lane.

A review of the existing analysis reveals that the intersection operates at levels of service “C” or better and all movements operate at levels of service “E” or better during the analyzed peak periods. See Table III for the individual movement levels of service and delays.

Route 303 & Glenshaw Street

Glenshaw Street intersects Route 303 to form an unsignalized T-intersection with the eastbound approach of Glenshaw Street operating under stop control. The northbound approach of Route 303 provides a shared left turn/through lane and a dedicated through lane, while the southbound approach provides a dedicated through lane and a shared through/right turn lane. The eastbound approach of Glenshaw Street provides a shared left turn/right turn lane.

A review of the existing analysis reveals that all movements operate at levels of service “C” or better during the analyzed peak periods. See Table III for the individual movement levels of service and delays.

FUTURE CONDITIONS

Traffic volumes and operational analyses were developed for both the 2023 No Build and Build conditions. It should be noted that a meeting with NYSDOT and this office was held on March 25, 2021, where it was requested to additionally analyze 20 years after the No Build year (2043), which will be referred to as the Design Horizon Year for the remainder of this report. As such traffic volumes and operational analyses were developed for both the Design Horizon Year No Build and Design Horizon Year Build conditions. The No Build conditions provide a baseline for assessing the impact of the site development traffic on the roadway system. The process of developing the No Build, Build, Design Horizon Year No Build, and Design Horizon Year Build traffic volumes and the subsequent analyses is outlined below.

It should be noted that a subsequent review letter for the adjacent 125 and 155 Greenbush Road project, prepared by AKRF, dated February 26, 2021, requests use of a 1% background growth rate per year, as opposed to the 0.5% background growth rate utilized within *Traffic Impact Study* for 125 and 155 Greenbush Road, prepared by Maser Consulting and dated November, 2020. Therefore, in order to account for the difference in growth rate, the No Build volumes shown in Figure 4 were then grown utilizing a growth rate of 2%.

Through consultation with the Town of Orangetown Planning Board staff, there are six developments in the vicinity of the site that has been approved but not yet constructed that are identified as a potential significant traffic generator, shown below. The Adjacent Development Traffic Volumes passing the site are shown on Figure 3.

- The redevelopment of 125 Greenbush Road and development of 155 Greenbush Road to include a total of 543,000 SF of warehouse space;
- The reoccupancy of 700 Bradley Hill Road, which consists of 117,325 SF of warehouse space;
- The development of 200-400 Oritani Drive with an Amazon warehouse facility;
- An industrial development known as Instrumentation Laboratory, located at 526 Route 303;
- A retail development known as “S” Corner Plaza, located at the southeast corner of Route 303 and Route 304;
- A 170,737 SF warehouse development known as Linen Choice Warehouse, located along Glenshaw Street.

In order to generate the 2043 Design Horizon Year No Build traffic volumes, a background growth rate of 1.0% was applied to the No Build traffic volumes for a period of twenty (20) years, consistent with the growth rate utilized by Creighton Manning within the *Traffic Impact Study for Proposed WPT Industrial REIT Warehouse Development*, dated September 1, 2022. Figure 11, in Appendix A, illustrates the Design Horizon Year No Build traffic volumes.

Traffic Generation

Trip generation projections for The Project were prepared utilizing trip generation research data as published under Land Use Code 150 – Warehousing in the Institute of Transportation Engineers' (ITE) publication, *Trip Generation, 11th Edition*. This publication sets forth trip generation rates based on traffic counts conducted at research sites throughout the country. It should also be noted that consistent with the trip generation data, the truck trip generation rates and directional distributions were referenced from the ITE publication, *Trip Generation, 11th Edition*.

Table IV
Trip Generation Considering Car and Truck Traffic

Land Use	Trip Type	AM PSH			PM PSH		
		In	Out	Total	In	Out	Total
175,760 SF Warehouse	Cars	33	10	43	12	33	45
	Trucks	2	0	2	1	2	3
	Total	35	10	45	13	35	48

As mentioned previously, the site was previously occupied by a church which has trip generation potential if it were reoccupied. The following Table V compares the proposed use to the existing trip generation potential of the site.

Table V
Existing vs. Proposed Trip Generation Comparison

Land Use	AM PSH			PM PSH		
	In	Out	Total	In	Out	Total
Existing 106,000 SF Church	22	15	37	23	29	52
Proposed 175,760 SF Warehouse	35	10	45	13	35	48
Difference	+13	-5	+8	-10	+6	-4

As shown in Table V above, it is anticipated that 8 additional trips during the weekday morning peak hour and 4 fewer trips during the weekday evening peak hour are anticipated to access the site from the adjacent roadway network with the proposed redevelopment. However, no credit was taken for the existing use of the site and all trip generation was considered an increase over vacant land. This accounts for a “worst case scenario” from a traffic impact perspective.

Furthermore, it should be noted that the number of new trips falls below the industry accepted standard of a significant increase in traffic of 100 trips. Based on *Transportation Impact Analysis for Site Development*, published by the ITE “it is suggested that a transportation impact study be conducted whenever a proposed development will generate 100 or more added (new) trips during the adjacent roadways’ peak hour or the development’s peak hour.”

Once the magnitude of traffic to be generated by the site is known, it is necessary to assign that traffic to the adjacent street system. The distribution of new traffic to the surrounding roadways is based on the location of primary arterial roadways, major signalized intersections and existing traffic patterns.

Located in Appendix A, Figure 5 illustrates the percent distribution of car site generated trips, Figure 6 illustrates the car site generated volumes, Figure 7 illustrates the percent distribution of truck site generated trips, Figure 8 illustrates the truck site generated volumes, and Figure 9 illustrates the total site generated volumes assigned to the study area network. The site generated volumes were added to the 2023 No Build traffic volumes and 2043 Design Horizon Year No Build traffic volumes to generate the 2023 Build traffic volumes and 2043 Design Horizon Year Build traffic volumes, which are shown in Figures 10 and 12, respectively.

Future Capacity Analysis

Operational conditions at the study intersections were analyzed under the No Build, Build, Design Horizon Year No Build, and Design Horizon Year Build conditions and are summarized in Table VI and VII below.

Table VI
2023 Future Levels of Service

Intersection	Direction/ Movement	AM PSH			PM PSH		
		No Build	Build	Build w/ Mit.	No Build	Build	Build w/ Mit.
Route 303 & Mountainview Avenue	EB	L	D (47)	D (48)	D (40)	C (35)	D (39)
		TR	-	-	C (23)	-	B (15)
	WB	LTR	F (91)	F (91)	F (90)	F (141)	F (147)
	NB	LTR	E (61)	E (64)	E (64)	F (94)	F (98)
	SB	LTR	D (39)	D (39)	D (39)	E (73)	E (76)
	Overall		D (54)	E (56)	D (53)	F (87)	F (90)
Route 303 & Orangetown Road (CR 20) / Driveway	EB	LT	E (62)	E (63)	-	E (71)	E (71)
		R	A (6)	A (6)	-	A (6)	A (7)
	WB	LT	E (57)	E (57)	-	E (70)	E (70)
		R	A (0)	A (0)	-	A (0)	A (0)
	NB	LTR	B (18)	B (18)	-	C (30)	C (31)
	SB	LTR	C (29)	C (30)	-	D (40)	D (41)
	Overall		C (26)	C (26)	-	D (38)	D (38)
Route 303 & Route 340/ South Greenbush Road	EB	LTR	C (34)	C (34)	-	C (25)	C (25)
	WB	LTR	D (46)	D (46)	-	E (62)	E (63)
	NB	LTR	B (17)	B (18)	-	C (27)	C (27)
	SB	LTR	B (10)	B (10)	-	C (25)	C (26)
	Overall		B (20)	B (20)	-	C (34)	C (34)
Route 303 & Glenshaw Street	EB	LR	d (28)	d (29)	-	e (40)	e (42)
	NB	L	b (10)	b (10)	-	b (14)	b (14)
		T	a (1)	a (1)	-	a (1)	a (1)
Route 303 & Site Driveway	EB	R	-	b (11)	-	-	b (12)
Mountainview Avenue & Site Driveway	EB	L	-	a (8)	-	-	a (8)
	SB	LR	-	b (12)	-	-	b (12)

a (#) - Unsignalized Intersection Level of Service (seconds of delay per vehicle)

A (#) - Signalized Intersection Level of Service (seconds of delay per vehicle)

Table VII
2043 Future Levels of Service

Intersection	Direction/ Movement	AM PSH			PM PSH		
		No Build	Build	Build w/ Mit.	No Build	Build	Build w/ Mit.
Route 303 & Mountainview Avenue	EB	L	E (56)	E (59)	D (41)	D (41)	D (40)
		TR	-	-	C (27)	-	B (19)
	WB	LTR	F (137)	F (138)	F (151)	F (283)	F (293)
	NB	LTR	F (122)	F (129)	F (129)	F (193)	F (197)
	SB	LTR	E (63)	E (64)	E (64)	F (154)	F (160)
	Overall		F (94)	F (99)	F (97)	F (177)	F (181)
Route 303 & Orangeburg Road (CR 20) / Driveway	EB	LT	E (66)	E (67)	-	F (88)	F (89)
		R	B (11)	B (11)	-	B (12)	B (12)
	WB	LT	E (58)	E (58)	-	E (73)	E (73)
		R	A (0)	A (0)	-	A (1)	A (1)
	NB	LTR	C (30)	C (32)	-	E (65)	E (66)
	SB	LTR	D (35)	D (36)	-	E (64)	E (69)
	Overall		C (33)	C (34)	-	E (63)	E (65)
Route 303 & Route 340/ South Greenbush Road	EB	LTR	C (31)	C (31)	-	C (26)	C (26)
	WB	LTR	D (50)	D (50)	-	F (123)	F (124)
	NB	LTR	C (24)	C (24)	-	C (31)	C (31)
	SB	LTR	B (19)	B (19)	-	F (88)	F (91)
	Overall		C (26)	C (27)	-	E (77)	E (79)
Route 303 & Glenshaw Street	EB	LR	f (64)	f (67)	-	f (185)	f (194)
	NB	L	b (12)	b (12)	-	c (17)	c (17)
		T	a (1)	a (1)	-	a (2)	a (2)
Route 303 & Site Driveway	EB	R	-	b (11)	-	-	b (13)
Mountainview Avenue & Site Driveway	EB	L	-	a (8)	-	-	a (8)
	SB	LR	-	b (13)	-	-	b (13)

a (#) - Unsignalized Intersection Level of Service (seconds of delay per vehicle)

A (#) - Signalized Intersection Level of Service (seconds of delay per vehicle)

Route 303 & Mountainview Avenue

With the addition of site generated traffic, the intersection is anticipated to operate at overall No Build level of service “F”. Additionally, the westbound and northbound approaches are anticipated to operate at level of service “F” during both peak hours, and the southbound approach is anticipated to operate at level of service “F” during the weekday evening peak hour. It is important to note that these operational conditions are already present under No Build conditions, indicating that the increased delays are a function of the magnitude of existing traffic volumes rather than the addition of the site traffic. The additional site traffic at the intersection represents approximately 1.3% of the peak hour traffic.

It should be noted that it is proposed by The Project to construct intersection improvements in the future. Specifically, Mountainview Avenue is proposed to be widened to the Town's desirable cartway width of 40' as well as construct a dedicated left turn lane and a shared through/right turn lane for the eastbound approach of the intersection. These improvements are analyzed in the Build with Mitigation scenario. In the Build with Mitigation scenario, the eastbound and westbound approaches are generally anticipated to decrease delays. See Tables VI and VII for the individual movement levels of service and delays.

Route 303 & Orangeburg Road (CR 20)/Driveway

With the addition of site generated traffic, the intersection is anticipated to continue to operate at No Build levels of service "E" or better during the studied peak periods. Additionally, each movement is anticipated to continue to generally operate at No Build levels of service with little to no changes in delay. See Tables VI and VII for the individual movement levels of service and delays.

Route 303 & Route 340/South Greenbush Road

With the addition of site generated traffic, the intersection is anticipated to continue to operate at No Build levels of service "E" or better during the studied peak periods. Additionally, each movement is anticipated to continue to operate at No Build levels of service "D" or better with little to no changes in delay, with the exception of the westbound and southbound approaches which are anticipated to continue to operate at No Build level of service "F" during the weekday evening peak hour. The additional site traffic at the intersection represents approximately 0.5% of the peak hour traffic. See Tables VI and VII for the individual movement levels of service and delays.

Route 303 & Glenshaw Street

With the addition of site generated traffic, all movements at the intersection are anticipated to continue to operate at No Build levels of service "C" or better during the studied peak periods, with the exception of the eastbound approach which is anticipated to continue to operate at No Build level of service "F" during both peak hours. See Tables VI and VII for the individual movement levels of service and delays.

A Traffic Signal Warrant Analysis for the intersection of Route 303 and Glenshaw Street was prepared and is included in Appendix D. This analysis presents a comprehensive investigation of traffic conditions and physical characteristics required to determine the necessity for a signal installation and to determine a proper design and control utilizing the Traffic Signal Warrants as set forth in the Manual on Uniform Traffic Control Devices for Street and Highways, 2009 Edition (MUTCD). The following warrants were reviewed:

- Warrant 1 – Eight-Hour Vehicular Volume
- Warrant 2 – Four-Hour Vehicular Volume
- Warrant 3 – Peak Hour

In order to provide a conservative analysis, the 2043 Design Horizon Year was utilized for the traffic signal warrant analyses contained herein. The traffic volumes at the subject intersection were adjusted and grown according to the methodologies outlined in this report. As shown in the analysis worksheets contained in Appendix D, the subject intersection satisfies Warrant 3. However, as the intersection only meets Warrant 3 during one peak hour, the installation of a traffic signal is not recommended.

Route 303 & Site Driveway

The site driveway is proposed to intersect Route 303 to form an unsignalized T-intersection with the eastbound approach of the site driveway operating under stop control. The eastbound approach of the site driveway is proposed to provide a dedicated right turn lane. The northbound approach of Route 303 provides two dedicated through lanes, while the southbound approach provides a dedicated through lane and a shared through/right turn lane.

As designed, the individual intersection movements are anticipated to operate at levels of service “B” during the studied peak hours. See Tables VI and VII for the individual movement levels of service and delays.

Mountainview Avenue & Site Driveway

The site driveway is proposed to intersect Mountainview Avenue to form an unsignalized T-intersection with the southbound approach of the site driveway operating under stop control. The eastbound and westbound approaches of Mountainview Avenue are proposed to provide a shared left turn/through lane and a shared through/right turn lane, respectively. The southbound approach of the site driveway is proposed to provide a single lane for left and right turns.

As designed, the individual intersection movements are anticipated to operate at levels of service “B” or better during the studied peak hours. See Tables VI and VII for the individual movement levels of service and delays.

Queue Analysis

Queue length conditions at the study intersections were analyzed under the No Build and Build conditions. The 50th and 95th percentile queues for each study peak hour are summarized in Tables VIII, IX, X and XI below.

Table VIII
2023 Queue Analysis - 50th Percentile

Intersection	Direction/ Movement	Storage Length	AM PSH			PM PSH		
			No Build	Build	Build w/ Mit.	No Build	Build	Build w/ Mit.
Route 303 & Mountainview Avenue	EB	L	195'	130'	135'	40'	95'	114'
		TR	-	-	-	55'	-	27'
	WB	LTR	-	122'	124'	123'	238'	242'
	NB	LTR	-	345'	353'	353'	457'	463'
	SB	LTR	-	243'	245'	245'	394'	405'
Route 303 & Orangetown Road (CR 20)/ Driveway	EB	LT	-	162'	167'	-	202'	203'
		R	-	-	-	-	12'	13'
	WB	LT	-	17'	17'	-	46'	46'
	NB	LTR	-	88'	91'	-	171'	172'
	SB	LTR	-	191'	192'	-	302'	306'
Route 303 & Route 340/ South Greenbush Road	EB	LTR	-	36'	36'	-	33'	33'
	WB	LTR	-	150'	153'	-	307'	308'
	NB	LTR	-	148'	150'	-	204'	205'
	SB	LTR	-	69'	70'	-	191'	196'
Route 303 & Glenshaw Street	EB	LR	-	-	-	-	-	-
	NB	L	-	-	-	-	-	-
Route 303 & Site Driveway	EB	R	-	-	-	-	-	-
Mountainview Avenue & Site Driveway	SB	LR	-	-	-	-	-	-

Table IX
2023 Queue Analysis - 95th Percentile

Intersection	Direction/ Movement	Storage Length	AM PSH			PM PSH		
			No Build	Build	Build w/ Mit.	No Build	Build	Build w/ Mit.
Route 303 & Mountainview Avenue	EB	L	195'	220'	227'	82'	171'	196'
		TR	-	-	-	120'	-	-
	WB	LTR	-	254'	259'	256'	412'	417'
	NB	LTR	-	468'	480'	480'	585'	590'
	SB	LTR	-	319'	321'	321'	526'	538'
Route 303 & Orangeburg Road (CR 20)/ Driveway	EB	LT	-	238'	247'	-	343'	346'
		R	-	54'	54'	-	64'	65'
	WB	LT	-	43'	43'	-	93'	93'
	NB	LTR	-	162'	164'	-	231'	234'
	SB	LTR	-	240'	241'	-	293'	294'
Route 303 & Route 340/ South Greenbush Road	EB	LTR	-	68'	68'	-	69'	69'
	WB	LTR	-	232'	237'	-	530'	533'
	NB	LTR	-	240'	243'	-	266'	267'
	SB	LTR	-	143'	145'	-	274'	286'
Route 303 & Glenshaw Street	EB	LR	-	33'	33'	-	103'	105'
	NB	L	-	10'	10'	-	5'	5'
Route 303 & Site Driveway	EB	R	-	-	-	-	3'	-
Mountainview Avenue & Site Driveway	SB	LR	-	-	-	-	5'	-

Table X
2043 Queue Analysis - 50th Percentile

Intersection	Direction/ Movement	Storage Length	AM PSH			PM PSH		
			No Build	Build	Build w/ Mit.	No Build	Build	Build w/ Mit.
Route 303 & Mountainview Avenue	EB	L	195'	174'	178'	50'	126'	147'
		TR	-	-	-	80'	-	-
	WB	LTR	-	183'	186'	191'	351'	355'
	NB	LTR	-	489'	504'	504'	645'	652'
	SB	LTR	-	322'	325'	325'	569'	580'
Route 303 & Orangeburg Road (CR 20)/ Driveway	EB	LT	-	193'	198'	-	256'	258'
		R	-	37'	38'	-	51'	52'
	WB	LT	-	17'	17'	-	47'	47'
	NB	LTR	-	192'	200'	-	341'	346'
	SB	LTR	-	245'	247'	-	434'	444'
Route 303 & Route 340/ South Greenbush Road	EB	LTR	-	43'	43'	-	41'	41'
	WB	LTR	-	202'	205'	-	497'	500'
	NB	LTR	-	222'	225'	-	271'	272'
	SB	LTR	-	140'	146'	-	263'	270'
Route 303 & Glenshaw Street	EB	LR	-	-	-	-	-	-
	NB	L	-	-	-	-	-	-
Route 303 & Site Driveway	EB	R	-	-	-	-	-	-
Mountainview Avenue & Site Driveway	SB	LR	-	-	-	-	-	-

Table XI
2043 Queue Analysis - 95th Percentile

Intersection	Direction/ Movement	Storage Length	AM PSH			PM PSH		
			No Build	Build	Build w/ Mit.	No Build	Build	Build w/ Mit.
Route 303 & Mountainview Avenue	EB	L	195'	321'	331'	98'	214'	260'
		TR	-	-	-	156'	-	-
	WB	LTR	-	342'	347'	352'	537'	543'
	NB	LTR	-	590'	595'	595'	607'	609'
	SB	LTR	-	461'	466'	466'	706'	718'
Route 303 & Orangeburg Road (CR 20)/ Driveway	EB	LT	-	309'	324'	-	448'	452'
		R	-	112'	113'	-	122'	122'
	WB	LT	-	43'	43'	-	100'	100'
	NB	LTR	-	381'	420'	-	364'	369'
	SB	LTR	-	253'	253'	-	296'	296'
Route 303 & Route 340/ South Greenbush Road	EB	LTR	-	79'	79'	-	83'	83'
	WB	LTR	-	303'	309'	-	724'	728'
	NB	LTR	-	329'	331'	-	349'	350'
	SB	LTR	-	264'	269'	-	276'	275'
Route 303 & Glenshaw Street	EB	LR	-	80'	83'	-	283'	288'
	NB	L	-	15'	15'	-	8'	8'
Route 303 & Site Driveway	EB	R	-	-	-	-	-	5'
Mountainview Avenue & Site Driveway	SB	LR	-	-	-	-	-	5'

Route 303 & Mountainview Avenue

With the addition of site generated traffic, there is anticipated to be a maximum increase of approximately 2 vehicles in the 50th and 95th percentile queues for all movements at the intersection. It is not anticipated that the increase in queues will have a detrimental impact on the operation of the intersection. It should be noted that in the Build with Mitigation scenario, the eastbound approach queues decreased by approximately 6 vehicles. See Tables VIII, IX, X and XI for the individual movement 50th and 95th percentile queues.

Route 303 & Orangeburg Road (CR 20)/Driveway

With the addition of site generated traffic, there is anticipated to be a maximum increase of approximately 2 vehicles in the 50th and 95th percentile queues for all movements at the intersection. It is not anticipated that the increase in queues will have a detrimental impact on the operation of the intersection. See Tables VIII, IX, X and XI for the individual movement 50th and 95th percentile queues.

Route 303 & Route 340/South Greenbush Road

With the addition of site generated traffic, there is anticipated to be a minimal increase in the 50th and 95th percentile queues for all movements at the intersection. It is not anticipated that the increase in queues will have a detrimental impact on the operation of the intersection. See Tables VIII, IX, X and XI for the individual movement 50th and 95th percentile queues.

Route 303 & Glenshaw Street

With the addition of site generated traffic, there is anticipated to be a minimal increase in the 95th percentile queues for all movements at the intersection. It is not anticipated that the increase in queues will have a detrimental impact on the operation of the intersection. It should be noted that Synchro analyses do not provide 50th percentile queues for unsignalized intersections. See Tables IX and XI for the individual movement 95th percentile queues.

Route 303 & Site Driveway

As designed, the site driveway is anticipated to operate with a 95th percentile queue length of 5 feet. The driveway provides significant throat length prior to the first on-site intersection. Therefore, it is not anticipated that this queue will impact on-site circulation. It should be noted that Synchro analyses do not provide 50th percentile queues for unsignalized intersections. See Tables IX and XI for the individual movement 95th percentile queues.

Mountainview Avenue & Site Driveway

As designed, the site driveway is anticipated to operate with a 95th percentile queue length of 5 feet. The driveway provides significant throat length prior to the first on-site intersection. Therefore, it is not anticipated that this queue will impact on-site circulation. It should be noted that Synchro analyses do not provide 50th percentile queues for unsignalized intersections. See Tables IX and XI for the individual movement 95th percentile queues.

ALTERNATE FUTURE CONDITIONS

The tenant for the proposed warehouse is undisclosed, therefore, a larger traffic generating use could occupy the proposed buildings. As such, an Alternate Analysis was conducted using LUC 156 – High-Cube Parcel Hub Warehouse, as requested by the Town of Orangetown Planning Board.

Alternate Traffic Generation

Trip generation projections for the Alternate Analysis were prepared utilizing trip generation research data as published under LUC 156 – High-Cube Parcel Hub Warehouse in the ITE publication, *Trip Generation, 11th Edition*. This publication sets forth trip generation rates based on traffic counts conducted at research sites throughout the country. It should also be noted that the ITE publication, *Trip Generation, 11th Edition* does not publish truck trip generation rates and directional distributions for LUC 156. As such, the truck trip generation rates and directional distributions for LUC 150 was applied.

Table XII
Alternate Trip Generation

Land Use	Trip Type	AM PSH			PM PSH		
		In	Out	Total	In	Out	Total
175,760 SF High-Cube Parcel Hub Warehouse	Cars	59	57	116	71	33	104
	Trucks	3	4	7	5	3	8
	Total	62	61	123	76	36	112

The site generated trips for the Alternate Analysis were assigned to the adjacent roadways based on the distributions outlined previously. Alternate traffic volumes are included in Appendix E for reference.

Alternate Future Capacity Analysis

Operational conditions at the study intersections were analyzed under the No Build, Build, Design Horizon Year No Build, and Design Horizon Year Build conditions and are summarized in Table XIII and XIV below. All alternate capacity analysis worksheets are contained in Appendix F.

Table XIII
2023 Future Levels of Service

Intersection	Direction / Movement	AM PSH			PM PSH		
		No Build	Build	Build w/ Mit.	No Build	Build	Build w/ Mit.
Route 303 & Mountainview Avenue	EB	L	D (47)	E (63)	D (43)	C (35)	D (40)
		TR	-	-	C (25)	-	B (15)
	WB	LTR	F (91)	F (91)	F (94)	F (141)	F (152)
	NB	LTR	E (61)	E (67)	E (67)	F (94)	F (110)
	SB	LTR	D (39)	D (41)	D (41)	E (73)	E (76)
	Overall		D (54)	E (59)	E (56)	F (87)	F (96)
Route 303 & Orangeburg Road (CR 20)/ Driveway	EB	LT	E (62)	E (63)	-	E (71)	E (73)
		R	A (6)	A (6)	-	A (6)	A (7)
	WB	LT	E (57)	E (57)	-	E (70)	E (71)
		R	A (0)	A (0)	-	A (0)	A (0)
	NB	LTR	B (18)	B (19)	-	C (30)	C (33)
	SB	LTR	C (29)	C (31)	-	D (40)	D (41)
Overall		C (26)	C (27)	-	D (38)	D (39)	-
Route 303 & Route 340/ South Greenbush Road	EB	LTR	C (34)	C (34)	-	C (25)	C (25)
	WB	LTR	D (46)	D (46)	-	E (62)	E (63)
	NB	LTR	B (17)	B (18)	-	C (27)	C (27)
	SB	LTR	B (10)	B (11)	-	C (25)	C (27)
Overall		B (20)	C (20)	-	C (34)	C (35)	-
Route 303 & Glenshaw Street	EB	LR	d (28)	d (30)	-	e (40)	e (44)
	NB	L	b (10)	b (10)	-	b (14)	b (14)
		T	a (1)	a (1)	-	a (1)	a (1)
Route 303 & Site Driveway	EB	R	-	b (11)	-	-	b (12)
Mountainview Avenue & Site Driveway	EB	L	-	a (8)	-	-	a (8)
	SB	LR	-	b (13)	-	-	b (13)

a (#) - Unsignalized Intersection Level of Service (seconds of delay per vehicle)

A (#) - Signalized Intersection Level of Service (seconds of delay per vehicle)

Table XIV
2043 Future Levels of Service

Intersection	Direction/ Movement	AM PSH			PM PSH		
		No Build	Build	Build w/ Mit.	No Build	Build	Build w/ Mit.
Route 303 & Mountainview Avenue	EB	L	E (56)	F (82)	D (44)	D (41)	D (47)
		TR	-	-	C (28)	-	B (19)
	WB	LTR	F (137)	F (142)	F (170)	F (283)	F (303)
	NB	LTR	F (122)	F (135)	F (135)	F (193)	F (211)
	SB	LTR	E (63)	E (70)	E (70)	F (154)	F (160)
	Overall		F (94)	F (106)	F (102)	F (177)	F (189)
Route 303 & Orangeburg Road (CR 20)/ Driveway	EB	LT	E (66)	E (67)	-	F (88)	F (98)
		R	B (11)	B (11)	-	B (12)	B (12)
	WB	LT	E (58)	E (58)	-	E (73)	E (73)
		R	A (0)	A (0)	-	A (1)	A (1)
	NB	LTR	C (30)	C (34)	-	E (65)	E (72)
	SB	LTR	D (35)	D (36)	-	E (64)	E (69)
	Overall		C (33)	D (35)	-	E (63)	E (69)
Route 303 & Route 340/ South Greenbush Road	EB	LTR	C (31)	C (31)	-	C (26)	C (26)
	WB	LTR	D (50)	D (50)	-	F (123)	F (130)
	NB	LTR	C (24)	C (24)	-	C (31)	C (32)
	SB	LTR	B (19)	C (20)	-	F (88)	F (93)
	Overall		C (26)	C (27)	-	E (77)	F (81)
Route 303 & Glenshaw Street	EB	LR	f (64)	f (72)	-	f (185)	f (216)
	NB	L	b (12)	b (12)	-	c (17)	c (17)
		T	a (1)	a (2)	-	a (2)	a (2)
Route 303 & Site Driveway	EB	R	-	b (12)	-	-	b (13)
Mountainview Avenue & Site Driveway	EB	L	-	a (8)	-	-	a (8)
	SB	LR	-	b (14)	-	-	b (14)

a (#) - Unsignalized Intersection Level of Service (seconds of delay per vehicle)

A (#) - Signalized Intersection Level of Service (seconds of delay per vehicle)

Route 303 & Mountainview Avenue

With consideration of the alternative scenario, the intersection is anticipated to operate at overall No Build level of service “F”. Additionally, the eastbound, westbound and northbound approaches are anticipated to operate at level of service “F” during the weekday morning peak hour. Further, the westbound, northbound and southbound approaches are anticipated to operate at level of service “F” during the weekday evening peak hour. It is important to note that these operational conditions are already present under No Build conditions, indicating that the increased delays are a function of the magnitude of existing traffic volumes rather than the addition of the site traffic. The additional site traffic at the intersection represents approximately 3.2% of the peak hour traffic.

As noted previously, it is proposed by The Project to construct intersection improvements in the future. Specifically, Mountainview Avenue is proposed to be widened to the Town's desirable cartway width of 40' as well as construct a dedicated left turn lane and a shared through/right turn lane for the eastbound approach of the intersection. These improvements are analyzed in the Build with Mitigation scenario. In the Build with Mitigation scenario, the eastbound and westbound approaches are generally anticipated to decrease delays. See Tables XIII and XIV for the individual movement levels of service and delays.

Route 303 & Orangeburg Road (CR 20)/Driveway

With consideration of the alternative scenario, the intersection is anticipated to continue to operate at No Build levels of service "E" or better during the studied peak periods. Additionally, each movement is anticipated to continue to generally operate at No Build levels of service with little to no changes in delay. See Tables XIII and XIV for the individual movement levels of service and delays.

Route 303 & Route 340/South Greenbush Road

With consideration of the alternative scenario, the intersection is anticipated to continue to operate at No Build levels of service "C" during the weekday morning peak hour and at level of service "F" during the weekday evening peak hour. It should be noted that only an additional four (4) seconds of delay are added to the overall intersection delay during the weekday evening peak hour during Design Horizon Year conditions. Additionally, each movement is anticipated to continue to operate at No Build levels of service "D" or better with little to no changes in delay, with the exception of the westbound and southbound approaches which are anticipated to continue to operate at No Build level of service "F" during the weekday evening peak hour. The additional site traffic at the intersection represents approximately 1.3% of the peak hour traffic. See Tables XIII and XIV for the individual movement levels of service and delays.

Route 303 & Glenshaw Street

With consideration of the alternative scenario, all movements at the intersection are anticipated to continue to operate at No Build levels of service "C" or better during the studied peak periods, with the exception of the eastbound approach which is anticipated to continue to operate at No Build level of service "F" during both peak hours. See Tables XIII and XIV for the individual movement levels of service and delays.

An alternate Traffic Signal Warrant Analysis for the intersection of Route 303 and Glenshaw Street was prepared and is included in Appendix G. This analysis presents a comprehensive investigation of traffic conditions and physical characteristics required to determine the necessity for a signal installation and to determine a proper design and control utilizing the Traffic Signal Warrants as set forth in the Manual on Uniform Traffic Control Devices for Street and Highways, 2009 Edition (MUTCD). The following warrants were reviewed:

- Warrant 1 – Eight-Hour Vehicular Volume
- Warrant 2 – Four-Hour Vehicular Volume
- Warrant 3 – Peak Hour

In order to provide a conservative analysis, the 2043 Design Horizon Year was utilized for the traffic signal warrant analyses contained herein. The traffic volumes at the subject intersection were adjusted and grown according to the methodologies outlined in this report. As shown in the analysis worksheets contained in Appendix G, the subject intersection satisfies Warrant 3. However, as the intersection only meets Warrant 3 during one peak hour, the installation of a traffic signal is not recommended.

Route 303 & Site Driveway

As designed, the individual intersection movements are anticipated to operate at levels of service “B” during the studied peak hours. See Tables XIII and XIV for the individual movement levels of service and delays.

Mountainview Avenue & Site Driveway

As designed, the individual intersection movements are anticipated to operate at levels of service “B” or better during the studied peak hours. See Tables XIII and XIV for the individual movement levels of service and delays.

Alternate Queue Analysis

Alternate queue length conditions at the study intersections were analyzed under the No Build and Build conditions. The 50th and 95th percentile queues for each study peak hour are summarized in Tables XV, XVI, XVII and XVIII below.

Table XV
2023 Queue Analysis - 50th Percentile

Intersection	Direction/ Movement		Storage Length	AM PSH			PM PSH		
				No Build	Build	Build w/ Mit.	No Build	Build	Build w/ Mit.
Route 303 & Mountainview Avenue	EB	L	195'	130'	164'	52'	95'	115'	42'
		TR	-	-	-	65'	-	-	27'
	WB	LTR	-	122'	126'	127'	238'	251'	234'
	NB	LTR	-	345'	358'	358'	457'	489'	489'
	SB	LTR	-	243'	255'	255'	394'	405'	405'
Route 303 & Orangetown Road (CR 20)/ Driveway	EB	LT	-	162'	171'	-	202'	214'	-
		R	-	-	-	-	12'	13'	-
	WB	LT	-	17'	17'	-	46'	46'	-
	NB	LTR	-	88'	93'	-	171'	180'	-
	SB	LTR	-	191'	200'	-	302'	306'	-
Route 303 & Route 340/ South Greenbush Road	EB	LTR	-	36'	36'	-	33'	33'	-
	WB	LTR	-	150'	155'	-	307'	319'	-
	NB	LTR	-	148'	152'	-	204'	207'	-
	SB	LTR	-	69'	70'	-	191'	198'	-
Route 303 & Glenshaw Street	EB	LR	-	-	-	-	-	-	-
	NB	L	-	-	-	-	-	-	-
Route 303 & Site Driveway	EB	R	-	-	-	-	-	-	-
Mountainview Avenue & Site Driveway	SB	LR	-	-	-	-	-	-	-

Table XVI
2023 Queue Analysis - 95th Percentile

Intersection	Direction/ Movement	Storage Length	AM PSH			PM PSH		
			No Build	Build	Build w/ Mit.	No Build	Build	Build w/ Mit.
Route 303 & Mountainview Avenue	EB	L	195'	220'	303'	102'	171'	198'
		TR	-	-	-	133'	-	81'
	WB	LTR	-	254'	264'	265'	412'	426'
	NB	LTR	-	468'	492'	492'	585'	601'
	SB	LTR	-	319'	335'	335'	526'	538'
Route 303 & Orangetown Road (CR 20)/ Driveway	EB	LT	-	238'	251'	-	343'	368'
		R	-	54'	54'	-	64'	65'
	WB	LT	-	43'	43'	-	93'	93'
	NB	LTR	-	162'	168'	-	231'	243'
	SB	LTR	-	240'	248'	-	293'	293'
Route 303 & Route 340/ South Greenbush Road	EB	LTR	-	68'	68'	-	69'	69'
	WB	LTR	-	232'	239'	-	530'	550'
	NB	LTR	-	240'	246'	-	266'	270'
	SB	LTR	-	143'	155'	-	274'	291'
Route 303 & Glenshaw Street	EB	LR	-	33'	35'	-	103'	110'
	NB	L	-	10'	10'	-	5'	5'
Route 303 & Site Driveway	EB	R	-	-	3'	-	-	-
Mountainview Avenue & Site Driveway	SB	LR	-	-	8'	-	-	-

Table XVII
2043 Queue Analysis - 50th Percentile

Intersection	Direction/ Movement	Storage Length	AM PSH			PM PSH		
			No Build	Build	Build w/ Mit.	No Build	Build	Build w/ Mit.
Route 303 & Mountainview Avenue	EB	L	195'	174'	208'	62'	126'	148'
		TR	-	-	-	88'	-	-
	WB	LTR	-	183'	190'	201'	351'	365'
	NB	LTR	-	489'	514'	514'	645'	676'
	SB	LTR	-	322'	338'	338'	569'	580'
Route 303 & Orangeburg Road (CR 20)/ Driveway	EB	LT	-	193'	201'	-	256'	286'
		R	-	37'	39'	-	51'	52'
	WB	LT	-	17'	17'	-	47'	47'
	NB	LTR	-	192'	127'	-	341'	407'
	SB	LTR	-	245'	254'	-	434'	444'
Route 303 & Route 340/ South Greenbush Road	EB	LTR	-	43'	43'	-	41'	41'
	WB	LTR	-	202'	207'	-	497'	516'
	NB	LTR	-	222'	227'	-	271'	275'
	SB	LTR	-	140'	156'	-	263'	272'
Route 303 & Glenshaw Street	EB	LR	-	-	-	-	-	-
	NB	L	-	-	-	-	-	-
Route 303 & Site Driveway	EB	R	-	-	-	-	-	-
Mountainview Avenue & Site Driveway	SB	LR	-	-	-	-	-	-

Table XVIII
2043 Queue Analysis - 95th Percentile

Intersection	Direction/ Movement	Storage Length	AM PSH			PM PSH		
			No Build	Build	Build w/ Mit.	No Build	Build	Build w/ Mit.
Route 303 & Mountainview Avenue	EB	L	195'	321'	393'	117'	214'	265'
		TR	-	-	-	166'	-	-
	WB	LTR	-	342'	351'	361'	537'	553'
	NB	LTR	-	590'	594'	594'	607'	614'
	SB	LTR	-	461'	482'	482'	706'	718'
Route 303 & Orangeburg Road (CR 20)/ Driveway	EB	LT	-	309'	332'	-	448'	473'
		R	-	112'	116'	-	122'	122'
	WB	LT	-	43'	43'	-	100'	101'
	NB	LTR	-	381'	462'	-	364'	391'
	SB	LTR	-	253'	251'	-	296'	296'
Route 303 & Route 340/ South Greenbush Road	EB	LTR	-	79'	79'	-	83'	83'
	WB	LTR	-	303'	314'	-	724'	744'
	NB	LTR	-	329'	332'	-	349'	354'
	SB	LTR	-	264'	287'	-	276'	276'
Route 303 & Glenshaw Street	EB	LR	-	80'	85'	-	283'	303'
	NB	L	-	15'	15'	-	8'	8'
Route 303 & Site Driveway	EB	R	-	-	3'	-	-	3'
Mountainview Avenue & Site Driveway	SB	LR	-	-	8'	-	-	5'

Route 303 & Mountainview Avenue

With consideration of the alternative scenario, there is anticipated to be a maximum increase of approximately 3 vehicles in the 50th and 95th percentile queues for all movements at the intersection. It is not anticipated that the increase in queues will have a detrimental impact on the operation of the intersection. It should be noted that in the Build with Mitigation scenario, the eastbound approach queues decreased by approximately 9 vehicles. See Tables XV, XVI, XVII and XVIII for the individual movement 50th and 95th percentile queues.

Route 303 & Orangeburg Road (CR 20)/Driveway

With consideration of the alternative scenario, there is anticipated to be a maximum increase of approximately 3 vehicles in the 50th and 95th percentile queues for all movements at the intersection. It is not anticipated that the increase in queues will have a detrimental impact on the operation of the intersection. See Tables XV, XVI, XVII and XVIII for the individual movement 50th and 95th percentile queues.

Route 303 & Route 340/South Greenbush Road

With consideration of the alternative scenario, there is anticipated to be a minimal increase in the 50th and 95th percentile queues for all movements at the intersection. It is not anticipated that the increase in queues will have a detrimental impact on the operation of the intersection. See Tables XV, XVI, XVII and XVIII for the individual movement 50th and 95th percentile queues.

Route 303 & Glenshaw Street

With consideration of the alternative scenario, there is anticipated to be maximum increase of approximately 1 vehicle in the 95th percentile queues for all movements at the intersection. It is not anticipated that the increase in queues will have a detrimental impact on the operation of the intersection. It should be noted that Synchro analyses do not provide 50th percentile queues for unsignalized intersections. See Tables XVI and XVIII for the individual movement 95th percentile queues.

Route 303 & Site Driveway

As designed, the site driveway is anticipated to operate with a 95th percentile queue length of 3 feet. The driveway provides significant throat length prior to the first on-site intersection. Therefore, it is not anticipated that this queue will impact on-site circulation. It should be noted that Synchro analyses do not provide 50th percentile queues for unsignalized intersections. See Tables XVI and XVIII for the individual movement 95th percentile queues.

Mountainview Avenue & Site Driveway

As designed, the site driveway is anticipated to operate with a 95th percentile queue length of 8 feet. The driveway provides significant throat length prior to the first on-site intersection. Therefore, it is not anticipated that this queue will impact on-site circulation. It should be noted that Synchro analyses do not provide 50th percentile queues for unsignalized intersections. See Tables XVI and XVIII for the individual movement 95th percentile queues.

SITE PLAN

Site Access and Circulation

The site plan was reviewed with respect to the site access and on-site circulation design. As noted previously, access to The Project will be provided via one (1) right turn in/right turn out driveway along Route 303 and one (1) reconstructed full movement driveway along Mountainview Avenue.

The newly constructed parking areas will be served by aisles with a width of 25 feet within the passenger car circulation area, which satisfies the Ordinance requirement. These aisles will provide for two-way circulation and 90-degree parking. Additionally, the truck circulation area will be served by a single two-way aisle with a width of 75 feet. Review of the site plan design indicates that the site can sufficiently accommodate, within paved areas, the anticipated tractor trailer activity which is appropriately separated from passenger vehicle circulation and parking.

Parking

The site as proposed provides 180 parking spaces (inclusive of 30 land banked spaces) which is consistent with typical industry standards which provide 1 space per 1,000 SF or 176 spaces for a warehouse of this size.

The site will provide car parking spaces which will have dimensions of 9' by 18' which meets the Ordinance requirements. The proposed trailer storage spaces and loading spaces will both have dimensions of 13' by 60' which are consistent with accepted engineering design standards and will adequately accommodate the proposed design vehicle.

FINDINGS & CONCLUSIONS

Findings

Based upon the detailed analyses as documented herein, the following findings are noted:

- The proposed 175,760 SF warehouse development is projected to generate 33 passenger car entering trips and 10 passenger car exiting trips during the weekday morning peak hour, and 12 passenger car entering trips and 33 passenger car exiting trips during the evening peak hour.
- The proposed 175,760 SF warehouse development is projected to generate 2 truck entering trips and 0 truck exiting trips during the weekday morning peak hour, and 1 truck entering trip and 2 truck exiting trips during the weekday evening peak hour.
- Access to the site is proposed to be provided via one (1) right turn in/right turn out driveway along Route 303 southbound and one (1) full movement driveway along Mountainview Avenue.
- With the addition of site generated traffic, the intersection of Route 303 and Mountainview Avenue is anticipated to continue to operate at overall level of service “F” during both peak hours. Additionally, multiple movements are anticipated to continue to operate at No Build level of service “F” during both peak hours. It should be noted that it is proposed by The Project to improve the eastbound approach in the future and construct a dedicated left turn lane and a shared through/right turn lane which is analyzed in the Build with Mitigation scenario. In the Build with Mitigation scenario, the eastbound and westbound approaches are generally anticipated to decrease delays.
- With the addition of site generated traffic, the intersection of Route 303 and Orangeburg Road (CR 20)/driveway is anticipated to continue to operate at overall No Build levels of service “E” or better during the peak hours studied. Additionally, each movement is anticipated to continue to generally operate at No Build levels of service with little to no changes in delay.
- With the addition of site generated traffic, the intersection of Route 303 and Route 340/South Greenbush Road is anticipated to continue to operate at overall No Build levels of service “E” or better during the peak hours studied. Additionally, each movement is anticipated to continue to operate at No Build levels of service “D” or better with little no changes in delay, with the exception of the westbound and southbound approaches which are anticipated to continue to operate at No Build level of service “F” during the weekday evening peak hour.
- With the addition of site generated traffic, all movements at the intersection of Route 303 and Glenshaw Street are anticipated to operate at No Build levels of service “C” or better during the peak hours studied, with the exception of the eastbound approach which is anticipated to continue to operate at No Build level of service “F” during both peak hours. It should be noted that the intersection’s anticipated 2043 Design Horizon Year traffic volumes do satisfy Warrant 3 – Peak Hour as outlined in the MUTCD. However, as the intersection only meets Warrant 3 during one peak hour, the installation of a traffic signal is not recommended.
- As designed, the individual intersection movements of Route 303 and the site driveway are anticipated to operate at levels of service “B” or better during the peak hours studied.

- As designed, the individual intersection movements of Mountainview Avenue and the site driveway are anticipated to operate at levels of service “B” or better during the peak hours studied.
- As proposed, The Project’s site driveways and internal circulation have been designed to provide for safe and efficient movement of automobiles and large wheel base vehicles.
- The proposed parking supply and design is sufficient to support the projected demand based on typical industry standards.

Conclusions

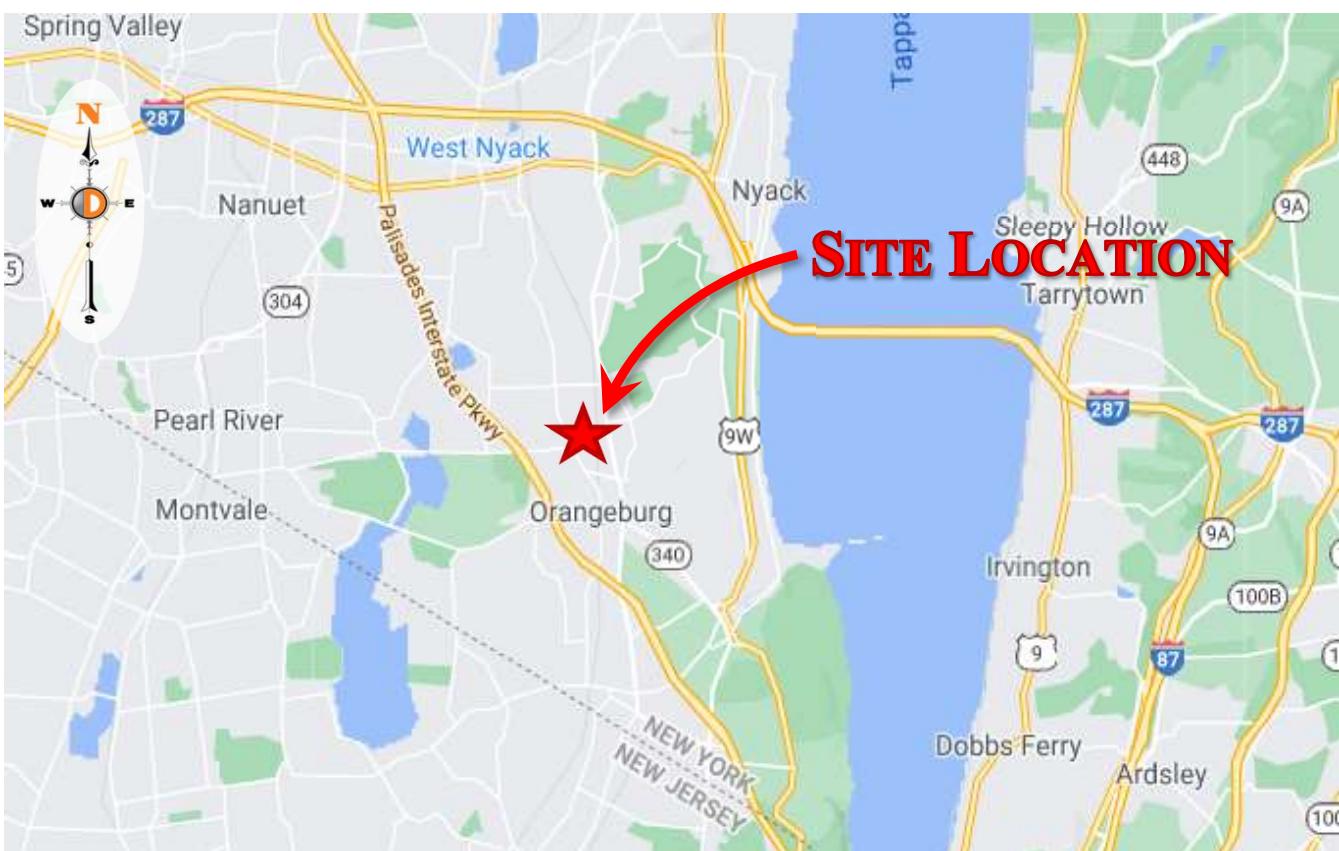
Based upon our Traffic Impact Study as detailed in the body of this report, it is the professional opinion of Dynamic Traffic, LLC that the adjacent street system of the Town of Orangetown and NYSDOT will not experience any significant degradation in operating conditions with the construction of The Project. The site driveway is located to provide safe and efficient access to the adjacent roadway system. The site plan as proposed provides for good circulation throughout the site and provides adequate parking to accommodate The Project’s needs.

Appendix A

Traffic Volume Figures



SITE LOCATION



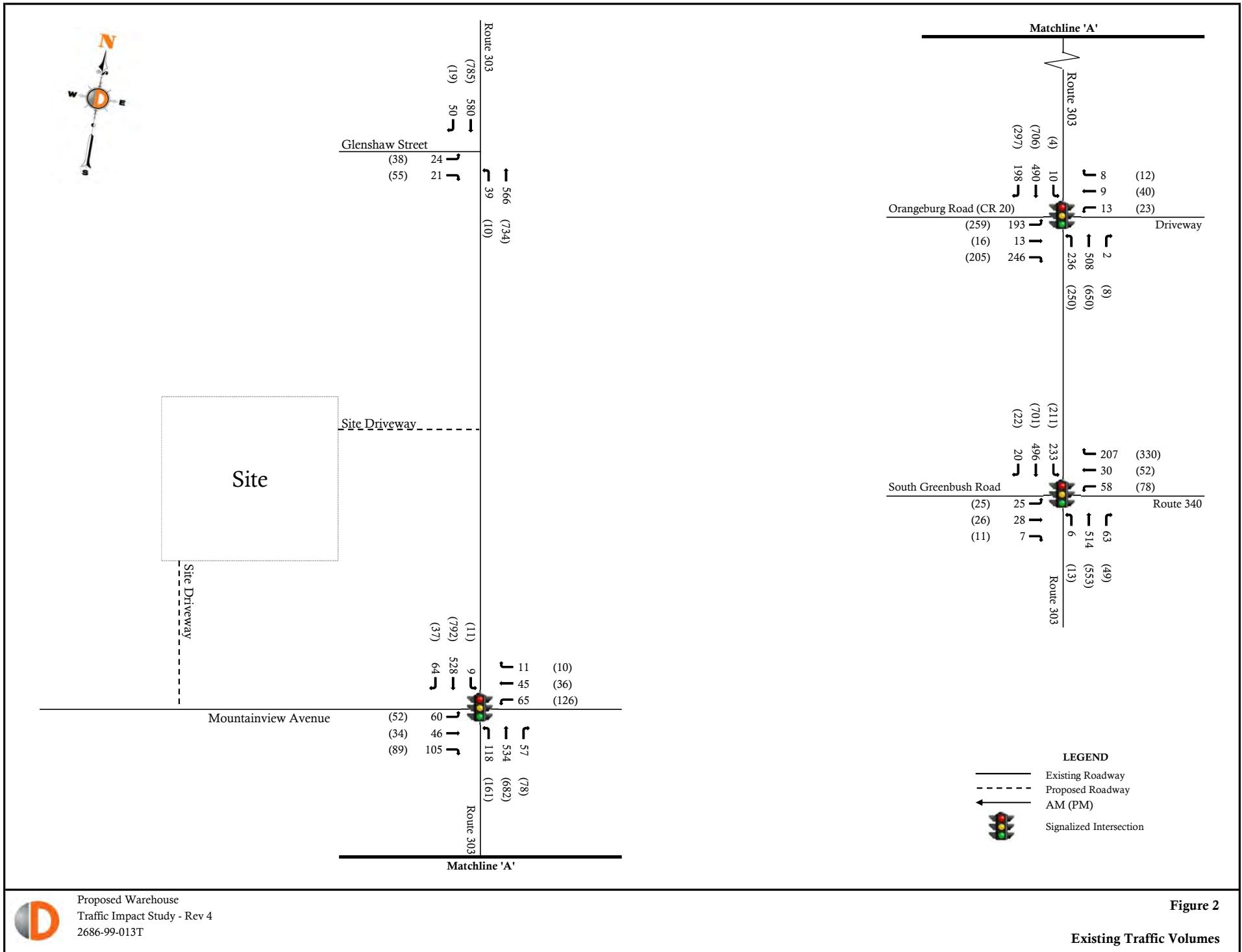
SITE LOCATION

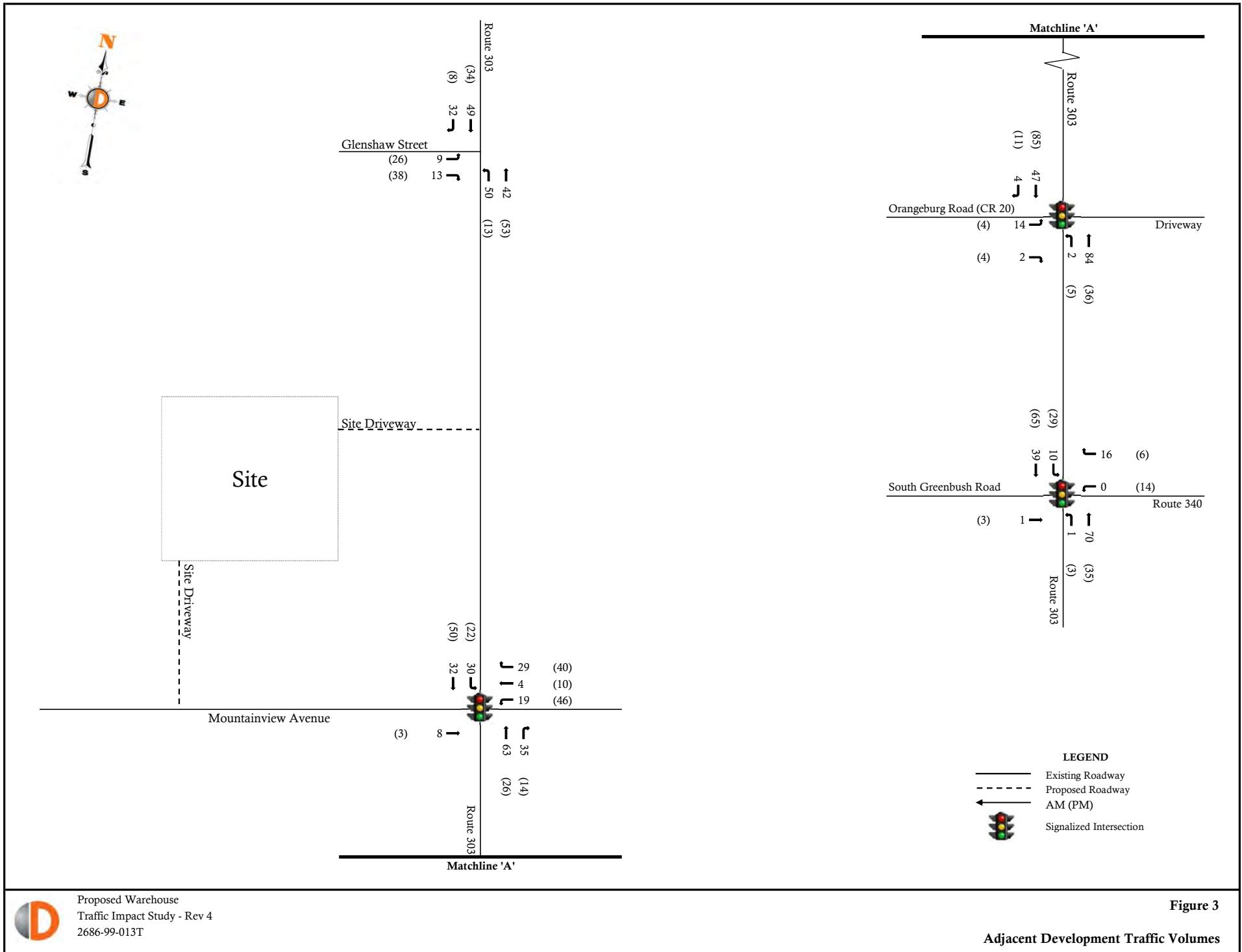


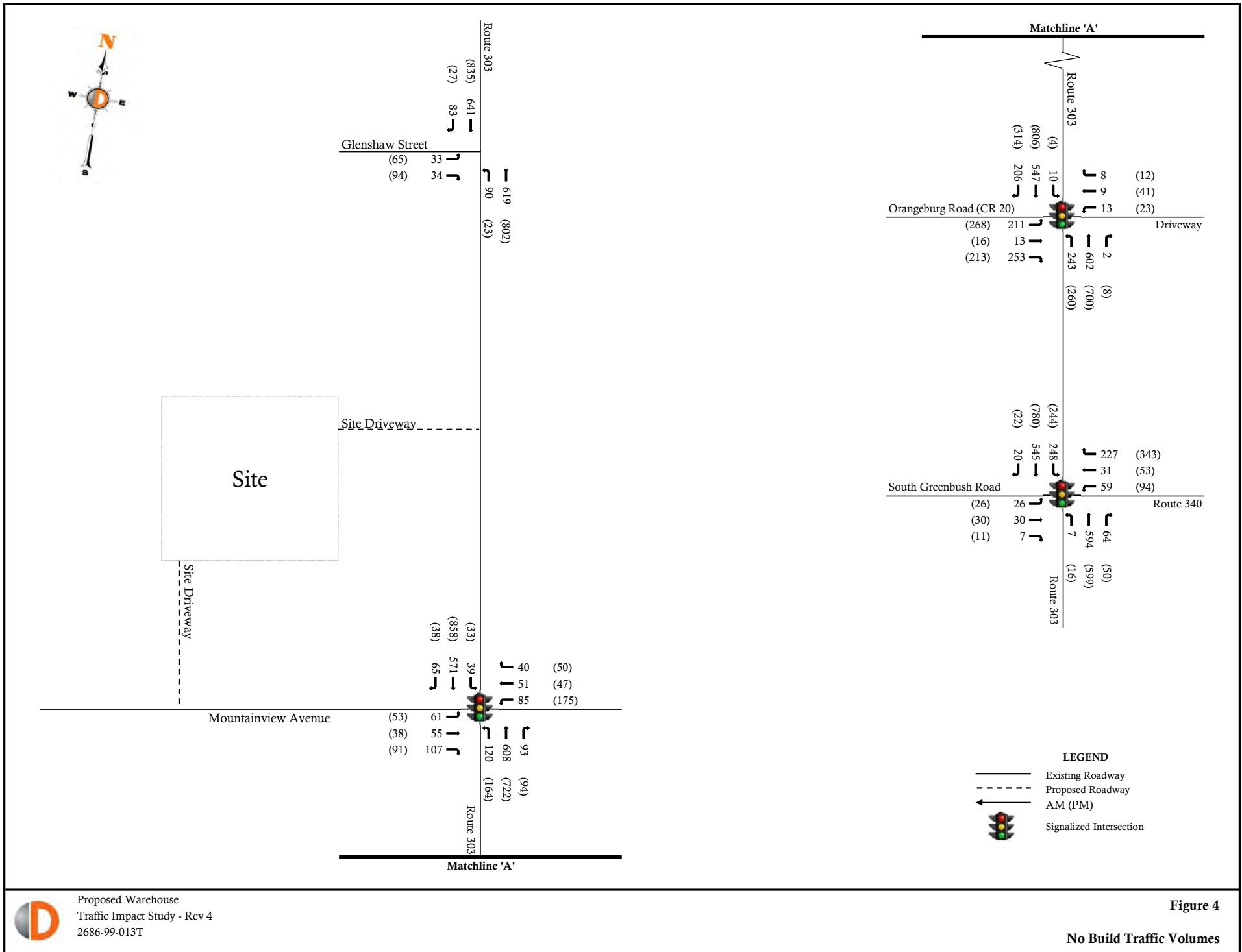
Proposed Warehouse
Traffic Impact Study - Rev 4
2686-99-013T

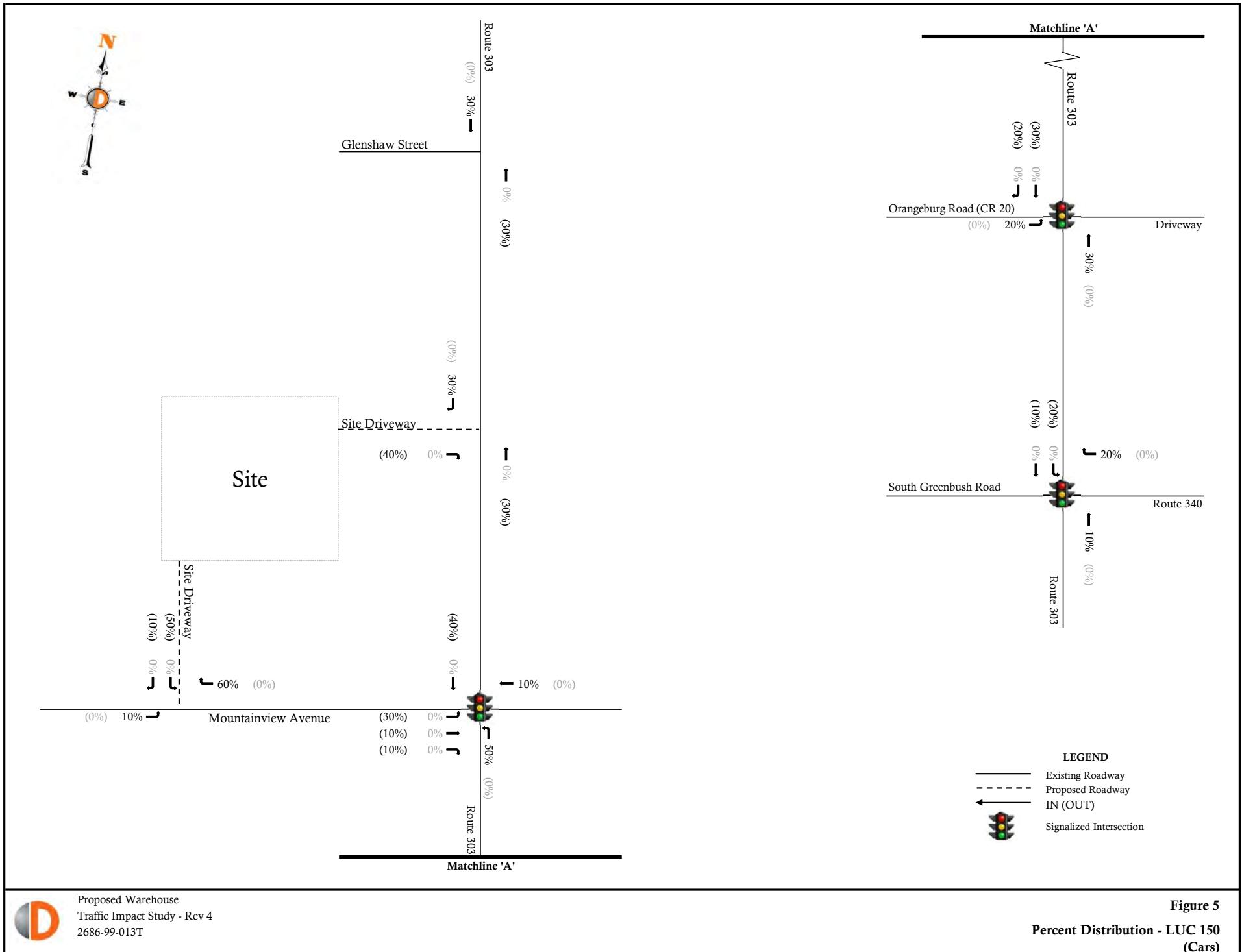
Figure 1

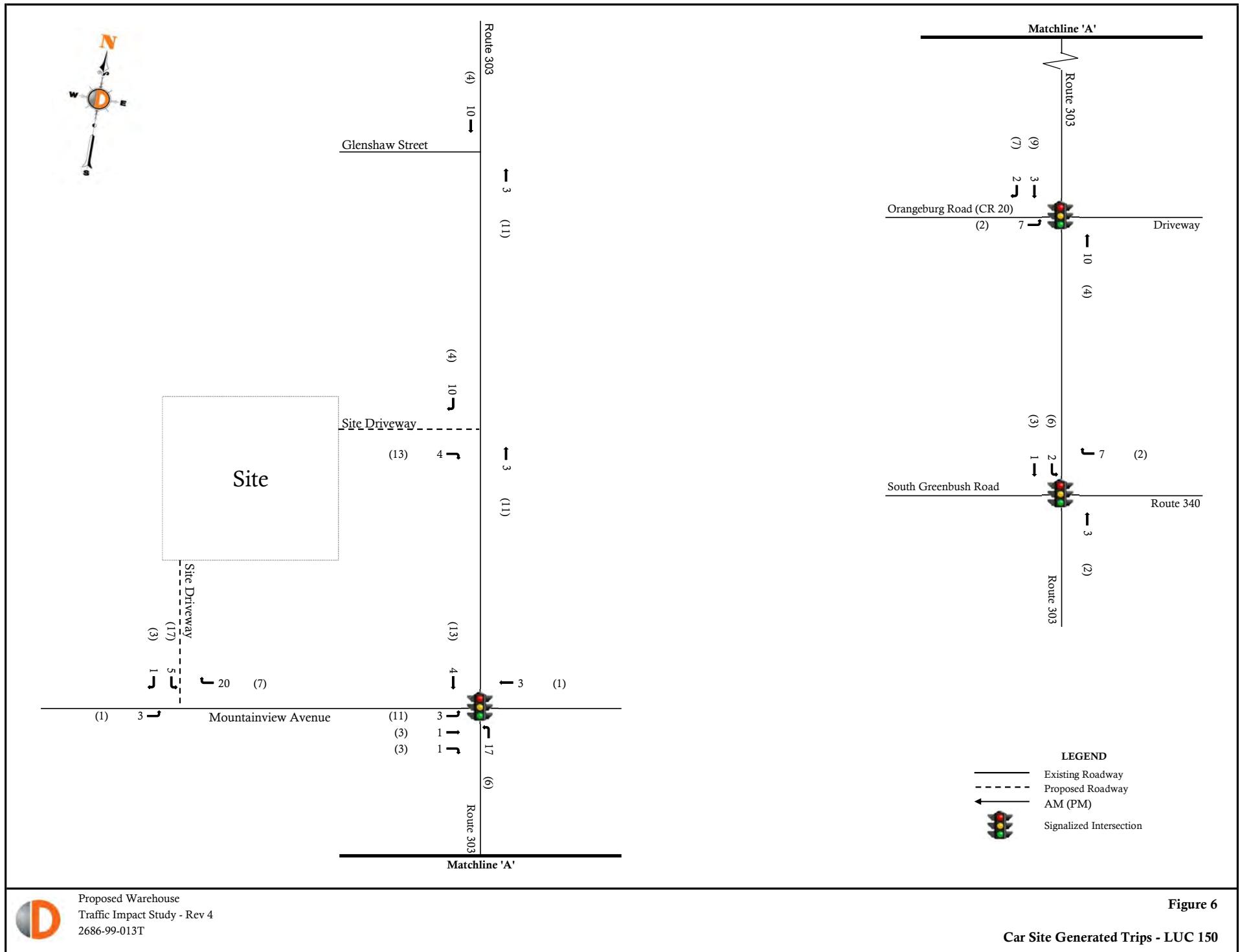
Site Location Map











Proposed Warehouse
Traffic Impact Study - Rev 4
2686-99-013T

Figure 6

Car Site Generated Trips - LUC 150

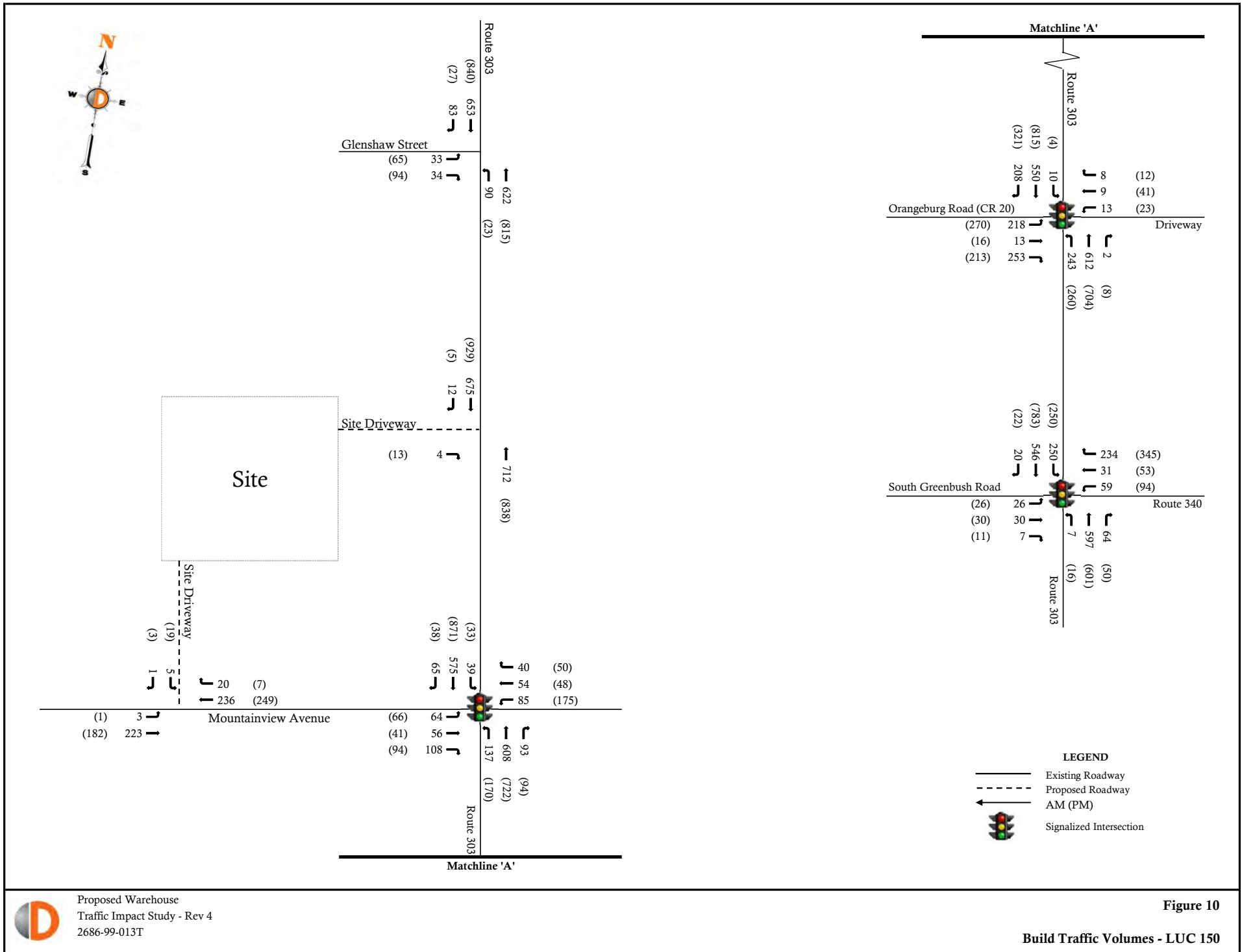


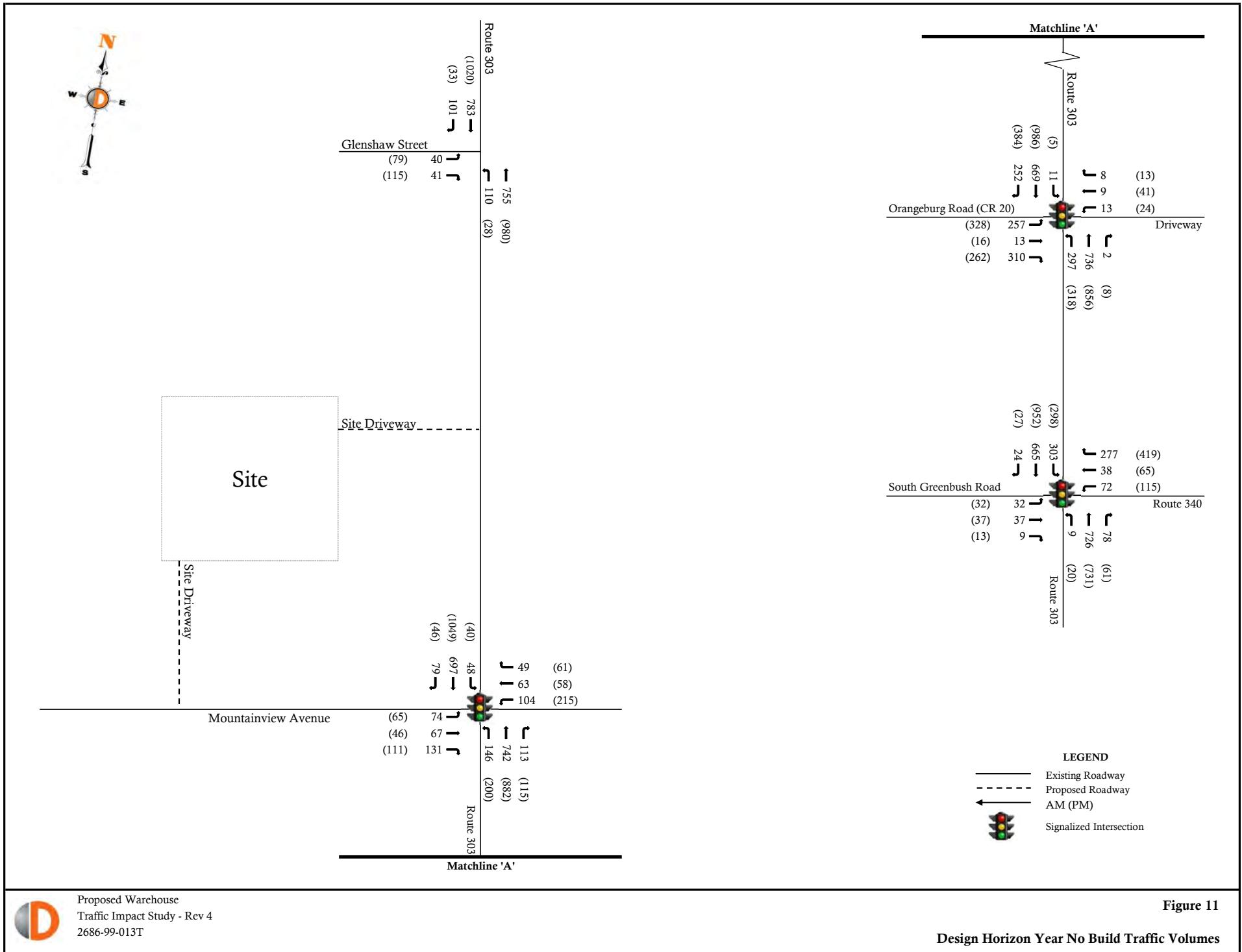


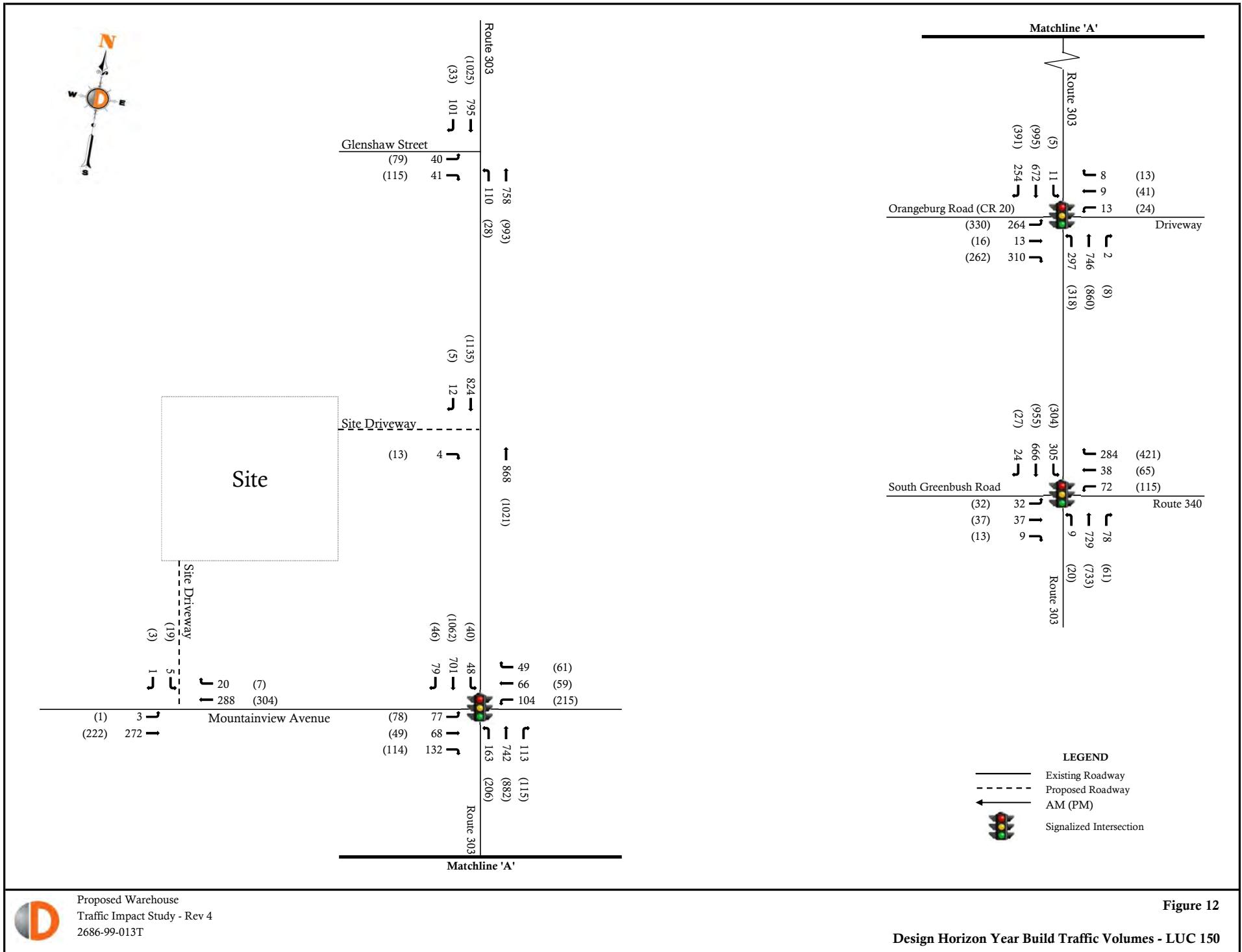
Figure 8

Truck Site Generated Trips - LUC 150









Appendix B

Project Information

National Data & Surveying Services Intersection Turning Movement Count

Location: Route 303 & Mountain View Ave/ Greenbush Rd
City: Orangeburg
Control: Signalized

Project ID: 22-380027-001
Date: 6/2/2022

Data - Total

NS/EW Streets:	Route 303				Route 303				Mountain View Ave/ Greenbush Rd				Mountain View Ave/ Greenbush Rd				TOTAL
	0.5 NL	1.5 NT	0 NR	0 NU	0.5 SL	1.5 ST	0 SR	0 SU	0 EL	1 ET	0 ER	0 EU	0 WL	1 WT	0 WR	0 WU	
AM	NORTHBOUND		SOUTHBOUND		EASTBOUND		WESTBOUND										
7:00 AM	15	77	24	0	2	80	2	0	11	5	11	0	30	9	4	0	270
7:15 AM	12	116	25	0	6	94	3	0	8	9	13	0	29	8	3	0	326
7:30 AM	21	163	20	0	2	100	6	0	13	12	16	0	23	7	6	0	389
7:45 AM	36	162	16	0	2	109	10	0	14	13	27	0	20	10	1	0	420
8:00 AM	28	128	13	0	2	120	20	0	14	16	25	0	12	14	4	0	396
8:15 AM	25	110	10	0	2	107	12	0	22	12	29	0	11	9	3	0	352
8:30 AM	29	134	18	0	2	117	13	0	10	5	24	0	22	12	3	0	389
8:45 AM	20	136	10	0	3	117	11	0	12	5	22	0	19	11	2	0	368
TOTAL VOLUMES : APPROACH %'s :	NL 186	NT 1026	NR 136	NU 0	SL 21	ST 844	SR 77	SU 0	EL 104	ET 77	ER 167	EU 0	WL 166	WT 80	WR 26	WU 0	TOTAL 2910
13.80% 76.11% 10.09% 0.00%	2.23% 89.60% 8.17% 0.00%	29.89% 22.13% 47.99% 0.00%	61.03% 29.41% 9.56% 0.00%														
PEAK HR :	07:45 AM - 08:45 AM																TOTAL
PEAK HR VOL :	118 0.819	534 0.824	57 0.792	0 0.000	8 1.000	453 0.944	55 0.688	0 0.000	60 0.682	46 0.719	105 0.905	0 0.000	65 0.739	45 0.804	11 0.688	0 0.000	1557 0.927
PEAK HR FACTOR :	0.828 0.908				0.837 0.875								0.818 0.872				
PM	NORTHBOUND		SOUTHBOUND		EASTBOUND		WESTBOUND										
4:00 PM	30	165	9	0	2	134	13	0	10	8	23	0	16	5	4	0	419
4:15 PM	31	170	25	0	3	147	3	0	12	9	14	0	20	7	2	0	443
4:30 PM	49	176	21	0	2	172	9	0	8	11	23	0	35	9	1	0	516
4:45 PM	31	139	17	0	3	196	4	0	15	7	24	0	28	8	2	0	474
5:00 PM	37	126	9	0	2	212	18	0	13	4	21	0	33	9	3	0	487
5:15 PM	24	140	7	0	1	174	13	0	9	5	13	0	44	8	2	0	440
5:30 PM	20	143	19	0	2	209	6	0	5	4	21	0	50	14	1	0	494
5:45 PM	25	132	11	0	2	176	8	0	5	4	25	0	36	14	1	0	439
TOTAL VOLUMES : APPROACH %'s :	NL 247	NT 1191	NR 118	NU 0	SL 17	ST 1420	SR 74	SU 0	EL 77	ET 52	ER 164	EU 0	WL 262	WT 74	WR 16	WU 0	TOTAL 3712
15.87% 76.54% 7.58% 0.00%	1.13% 93.98% 4.90% 0.00%	26.28% 17.75% 55.97% 0.00%	74.43% 21.02% 4.55% 0.00%														
PEAK HR :	04:15 PM - 05:15 PM																TOTAL
PEAK HR VOL :	148 0.755	611 0.868	72 0.720	0 0.000	10 0.833	727 0.857	34 0.472	0 0.000	48 0.800	31 0.705	82 0.854	0 0.000	116 0.829	33 0.917	8 0.667	0 0.000	1920 0.930
PEAK HR FACTOR :	0.845 0.831				0.875 0.875								0.872 0.872				

National Data & Surveying Services Intersection Turning Movement Count

Location: Route 303 & Mountain View Ave/ Greenbush Rd
City: Orangeburg
Control: Signalized

Project ID: 22-380027-001
Date: 6/2/2022

Data - Cars

NS/EW Streets:	Route 303				Route 303				Mountain View Ave/ Greenbush Rd				Mountain View Ave/ Greenbush Rd				
	0.5 NL	1.5 NT	0 NR	0 NU	0.5 SL	1.5 ST	0 SR	0 SU	0 EL	1 ET	0 ER	0 EU	0 WL	1 WT	0 WR	0 WU	
AM	NORTHBOUND		SOUTHBOUND		EASTBOUND		WESTBOUND										
7:00 AM	15	71	23	0	2	63	2	0	11	5	10	0	28	7	4	0	241
7:15 AM	9	105	23	0	5	67	3	0	8	9	12	0	22	6	2	0	271
7:30 AM	20	151	20	0	2	84	6	0	13	11	14	0	11	5	2	0	339
7:45 AM	35	136	16	0	1	95	8	0	12	10	26	0	13	8	0	0	360
8:00 AM	25	115	13	0	1	103	19	0	14	5	25	0	12	14	1	0	347
8:15 AM	22	91	9	0	1	91	11	0	16	8	26	0	9	8	0	0	292
8:30 AM	26	115	17	0	1	100	11	0	8	5	23	0	19	6	0	0	331
8:45 AM	19	115	10	0	2	89	9	0	11	4	19	0	13	9	0	0	300
TOTAL VOLUMES : APPROACH %'s :	NL 171 14.24%	NT 899 74.85%	NR 131 10.91%	NU 0 0.00%	SL 15 1.93%	ST 692 89.18%	SR 69 8.89%	SU 0 0.00%	EL 93 30.49%	ET 57 18.69%	ER 155 50.82%	EU 0 0.00%	WL 127 63.82%	WT 63 31.66%	WR 9 4.52%	WU 0 0.00%	TOTAL 2481
PEAK HR :	07:45 AM - 08:45 AM																TOTAL
PEAK HR VOL :	108 0.771	457 0.840	55 0.809	0 0.000	4 1.000	389 0.944	49 0.645	0 0.000	50 0.781	28 0.700	100 0.962	0 0.000	53 0.697	36 0.643	1 0.250	0 0.000	1330 0.924
PEAK HR FACTOR :	0.829				0.898				0.890				0.833				
PM	NORTHBOUND		SOUTHBOUND		EASTBOUND		WESTBOUND										
4:00 PM	28	158	4	0	0	123	13	0	9	7	19	0	15	2	3	0	381
4:15 PM	30	155	18	0	1	130	3	0	12	7	13	0	18	7	1	0	395
4:30 PM	47	163	15	0	1	166	8	0	8	8	23	0	34	8	1	0	482
4:45 PM	30	126	12	0	2	191	4	0	15	7	23	0	27	8	2	0	447
5:00 PM	36	109	9	0	1	202	18	0	13	4	20	0	33	9	3	0	457
5:15 PM	23	135	7	0	0	170	13	0	9	5	13	0	44	7	1	0	427
5:30 PM	20	138	19	0	0	203	6	0	4	4	21	0	49	14	1	0	479
5:45 PM	25	125	11	0	2	167	8	0	5	4	25	0	35	14	1	0	422
TOTAL VOLUMES : APPROACH %'s :	NL 239 16.56%	NT 1109 76.85%	NR 95 6.58%	NU 0 0.00%	SL 7 0.49%	ST 1352 94.41%	SR 73 5.10%	SU 0 0.00%	EL 75 26.98%	ET 46 16.55%	ER 157 56.47%	EU 0 0.00%	WL 255 75.67%	WT 69 20.47%	WR 13 3.86%	WU 0 0.00%	TOTAL 3490
PEAK HR :	04:15 PM - 05:15 PM																TOTAL
PEAK HR VOL :	143 0.761	553 0.848	54 0.750	0 0.000	5 0.625	689 0.853	33 0.458	0 0.000	48 0.800	26 0.813	79 0.859	0 0.000	112 0.824	32 0.889	7 0.583	0 0.000	1781 0.924
PEAK HR FACTOR :	0.833				0.822				0.850				0.839				

National Data & Surveying Services Intersection Turning Movement Count

Location: Route 303 & Mountain View Ave/ Greenbush Rd
City: Orangeburg
Control: Signalized

Project ID: 22-380027-001
Date: 6/2/2022

Data - HT

NS/EW Streets:	Route 303				Route 303				Mountain View Ave/ Greenbush Rd				Mountain View Ave/ Greenbush Rd				
	0.5 NL	1.5 NT	0 NR	0 NU	0.5 SL	1.5 ST	0 SR	0 SU	0 EL	1 ET	0 ER	0 EU	0 WL	1 WT	0 WR	0 WU	TOTAL
AM																	
7:00 AM	0	5	0	0	0	12	0	0	0	0	1	0	1	1	0	20	
7:15 AM	2	9	2	0	1	24	0	0	0	0	1	0	0	0	1	40	
7:30 AM	1	12	0	0	0	13	0	0	0	1	2	0	0	0	0	32	
7:45 AM	0	22	0	0	1	12	1	0	1	0	0	0	6	0	1	45	
8:00 AM	2	12	0	0	1	16	0	0	0	1	0	0	0	0	3	35	
8:15 AM	3	17	1	0	1	15	0	0	0	0	2	0	0	0	3	42	
8:30 AM	3	18	1	0	1	16	2	0	0	0	1	0	1	0	0	43	
8:45 AM	1	20	0	0	1	25	1	0	1	0	2	0	2	1	2	56	
TOTAL VOLUMES : APPROACH %'s :	NL 12 9.16%	NT 115 87.79%	NR 4 3.05%	NU 0 0.00%	SL 6 4.20%	ST 133 93.01%	SR 4 2.80%	SU 0 0.00%	EL 2 14.29%	ET 3 21.43%	ER 9 64.29%	EU 0 0.00%	WL 13 52.00%	WT 2 8.00%	WR 10 40.00%	WU 0 0.00%	TOTAL 313
PEAK HR :	07:45 AM - 08:45 AM																TOTAL
PEAK HR VOL :	8 0.667	69 0.784	2 0.500	0 0.000	4 1.000	59 0.922	3 0.375	0 0.000	1 0.250	2 0.500	3 0.375	0 0.000	7 0.292	0 0.000	7 0.583	0 0.000	165 0.917
PEAK HR FACTOR :	0.898				0.868				0.750				0.500				
PM																	
4:00 PM	0	5	0	0	2	9	0	0	1	0	2	0	0	2	1	0	22
4:15 PM	1	15	2	0	2	14	0	0	0	0	1	0	1	0	1	0	37
4:30 PM	1	11	0	0	1	6	1	0	0	0	0	0	1	0	0	0	21
4:45 PM	1	11	1	0	1	5	0	0	0	0	1	0	0	0	0	0	20
5:00 PM	1	16	0	0	1	9	0	0	0	0	1	0	0	0	0	0	28
5:15 PM	1	5	0	0	1	3	0	0	0	0	0	0	0	1	1	0	12
5:30 PM	0	5	0	0	2	6	0	0	1	0	0	0	1	0	0	0	15
5:45 PM	0	4	0	0	0	9	0	0	0	0	0	0	1	0	0	0	14
TOTAL VOLUMES : APPROACH %'s :	NL 5 6.25%	NT 72 90.00%	NR 3 3.75%	NU 0 0.00%	SL 10 13.89%	ST 61 84.72%	SR 1 1.39%	SU 0 0.00%	EL 2 28.57%	ET 0 0.00%	ER 5 71.43%	EU 0 0.00%	WL 4 40.00%	WT 3 30.00%	WR 3 30.00%	WU 0 0.00%	TOTAL 169
PEAK HR :	04:15 PM - 05:15 PM																TOTAL
PEAK HR VOL :	4 1.000	53 0.828	3 0.375	0 0.000	5 0.625	34 0.607	1 0.250	0 0.000	0 0.000	0 0.000	3 0.750	0 0.000	2 0.500	0 0.000	1 0.250	0 0.000	106 0.716
PEAK HR FACTOR :	0.833				0.625				0.750				0.375				

National Data & Surveying Services Intersection Turning Movement Count

Location: Route 303 & Mountain View Ave/ Greenbush Rd
City: Orangeburg
Control: Signalized

Project ID: 22-380027-001
Date: 6/2/2022

Data - Buses

NS/EW Streets:	Route 303				Route 303				Mountain View Ave/ Greenbush Rd				Mountain View Ave/ Greenbush Rd						
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND						
AM	0.5 NL	1.5 NT	0 NR	0 NU	0.5 SL	1.5 ST	0 SR	0 SU	0 EL	1 ET	0 ER	0 EU	0 WL	1 WT	0 WR	0 WU	TOTAL		
	7:00 AM	0	1	1	0	0	5	0	0	0	0	0	1	1	0	0	9		
	7:15 AM	1	2	0	0	0	3	0	0	0	0	0	7	2	0	0	15		
	7:30 AM	0	0	0	0	0	3	0	0	0	0	0	9	2	4	0	18		
	7:45 AM	1	4	0	0	0	2	1	0	1	2	1	1	2	0	0	15		
	8:00 AM	1	1	0	0	0	1	1	0	0	10	0	0	0	0	0	14		
	8:15 AM	0	2	0	0	0	1	1	0	6	4	1	2	1	0	0	18		
	8:30 AM	0	1	0	0	0	1	0	0	2	0	0	2	6	3	0	15		
	8:45 AM	0	1	0	0	0	3	1	0	0	1	1	4	1	0	0	12		
	TOTAL VOLUMES : APPROACH %'s :	NL 3	NT 12	NR 1	NU 0	SL 0	ST 19	SR 4	SU 0	EL 9	ET 17	ER 3	EU 0	WL 26	WT 15	WR 7	WU 0	TOTAL 116	
PEAK HR :				07:45 AM - 08:45 AM												TOTAL			
PEAK HR VOL :				2				8				0				62			
PEAK HR FACTOR :				0.500				0.500				0.000				0.861			
PM																			
PM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				TOTAL		
	0.5 NL	1.5 NT	0 NR	0 NU	0.5 SL	1.5 ST	0 SR	0 SU	0 EL	1 ET	0 ER	0 EU	0 WL	1 WT	0 WR	0 WU	TOTAL		
	4:00 PM	2	2	5	0	0	2	0	0	1	2	0	1	1	0	0	16		
	4:15 PM	0	0	5	0	0	3	0	0	0	2	0	1	0	0	0	11		
	4:30 PM	1	2	6	0	0	0	0	0	3	0	0	0	1	0	0	13		
	4:45 PM	0	2	4	0	0	0	0	0	0	0	0	1	0	0	0	7		
	5:00 PM	0	1	0	0	0	1	0	0	0	0	0	0	0	0	0	2		
	5:15 PM	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1		
	5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		
	5:45 PM	0	3	0	0	0	0	0	0	0	0	0	0	0	0	0	3		
TOTAL VOLUMES : APPROACH %'s :				NL 3				NT 10				NR 20				NU 0		TOTAL 53	
PEAK HR :				04:15 PM - 05:15 PM												TOTAL 33			
PEAK HR VOL :				1				5				15				0			
PEAK HR FACTOR :				0.250				0.625				0.625				0.635			

National Data & Surveying Services Intersection Turning Movement Count

Location: Route 303 & Mountain View Ave/ Greenbush Rd
City: Orangeburg
Control: Signalized

Project ID: 22-380027-001
Date: 6/2/2022

Data - Bikes

National Data & Surveying Services

Intersection Turning Movement Count

Location: Route 303 & Mountain View Ave/ Greenbush Rd
City: Orangeburg

Project ID: 22-380027-001
Date: 6/2/2022

Data - Pedestrians (Crosswalks)

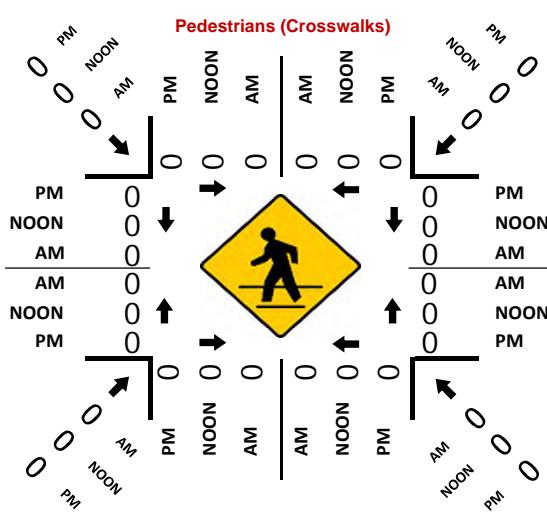
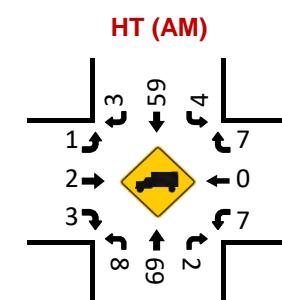
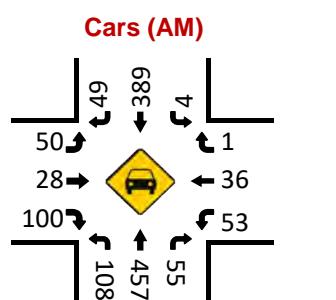
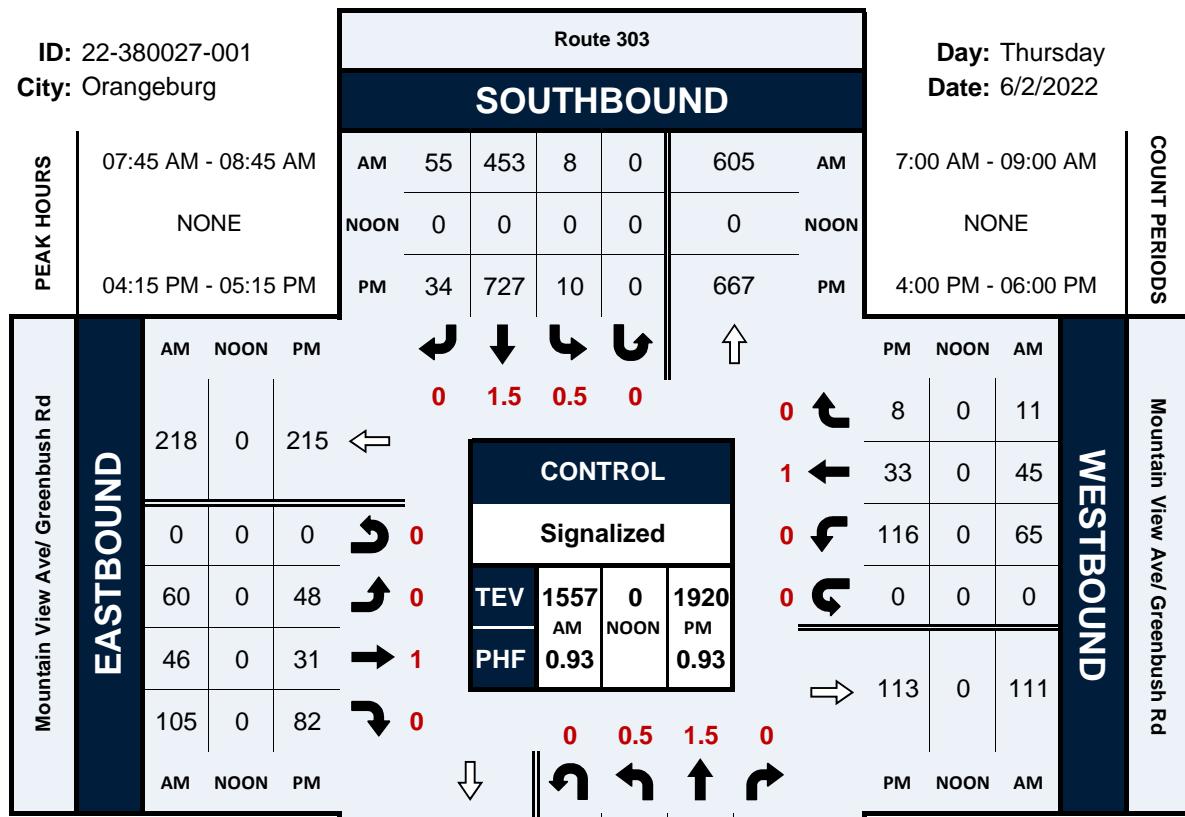
NS/EW Streets:	Route 303		Route 303		Mountain View Ave/ Greenbush Rd		Mountain View Ave/ Greenbush Rd		TOTAL
	NORTH LEG		SOUTH LEG		EAST LEG		WEST LEG		
	EB	WB	EB	WB	NB	SB	NB	SB	
7:00 AM	0	0	0	0	0	0	0	0	0
7:15 AM	0	2	0	0	0	0	0	0	2
7:30 AM	0	0	0	0	0	0	0	0	0
7:45 AM	0	0	0	0	0	0	0	0	0
8:00 AM	0	0	0	0	0	0	0	0	0
8:15 AM	0	0	0	0	0	0	0	0	0
8:30 AM	0	0	0	0	0	0	0	0	0
8:45 AM	0	0	0	0	0	0	0	0	0
TOTAL VOLUMES : APPROACH %'s :	EB 0 0.00%	WB 2 100.00%	EB 0	WB 0	NB 0	SB 0	NB 0	SB 0	TOTAL 2
PEAK HR :	07:45 AM - 08:45 AM								TOTAL
PEAK HR VOL : PEAK HR FACTOR :	0 0		0 0		0 0		0 0		0
PM		NORTH LEG		SOUTH LEG		EAST LEG		WEST LEG	
	EB	WB	EB	WB	NB	SB	NB	SB	TOTAL
4:00 PM	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	0	0	0	0	0	0	0
4:30 PM	0	0	0	0	0	0	0	0	0
4:45 PM	0	0	0	0	0	0	0	0	0
5:00 PM	0	0	0	0	0	0	0	0	0
5:15 PM	0	0	0	0	0	0	0	0	0
5:30 PM	0	0	0	0	0	0	0	0	0
5:45 PM	0	0	0	0	0	0	0	0	0
TOTAL VOLUMES : APPROACH %'s :	EB 0 0	WB 0 0	EB 0	WB 0	NB 0	SB 0	NB 0	SB 0	TOTAL 0
PEAK HR :	04:15 PM - 05:15 PM								TOTAL
PEAK HR VOL : PEAK HR FACTOR :	0 0		0 0		0 0		0 0		0

Route 303 & Mountain View Ave/ Greenbush Rd

Peak Hour Turning Movement Count

ID: 22-380027-001
City: Orangeburg

Day: Thursday
Date: 6/2/2022



National Data & Surveying Services Intersection Turning Movement Count

Location: Route 303 & Orangeburg Rd/Chase Bank Dwy
City: Orangeburg
Control: Signalized

Project ID: 22-380027-002
Date: 6/2/2022

Data - Total

NS/EW Streets:	Route 303				Route 303				Orangeburg Rd/Chase Bank Dwy				Orangeburg Rd/Chase Bank Dwy					
	0 NL	2 NT	0 NR	0 NU	0 SL	2 ST	0 SR	0 SU	0.5 EL	0.5 ET	1 ER	0 EU	0 WL	1 WT	0 WR	0 WU	TOTAL	
AM	NORTHBOUND		SOUTHBOUND		EASTBOUND		WESTBOUND											
7:00 AM	22	83	2	0	2	72	39	0	43	2	44	0	1	3	2	0	315	
7:15 AM	29	109	2	0	2	87	40	0	53	0	41	0	0	3	0	0	366	
7:30 AM	54	140	1	0	2	101	48	0	75	2	44	0	3	0	2	0	472	
7:45 AM	48	152	0	0	1	111	43	0	48	1	58	0	1	3	1	0	467	
8:00 AM	95	127	1	0	3	93	55	0	37	2	63	0	4	3	2	0	485	
8:15 AM	50	92	0	0	2	101	34	0	51	6	71	0	3	1	4	0	415	
8:30 AM	43	134	1	0	2	115	38	0	56	4	54	0	5	2	1	0	455	
8:45 AM	47	119	3	0	1	100	44	0	53	3	59	0	3	3	0	0	435	
TOTAL VOLUMES : APPROACH %'s :	NL 388 28.66%	NT 956 70.61%	NR 10 0.74%	NU 0 0.00%	SL 15 1.32%	ST 780 68.66%	SR 341 30.02%	SU 0 0.00%	EL 416 47.82%	ET 20 2.30%	ER 434 49.89%	EU 0 0.00%	WL 20 40.00%	WT 18 36.00%	WR 12 24.00%	WU 0 0.00%	TOTAL 3410 34.00%	
PEAK HR :	07:30 AM - 08:30 AM																TOTAL	
PEAK HR VOL :	247	511	2	0		8	406	180	0	211	11	236	0	11	7	9	0	1839
PEAK HR FACTOR :	0.650	0.840	0.500	0.000		0.667	0.914	0.818	0.000	0.703	0.458	0.831	0.000	0.688	0.583	0.563	0.000	0.948
PM	NORTHBOUND		SOUTHBOUND		EASTBOUND		WESTBOUND											
4:00 PM	48	141	3	0	0	125	59	0	46	5	64	0	1	6	4	0	502	
4:15 PM	59	155	2	0	1	134	63	0	62	3	49	0	3	7	4	0	542	
4:30 PM	62	157	3	0	0	172	64	0	56	1	51	0	5	10	0	0	581	
4:45 PM	55	122	0	0	1	170	63	0	53	6	42	0	7	5	2	0	526	
5:00 PM	53	122	2	0	2	163	79	0	51	5	46	0	6	15	4	0	548	
5:15 PM	37	112	1	0	2	176	78	0	48	3	38	0	5	13	2	0	515	
5:30 PM	35	123	3	0	2	201	83	0	49	3	44	0	2	3	2	0	550	
5:45 PM	56	120	0	0	1	182	54	0	43	4	54	0	2	2	2	0	520	
TOTAL VOLUMES : APPROACH %'s :	NL 405 27.53%	NT 1052 71.52%	NR 14 0.95%	NU 0 0.00%	SL 9 0.48%	ST 1323 70.56%	SR 543 28.96%	SU 0 0.00%	EL 408 49.39%	ET 30 3.63%	ER 388 46.97%	EU 0 0.00%	WL 31 27.68%	WT 61 54.46%	WR 20 17.86%	WU 0 0.00%	TOTAL 4284 42.84%	
PEAK HR :	04:15 PM - 05:15 PM																TOTAL	
PEAK HR VOL :	229	556	7	0		4	639	269	0	222	15	188	0	21	37	10	0	2197
PEAK HR FACTOR :	0.923	0.885	0.583	0.000		0.500	0.929	0.851	0.000	0.895	0.625	0.922	0.000	0.750	0.617	0.625	0.000	0.945

National Data & Surveying Services Intersection Turning Movement Count

Location: Route 303 & Orangeburg Rd/Chase Bank Dwy
City: Orangeburg
Control: Signalized

Project ID: 22-380027-002
Date: 6/2/2022

Data - Cars

NS/EW Streets:	Route 303				Route 303				Orangeburg Rd/Chase Bank Dwy				Orangeburg Rd/Chase Bank Dwy				
	0 NL	2 NT	0 NR	0 NU	0 SL	2 ST	0 SR	0 SU	0.5 EL	0.5 ET	1 ER	0 EU	0 WL	1 WT	0 WR	0 WU	TOTAL
AM	NORTHBOUND		SOUTHBOUND		EASTBOUND		WESTBOUND										
7:00 AM	15 NL	77 NT	2 NR	0 NU	2 SL	58 ST	34 SR	0 SU	42 EL	2 ET	39 ER	0 EU	1 WL	3 WT	2 WR	0 WU	277
7:15 AM	22	97	2	0	2	59	34	0	50	0	35	0	1	0	3	0	304
7:30 AM	49	124	1	0	2	85	36	0	72	2	41	0	3	0	2	0	417
7:45 AM	41	132	0	0	1	96	34	0	47	1	53	0	1	3	1	0	410
8:00 AM	84	113	1	0	1	83	49	0	36	2	55	0	4	3	2	0	433
8:15 AM	48	73	0	0	1	90	27	0	48	6	67	0	3	1	4	0	368
8:30 AM	40	118	1	0	2	99	33	0	48	4	48	0	5	2	1	0	401
8:45 AM	41	98	3	0	0	81	33	0	50	2	51	0	3	3	0	0	365
TOTAL VOLUMES : APPROACH %'s :	NL 340 28.76%	NT 832 70.39%	NR 10 0.85%	NU 0 0.00%	SL 11 1.17%	ST 651 69.11%	SR 280 29.72%	SU 0 0.00%	EL 393 49.06%	ET 19 2.37%	ER 389 48.56%	EU 0 0.00%	WL 20 40.00%	WT 18 36.00%	WR 12 24.00%	WU 0 0.00%	TOTAL 2975
PEAK HR :	07:30 AM - 08:30 AM																TOTAL
PEAK HR VOL :	222 0.661	442 0.837	2 0.500	0 0.000	5 0.625	354 0.922	146 0.745	0 0.000	203 0.705	11 0.458	216 0.806	0 0.000	11 0.688	7 0.583	9 0.563	0 0.000	1628 0.940
PEAK HR FACTOR :	0.841				0.949				0.888				0.750				
PM	NORTHBOUND		SOUTHBOUND		EASTBOUND		WESTBOUND										
4:00 PM	45 NL	131 NT	3 NR	0 NU	0 SL	112 ST	57 SR	0 SU	44 0.5 EL	5 ET	56 ER	0 EU	1 WL	1 WT	6 WR	4 WU	464
4:15 PM	58	143	2	0	1	117	61	0	54	3	46	0	3	7	4	0	499
4:30 PM	61	146	3	0	0	169	63	0	45	1	47	0	5	10	0	0	550
4:45 PM	53	110	0	0	1	166	61	0	45	6	41	0	7	5	2	0	497
5:00 PM	52	112	2	0	2	153	79	0	48	5	46	0	6	15	4	0	524
5:15 PM	36	109	1	0	2	173	77	0	45	3	38	0	5	13	2	0	504
5:30 PM	31	121	3	0	2	195	82	0	46	3	41	0	2	3	2	0	531
5:45 PM	54	115	0	0	1	174	51	0	41	4	52	0	2	2	2	0	498
TOTAL VOLUMES : APPROACH %'s :	NL 390 28.04%	NT 987 70.96%	NR 14 1.01%	NU 0 0.00%	SL 9 0.50%	ST 1259 69.98%	SR 531 29.52%	SU 0 0.00%	EL 368 48.10%	ET 30 3.92%	ER 367 47.97%	EU 0 0.00%	WL 31 27.68%	WT 61 54.46%	WR 20 17.86%	WU 0 0.00%	TOTAL 4067
PEAK HR :	04:15 PM - 05:15 PM																TOTAL
PEAK HR VOL :	224 0.918	511 0.875	7 0.583	0 0.000	4 0.500	605 0.895	264 0.835	0 0.000	192 0.889	15 0.625	180 0.957	0 0.000	21 0.750	37 0.617	10 0.625	0 0.000	2070 0.941
PEAK HR FACTOR :	0.883				0.933				0.939				0.680				

National Data & Surveying Services Intersection Turning Movement Count

Location: Route 303 & Orangeburg Rd/Chase Bank Dwy
City: Orangeburg
Control: Signalized

Project ID: 22-380027-002
Date: 6/2/2022

Data - HT

NS/EW Streets:	Route 303				Route 303				Orangeburg Rd/Chase Bank Dwy				Orangeburg Rd/Chase Bank Dwy				
	0 NL	2 NT	0 NR	0 NU	0 SL	2 ST	0 SR	0 SU	0.5 EL	0.5 ET	1 ER	0 EU	0 WL	1 WT	0 WR	0 WU	TOTAL
AM	0 NL	2 NT	0 NR	0 NU	0 SL	2 ST	0 SR	0 SU	0.5 EL	0.5 ET	1 ER	0 EU	0 WL	1 WT	0 WR	0 WU	28
7:00 AM	7	5	0	0	0	9	4	0	0	0	3	0	0	0	0	0	50
7:15 AM	7	10	0	0	0	22	4	0	2	0	5	0	0	0	0	0	43
7:30 AM	5	16	0	0	0	9	7	0	3	0	3	0	0	0	0	0	43
7:45 AM	2	15	0	0	0	13	7	0	1	0	5	0	0	0	0	0	43
8:00 AM	7	12	0	0	2	10	5	0	1	0	5	0	0	0	0	0	42
8:15 AM	2	18	0	0	1	9	6	0	1	0	3	0	0	0	0	0	40
8:30 AM	2	15	0	0	0	13	4	0	8	0	5	0	0	0	0	0	47
8:45 AM	5	20	0	0	1	14	9	0	3	1	7	0	0	0	0	0	60
TOTAL VOLUMES : APPROACH %'s :	NL 37 25.00%	NT 111 75.00%	NR 0 0.00%	NU 0 0.00%	SL 4 2.68%	ST 99 66.44%	SR 46 30.87%	SU 0 0.00%	EL 19 33.93%	ET 1 1.79%	ER 36 64.29%	EU 0 0.00%	WL 0 0	WT 0 0	WR 0 0	WU 0 0	TOTAL 353
PEAK HR :	07:30 AM - 08:30 AM																TOTAL
PEAK HR VOL :	16 0.571	61 0.847	0 0.000	0 0.000	3 0.375	41 0.788	25 0.893	0 0.000	6 0.500	0 0.000	16 0.800	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	TOTAL 168 0.977
PEAK HR FACTOR :	0.917				0.863				0.917								
PM	0 NL	2 NT	0 NR	0 NU	0 SL	2 ST	0 SR	0 SU	0.5 EL	0.5 ET	1 ER	0 EU	0 WL	1 WT	0 WR	0 WU	TOTAL
4:00 PM	3	5	0	0	0	10	2	0	1	0	8	0	0	0	0	0	29
4:15 PM	1	10	0	0	0	15	0	0	6	0	1	0	0	0	0	0	33
4:30 PM	1	6	0	0	0	3	1	0	7	0	4	0	0	0	0	0	22
4:45 PM	2	7	0	0	0	4	1	0	8	0	1	0	0	0	0	0	23
5:00 PM	1	9	0	0	0	10	0	0	3	0	0	0	0	0	0	0	23
5:15 PM	1	3	0	0	0	3	0	0	3	0	0	0	0	0	0	0	10
5:30 PM	4	2	0	0	0	5	1	0	3	0	2	0	0	0	0	0	17
5:45 PM	2	3	0	0	0	8	3	0	1	0	2	0	0	0	0	0	19
TOTAL VOLUMES : APPROACH %'s :	NL 15 25.00%	NT 45 75.00%	NR 0 0.00%	NU 0 0.00%	SL 0 0.00%	ST 58 87.88%	SR 8 12.12%	SU 0 0.00%	EL 32 64.00%	ET 0 0.00%	ER 18 36.00%	EU 0 0.00%	WL 0 0	WT 0 0	WR 0 0	WU 0 0	TOTAL 176
PEAK HR :	04:15 PM - 05:15 PM																TOTAL
PEAK HR VOL :	5 0.625	32 0.800	0 0.000	0 0.000	0 0.000	32 0.533	2 0.500	0 0.000	24 0.750	0 0.000	6 0.375	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	TOTAL 101 0.765
PEAK HR FACTOR :	0.841				0.567				0.682								

National Data & Surveying Services Intersection Turning Movement Count

Location: Route 303 & Orangeburg Rd/Chase Bank Dwy
City: Orangeburg
Control: Signalized

Project ID: 22-380027-002
Date: 6/2/2022

Data - Buses

NS/EW Streets:	Route 303				Route 303				Orangeburg Rd/Chase Bank Dwy				Orangeburg Rd/Chase Bank Dwy					
	NORTHBOUND		SOUTHBOUND		EASTBOUND		WESTBOUND		WL		WT		WR		WU			
AM	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	0	1	0	0	TOTAL	
7:00 AM	0	1	0	0	0	5	1	0	1	0	2	0	0	0	0	0	10	
7:15 AM	0	2	0	0	0	6	2	0	1	0	1	0	0	0	0	0	12	
7:30 AM	0	0	0	0	0	7	5	0	0	0	0	0	0	0	0	0	12	
7:45 AM	5	5	0	0	0	2	0	0	0	0	0	0	0	0	0	0	14	
8:00 AM	4	2	0	0	0	0	1	0	0	0	3	0	0	0	0	0	10	
8:15 AM	0	1	0	0	0	2	1	0	2	0	1	0	0	0	0	0	7	
8:30 AM	1	1	0	0	0	3	1	0	0	0	1	0	0	0	0	0	7	
8:45 AM	1	1	0	0	0	5	2	0	0	0	1	0	0	0	0	0	10	
TOTAL VOLUMES : APPROACH %'s :	NL 11 45.83%	NT 13 54.17%	NR 0 0.00%	NU 0 0.00%	SL 0 0.00%	ST 30 66.67%	SR 15 33.33%	SU 0 0.00%	EL 4 30.77%	ET 0 0.00%	ER 9 69.23%	EU 0 0.00%	WL 0 0	WT 0 0	WR 0 0	WU 0 0	TOTAL 82	
PEAK HR :	07:30 AM - 08:30 AM																	TOTAL
PEAK HR VOL :	9 0.450	8 0.400	0 0.000	0 0.000	0 0.425	11 0.393	9 0.450	0 0.000	2 0.250	0 0.000	4 0.333	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	43 0.768	
PM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND					
	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	0	1	0	0	TOTAL	
4:00 PM	0	5	0	0	0	3	0	0	1	0	0	0	0	0	0	0	9	
4:15 PM	0	2	0	0	0	2	2	0	2	0	2	0	0	0	0	0	10	
4:30 PM	0	5	0	0	0	0	0	0	4	0	0	0	0	0	0	0	9	
4:45 PM	0	5	0	0	0	0	0	1	0	0	0	0	0	0	0	0	6	
5:00 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	
5:15 PM	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1	
5:30 PM	0	0	0	0	0	1	0	0	0	0	1	0	0	0	0	0	2	
5:45 PM	0	2	0	0	0	0	0	0	1	0	0	0	0	0	0	0	3	
TOTAL VOLUMES : APPROACH %'s :	NL 0 0.00%	NT 20 100.00%	NR 0 0.00%	NU 0 0.00%	SL 0 0.00%	ST 6 60.00%	SR 4 40.00%	SU 0 0.00%	EL 8 72.73%	ET 0 0.00%	ER 3 27.27%	EU 0 0.00%	WL 0 0	WT 0 0	WR 0 0	WU 0 0	TOTAL 41	
PEAK HR :	04:15 PM - 05:15 PM																	TOTAL
PEAK HR VOL :	0 0.000	13 0.650	0 0.000	0 0.000	0 0.650	2 0.250	3 0.375	0 0.000	6 0.375	0 0.000	2 0.250	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	26 0.650	
PEAK HR FACTOR :																		

National Data & Surveying Services Intersection Turning Movement Count

Location: Route 303 & Orangeburg Rd/Chase Bank Dwy
City: Orangeburg
Control: Signalized

Project ID: 22-380027-002
Date: 6/2/2022

Data - Bikes

National Data & Surveying Services

Intersection Turning Movement Count

Location: Route 303 & Orangeburg Rd/Chase Bank Dwy
City: Orangeburg

Project ID: 22-380027-002
Date: 6/2/2022

Data - Pedestrians (Crosswalks)

NS/EW Streets:	Route 303		Route 303		Orangeburg Rd/Chase Bank Dwy		Orangeburg Rd/Chase Bank Dwy		TOTAL
	NORTH LEG		SOUTH LEG		EAST LEG		WEST LEG		
	EB	WB	EB	WB	NB	SB	NB	SB	
7:00 AM	0	0	0	0	0	0	0	0	0
7:15 AM	0	0	0	0	0	0	0	0	0
7:30 AM	0	0	0	0	0	0	0	0	0
7:45 AM	0	0	0	0	0	0	0	0	0
8:00 AM	0	0	0	0	2	1	0	0	3
8:15 AM	0	0	0	0	0	1	0	0	1
8:30 AM	0	0	0	1	0	0	0	0	1
8:45 AM	0	0	0	0	0	0	0	0	0
TOTAL VOLUMES : APPROACH %'s :	EB 0	WB 0	EB 0	WB 1 0.00%	NB 2 50.00%	SB 2 50.00%	NB 0	SB 0	TOTAL 5
PEAK HR :	07:30 AM - 08:30 AM								TOTAL
PEAK HR VOL :	0		0		2 0.250 0.333		2 0.500		4 0.333
PEAK HR FACTOR :									

PM	NORTH LEG		SOUTH LEG		EAST LEG		WEST LEG		TOTAL
	EB	WB	EB	WB	NB	SB	NB	SB	
4:00 PM	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	0	0	0	0	0	0	0
4:30 PM	1	1	0	0	0	0	0	0	2
4:45 PM	0	0	0	0	0	0	0	0	0
5:00 PM	0	0	0	0	0	0	0	0	0
5:15 PM	0	0	0	0	0	0	0	0	0
5:30 PM	0	0	0	0	0	0	0	0	0
5:45 PM	0	0	0	0	0	0	0	0	0
TOTAL VOLUMES : APPROACH %'s :	EB 1 50.00%	WB 1 50.00%	EB 0	WB 0	NB 0	SB 0	NB 0	SB 0	TOTAL 2
PEAK HR :	04:15 PM - 05:15 PM								TOTAL
PEAK HR VOL :	1 0.250 0.250		0 0.250 0.250		0 0		0 0		2 0.250
PEAK HR FACTOR :									

Route 303 & Orangeburg Rd/Chase Bank Dwy

Peak Hour Turning Movement Count

ID: 22-380027-002

City: Orangeburg

PEAK HOURS	07:30 AM - 08:30 AM				04:15 PM - 05:15 PM			
	NONE				0			
	0				0			
	AM	180	406	8	0	731	AM	
	NOON	0	0	0	0	0	NOON	
	PM	269	639	4	0	788	PM	

Route 303 SOUTHBOUND

Route 303

SOUTHBOUND

Day: Thursday

Date: 6/2/2022

PEAK HOURS	07:30 AM - 08:30 AM			04:15 PM - 05:15 PM			PEAK HOURS
	AM	NOON	PM	AM	NOON	PM	
	434	0	535	0	0	0	
	0	0	0	0	0	0	
	211	0	222	0	0.5	0.5	
	11	0	15	0	0.5	0.5	
	236	0	188	0	0	1	
	AM	NOON	PM	AM	NOON	PM	

Orangeburg Rd/Chase Bank Dwy

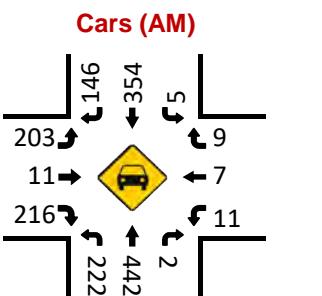
EASTBOUND

CONTROL			
Signalized			
TEV	1839	0	2197
PHF	0.95	AM	0.95

Orangeburg Rd/Chase Bank Dwy

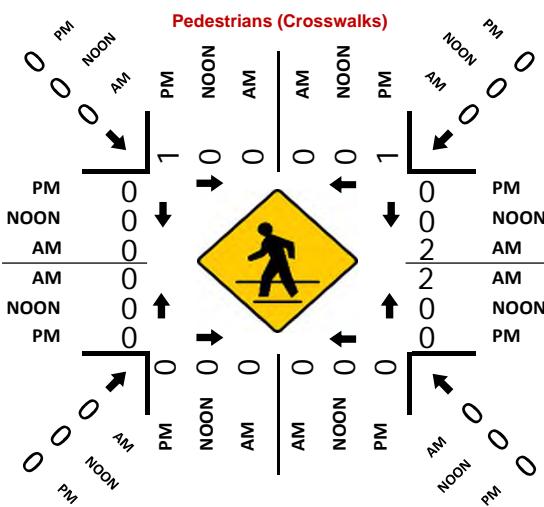
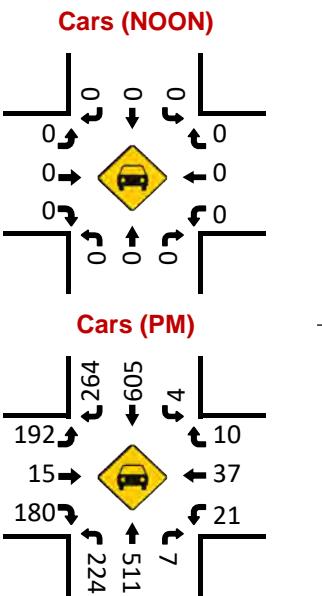
WESTBOUND

COUNT PERIODS	7:00 AM - 09:00 AM			4:00 PM - 06:00 PM			COUNT PERIODS
	PM	NOON	AM	PM	NOON	AM	
	0	10	9	0	0	7	
	1	37	0	0	21	11	
	0	21	0	0	0	0	
	0	0	0	26	0	21	
	PM	NOON	AM	PM	NOON	AM	



NORTHBOUND

Route 303



National Data & Surveying Services Intersection Turning Movement Count

Location: Route 303 & Greenbush/Rte 340
City: Orangeburg
Control: Signalized

Project ID: 22-380027-003
Date: 6/2/2022

Data - Total

NS/EW Streets:	Route 303				Route 303				Greenbush/Rte 340				Greenbush/Rte 340				TOTAL
	0 NL	2 NT	0 NR	0 NU	0.5 SL	1.5 ST	0 SR	0 SU	0 EL	1 ET	0 ER	0 EU	0 WL	1 WT	0 WR	0 WU	
AM	NORTHBOUND		SOUTHBOUND		EASTBOUND		WESTBOUND										
7:00 AM	2	89	9	0	32	75	7	0	0	7	1	0	11	1	20	0	254
7:15 AM	0	119	12	0	34	80	10	0	2	8	1	0	5	2	23	0	296
7:30 AM	1	158	9	0	39	87	9	0	5	5	5	0	7	3	37	0	365
7:45 AM	0	135	19	0	53	115	6	0	6	7	1	0	14	9	46	0	411
8:00 AM	1	144	11	0	49	103	2	0	8	4	1	0	14	9	69	0	415
8:15 AM	3	94	16	0	54	105	4	0	5	9	1	0	7	3	41	0	342
8:30 AM	2	124	17	0	50	116	6	0	5	8	4	0	23	9	44	0	408
8:45 AM	2	115	18	0	49	101	8	0	4	8	1	0	18	4	44	0	372
TOTAL VOLUMES : APPROACH %'s :	NL 11 1.00%	NT 978 88.91%	NR 111 10.09%	NU 0 0.00%	SL 360 30.15%	ST 782 65.49%	SR 52 4.36%	SU 0 0.00%	EL 35 33.02%	ET 56 52.83%	ER 15 14.15%	EU 0 0.00%	WL 99 21.38%	WT 40 8.64%	WR 324 69.98%	WU 0 0.00%	TOTAL 2863
PEAK HR :	07:45 AM - 08:45 AM																TOTAL
PEAK HR VOL :	6	497	63	0	206	439	18	0	24	28	7	0	58	30	200	0	1576
PEAK HR FACTOR :	0.500	0.863	0.829	0.000	0.954	0.946	0.750	0.000	0.750	0.778	0.438	0.000	0.630	0.833	0.725	0.000	0.949
PM	NORTHBOUND		SOUTHBOUND		EASTBOUND		WESTBOUND										
4:00 PM	3	132	20	0	40	137	14	0	9	10	3	0	14	10	59	0	451
4:15 PM	1	143	10	0	49	132	4	0	7	7	2	0	22	11	66	0	454
4:30 PM	6	141	17	0	54	161	7	0	3	3	3	0	14	18	80	0	507
4:45 PM	2	111	10	0	42	172	6	0	7	9	1	0	11	8	61	0	440
5:00 PM	3	85	8	0	46	169	3	1	5	5	4	0	25	11	77	0	442
5:15 PM	2	98	20	0	43	161	9	0	10	9	3	0	20	7	47	0	429
5:30 PM	2	96	15	0	49	191	6	0	4	6	1	0	17	5	55	0	447
5:45 PM	4	110	21	0	58	170	14	0	11	11	8	0	22	10	53	0	492
TOTAL VOLUMES : APPROACH %'s :	NL 23 2.17%	NT 916 86.42%	NR 121 11.42%	NU 0 0.00%	SL 381 21.92%	ST 1293 74.40%	SR 63 3.62%	SU 1 0.06%	EL 56 39.72%	ET 60 42.55%	ER 25 17.73%	EU 0 0.00%	WL 145 20.06%	WT 80 11.07%	WR 498 68.88%	WU 0 0.00%	TOTAL 3662
PEAK HR :	04:00 PM - 05:00 PM																TOTAL
PEAK HR VOL :	12	527	57	0	185	602	31	0	26	29	9	0	61	47	266	0	1852
PEAK HR FACTOR :	0.500	0.921	0.713	0.000	0.856	0.875	0.554	0.000	0.722	0.725	0.750	0.000	0.693	0.653	0.831	0.000	0.913
							0.921								0.835		

National Data & Surveying Services Intersection Turning Movement Count

Location: Route 303 & Greenbush/Rte 340
City: Orangeburg
Control: Signalized

Project ID: 22-380027-003
Date: 6/2/2022

Data - HT

NS/EW Streets:	Route 303				Route 303				Greenbush/Rte 340				Greenbush/Rte 340					
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND					
AM	0 NL	2 NT	0 NR	0 NU	0.5 SL	1.5 ST	0 SR	0 SU	0 EL	1 ET	0 ER	0 EU	0 WL	1 WT	0 WR	0 WU	TOTAL	
7:00 AM	0	11	0	0	3	8	0	0	0	0	0	0	2	0	1	0	25	
7:15 AM	0	15	1	0	5	20	0	0	0	0	0	0	1	0	6	0	48	
7:30 AM	1	15	3	0	3	10	0	0	0	0	0	0	0	0	3	0	35	
7:45 AM	0	16	1	0	5	13	0	0	0	0	0	0	3	0	1	0	41	
8:00 AM	1	13	0	0	4	10	0	0	2	0	0	0	2	0	2	0	34	
8:15 AM	0	17	3	0	2	9	3	0	0	1	0	0	0	0	2	0	37	
8:30 AM	0	10	2	0	5	13	0	0	1	1	0	0	2	1	8	0	43	
8:45 AM	0	19	0	0	6	16	2	0	1	2	0	0	1	0	3	0	50	
TOTAL VOLUMES :	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL	
APPROACH %'s :	2 1.56%	116 90.63%	10 7.81%	0 0.00%	33 24.09%	99 72.26%	5 3.65%	0 0.00%	6 60.00%	4 40.00%	0 0.00%	0 0.00%	11 28.95%	1 2.63%	26 68.42%	0 0.00%	313 TOTAL	
PEAK HR :	07:45 AM - 08:45 AM																TOTAL	
PEAK HR VOL :	1 0.250	56 0.824	6 0.500	0 0.000	16 0.800	45 0.865	3 0.250	0 0.000	5 0.625	2 0.500	0 0.000	0 0.000	7 0.583	1 0.250	13 0.406	0 0.000	155 0.901	
PEAK HR FACTOR :	0.788				0.889				0.875				0.477					
PM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND					
	0 NL	2 NT	0 NR	0 NU	0.5 SL	1.5 ST	0 SR	0 SU	0 EL	1 ET	0 ER	0 EU	0 WL	1 WT	0 WR	0 WU	TOTAL	
4:00 PM	0	4	0	0	2	12	2	0	0	1	0	0	0	0	0	4	0	25
4:15 PM	0	4	0	0	4	10	1	0	0	0	0	0	0	0	7	0	26	
4:30 PM	0	5	1	0	2	6	0	0	0	0	0	0	0	2	3	0	19	
4:45 PM	0	6	0	0	0	3	3	0	0	0	0	0	0	1	1	0	14	
5:00 PM	0	5	0	0	1	9	0	0	1	0	0	0	1	0	3	0	20	
5:15 PM	0	6	1	0	0	1	1	0	0	0	0	0	0	0	1	0	10	
5:30 PM	0	3	0	0	2	6	0	0	0	0	0	0	0	0	1	0	12	
5:45 PM	0	3	0	0	2	7	0	0	0	0	0	0	0	0	2	0	14	
TOTAL VOLUMES :	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL	
APPROACH %'s :	0 0.00%	36 94.74%	2 5.26%	0 0.00%	13 17.57%	54 72.97%	7 9.46%	0 0.00%	1 50.00%	1 50.00%	0 0.00%	0 0.00%	1 3.85%	3 11.54%	22 84.62%	0 0.00%	140 TOTAL	
PEAK HR :	04:00 PM - 05:00 PM																TOTAL	
PEAK HR VOL :	0 0.000	19 0.792	1 0.250	0 0.000	8 0.500	31 0.646	6 0.500	0 0.000	0 0.000	1 0.250	0 0.000	0 0.000	0 0.000	3 0.375	15 0.536	0 0.000	84 0.808	
PEAK HR FACTOR :	0.833				0.703				0.250				0.643					

National Data & Surveying Services Intersection Turning Movement Count

Location: Route 303 & Greenbush/Rte 340
City: Orangeburg
Control: Signalized

Project ID: 22-380027-003
Date: 6/2/2022

Data - Cars

NS/EW Streets:	Route 303				Route 303				Greenbush/Rte 340				Greenbush/Rte 340				TOTAL	
	0 NL	2 NT	0 NR	0 NU	0.5 SL	1.5 ST	0 SR	0 SU	0 EL	1 ET	0 ER	0 EU	0 WL	1 WT	0 WR	0 WU		
AM	NORTHBOUND		SOUTHBOUND		EASTBOUND		WESTBOUND											
7:00 AM	2	77	9	0	26	63	7	0	0	7	1	0	9	1	19	0	221	
7:15 AM	0	103	10	0	25	58	9	0	2	8	1	0	4	2	16	0	238	
7:30 AM	0	143	6	0	33	73	9	0	5	5	5	0	6	3	34	0	322	
7:45 AM	0	116	18	0	48	101	5	0	4	7	1	0	11	9	38	0	358	
8:00 AM	0	129	11	0	44	92	1	0	6	4	1	0	12	9	63	0	372	
8:15 AM	3	76	13	0	50	95	1	0	5	8	1	0	7	3	39	0	301	
8:30 AM	2	114	15	0	42	102	6	0	4	7	4	0	19	8	34	0	357	
8:45 AM	2	95	18	0	39	83	6	0	3	5	1	0	16	4	40	0	312	
TOTAL VOLUMES : APPROACH %'s :	NL 9 0.94%	NT 853 88.67%	NR 100 10.40%	NU 0 0.00%	SL 307 30.16%	ST 667 65.52%	SR 44 4.32%	SU 0 0.00%	EL 29 30.53%	ET 51 53.68%	ER 15 15.79%	EU 0 0.00%	WL 84 20.69%	WT 39 9.61%	WR 283 69.70%	WU 0 0.00%	TOTAL 2481	
PEAK HR :	07:45 AM - 08:45 AM																TOTAL	
PEAK HR VOL :	5	435	57	0	184	390	13	0	19	26	7	0	49	29	174	0	1388	
PEAK HR FACTOR :	0.417	0.843	0.792	0.000	0.920	0.956	0.542	0.000	0.792	0.813	0.438	0.000	0.645	0.806	0.690	0.000	0.933	
PM	NORTHBOUND		SOUTHBOUND		EASTBOUND		WESTBOUND											
4:00 PM	3	125	20	0	37	123	12	0	9	9	3	0	14	10	53	0	418	
4:15 PM	1	137	10	0	43	120	3	0	7	7	2	0	22	11	59	0	422	
4:30 PM	6	132	16	0	52	155	7	0	3	3	3	0	14	16	74	0	481	
4:45 PM	2	105	10	0	42	169	3	0	7	9	1	0	11	7	57	0	423	
5:00 PM	3	79	8	0	45	160	3	1	4	5	4	0	24	11	74	0	421	
5:15 PM	2	92	19	0	43	160	8	0	10	9	3	0	20	7	46	0	419	
5:30 PM	2	93	15	0	47	183	6	0	4	6	1	0	17	5	54	0	433	
5:45 PM	4	106	21	0	55	163	14	0	11	11	8	0	22	10	50	0	475	
TOTAL VOLUMES : APPROACH %'s :	NL 23 2.27%	NT 869 85.95%	NR 119 11.77%	NU 0 0.00%	SL 364 22.01%	ST 1233 74.55%	SR 56 3.39%	SU 1 0.06%	EL 55 39.57%	ET 59 42.45%	ER 25 17.99%	EU 0 0.00%	WL 144 20.93%	WT 77 11.19%	WR 467 67.88%	WU 0 0.00%	TOTAL 3492	
PEAK HR :	04:00 PM - 05:00 PM																TOTAL	
PEAK HR VOL :	12	499	56	0	174	567	25	0	26	28	9	0	61	44	243	0	1744	
PEAK HR FACTOR :	0.500	0.911	0.700	0.000	0.920	0.837	0.839	0.521	0.000	0.722	0.778	0.750	0.000	0.693	0.688	0.821	0.000	0.906
							0.895					0.750				0.837		

National Data & Surveying Services Intersection Turning Movement Count

Location: Route 303 & Greenbush/Rte 340
City: Orangeburg
Control: Signalized

Project ID: 22-380027-003
Date: 6/2/2022

Data - Buses

NS/EW Streets:	Route 303				Route 303				Greenbush/Rte 340				Greenbush/Rte 340					
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND					
AM	0 NL	2 NT	0 NR	0 NU	0.5 SL	1.5 ST	0 SR	0 SU	0 EL	1 ET	0 ER	0 EU	0 WL	1 WT	0 WR	0 WU	TOTAL	
	7:00 AM	0 0	1 1	0 0	3 4	4 2	0 1	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	8	
	7:15 AM	0 0	1 0	1 0	4 3	4 3	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 1	0 0	1 0	10	
	7:30 AM	0 0	0 0	0 0	3 3	4 4	0 0	0 0	0 0	0 0	0 0	0 0	0 0	1 1	0 0	0 0	8	
	7:45 AM	0 0	3 0	0 0	0 0	0 1	1 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	7 0	0 0	12	
	8:00 AM	0 0	2 1	0 1	1 2	1 1	1 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	4 1	0 0	9	
	8:15 AM	0 0	1 0	0 0	2 3	1 3	0 1	0 0	0 0	0 0	0 0	0 0	0 0	0 2	0 0	0 0	4	
	8:30 AM	0 0	0 0	0 0	3 3	1 1	0 0	0 0	0 0	0 0	0 0	0 0	0 0	2 2	0 0	2 0	8	
	8:45 AM	0 0	1 1	0 0	4 4	2 2	0 0	0 0	0 0	1 0	0 0	0 0	1 1	0 1	1 0	0 0	10	
	TOTAL VOLUMES : APPROACH %'s :	NL 0	NT 9	NR 1	NU 0	SL 20	ST 16	SR 3	SU 0	EL 0	ET 1	ER 0	EU 0	WL 4	WT 0	WR 15	WU 0	TOTAL 69
PEAK HR :	07:45 AM - 08:45 AM				51.28% 0.00%				41.03% 90.00%				7.69% 10.00%				TOTAL	
	PEAK HR VOL :	0 0.000	6 0.500	0 0.000	0 0.000	6 0.500	4 1.000	2 0.500	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	2 0.250	0 0.000	13 0.464	0 0.000	TOTAL 33
	PEAK HR FACTOR :	0.500				0.750								0.536				0.688
PM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				TOTAL	
	0 NL	2 NT	0 NR	0 NU	0.5 SL	1.5 ST	0 SR	0 SU	0 EL	1 ET	0 ER	0 EU	0 WL	1 WT	0 WR	0 WU	TOTAL	
	4:00 PM	0 0	3 2	0 0	0 0	1 2	2 2	0 0	0 0	0 0	0 0	0 0	0 0	0 0	2 0	0 0	8	
	4:15 PM	0 0	2 4	0 0	0 0	2 0	2 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 3	0 0	6	
	4:30 PM	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	3 0	0 0	7	
	4:45 PM	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	3 0	0 0	3	
	5:00 PM	0 0	1 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	1	
	5:15 PM	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0	
	5:30 PM	0 0	0 0	0 0	0 0	0 2	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	2	
	5:45 PM	0 0	1 1	0 0	0 0	1 1	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	1 0	0 0	3	
PEAK HR :	04:00 PM - 05:00 PM				40.00% 0.00%				60.00% 100.00%				0.00% 0.00%				TOTAL 30	
	PEAK HR VOL :	0 0.000	9 0.563	0 0.000	0 0.000	3 0.375	4 0.500	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	8 0.667	0 0.000	TOTAL 24	
	PEAK HR FACTOR :	0.563				0.438								0.667				0.750

National Data & Surveying Services Intersection Turning Movement Count

Location: Route 303 & Greenbush/Rte 340
City: Orangeburg
Control: Signalized

Project ID: 22-380027-003
Date: 6/2/2022

Data - Bikes

NS/EW Streets:	Route 303				Route 303				Greenbush/Rte 340				Greenbush/Rte 340				
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
AM	0 NL	2 NT	0 NR	0 NU	0.5 SL	1.5 ST	0 SR	0 SU	0 EL	1 ET	0 ER	0 EU	0 WL	1 WT	0 WR	0 WU	TOTAL
7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	
8:15 AM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	1	
8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
TOTAL VOLUMES : APPROACH %'s :	NL 0	NT 0	NR 0	NU 0	SL 1	ST 0	SR 0	SU 0	EL 0	ET 0	ER 0	EU 0	WL 0	WT 0	WR 1	WU 0	TOTAL 2
PEAK HR :	07:45 AM - 08:45 AM				100.00% 0.00% 0.00% 0.00%				0.00% 0.00% 100.00% 0.00%				0.00% 0.00% 0.00% 0.00%				TOTAL
PEAK HR VOL :	0 0.000	0 0.000	0 0.000	0 0.000	1 0.250	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	1 0.250	0 0.000	2
PEAK HR FACTOR :	0.000				0.250				0.250				0.250				0.500
PM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				TOTAL
	0 NL	2 NT	0 NR	0 NU	0.5 SL	1.5 ST	0 SR	0 SU	0 EL	1 ET	0 ER	0 EU	0 WL	1 WT	0 WR	0 WU	
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
TOTAL VOLUMES : APPROACH %'s :	NL 0	NT 0	NR 0	NU 0	SL 0	ST 0	SR 0	SU 0	EL 0	ET 0	ER 0	EU 0	WL 0	WT 0	WR 0	WU 0	TOTAL 0
PEAK HR :	04:00 PM - 05:00 PM				0.000 0.000 0.000 0.000				0.000 0.000 0.000 0.000				0.000 0.000 0.000 0.000				TOTAL 0
PEAK HR VOL :	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0
PEAK HR FACTOR :	0.000				0.000				0.000				0.000				0.000

National Data & Surveying Services Intersection Turning Movement Count

Location: Route 303 & Greenbush/Rte 340
City: Orangeburg

Project ID: 22-380027-003
Date: 6/2/2022

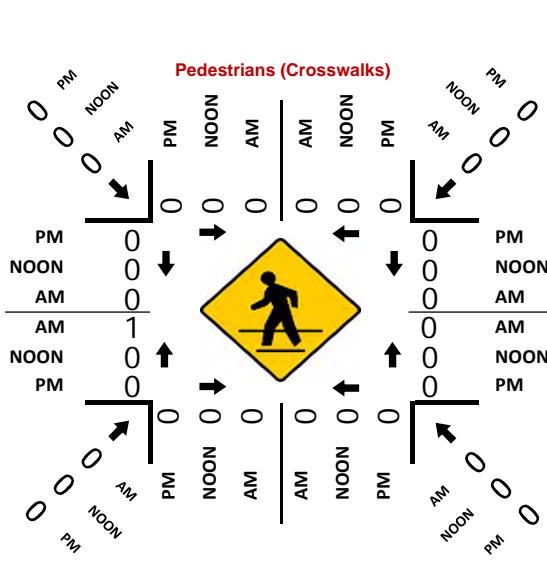
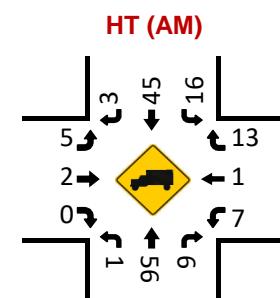
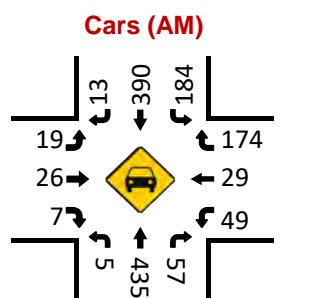
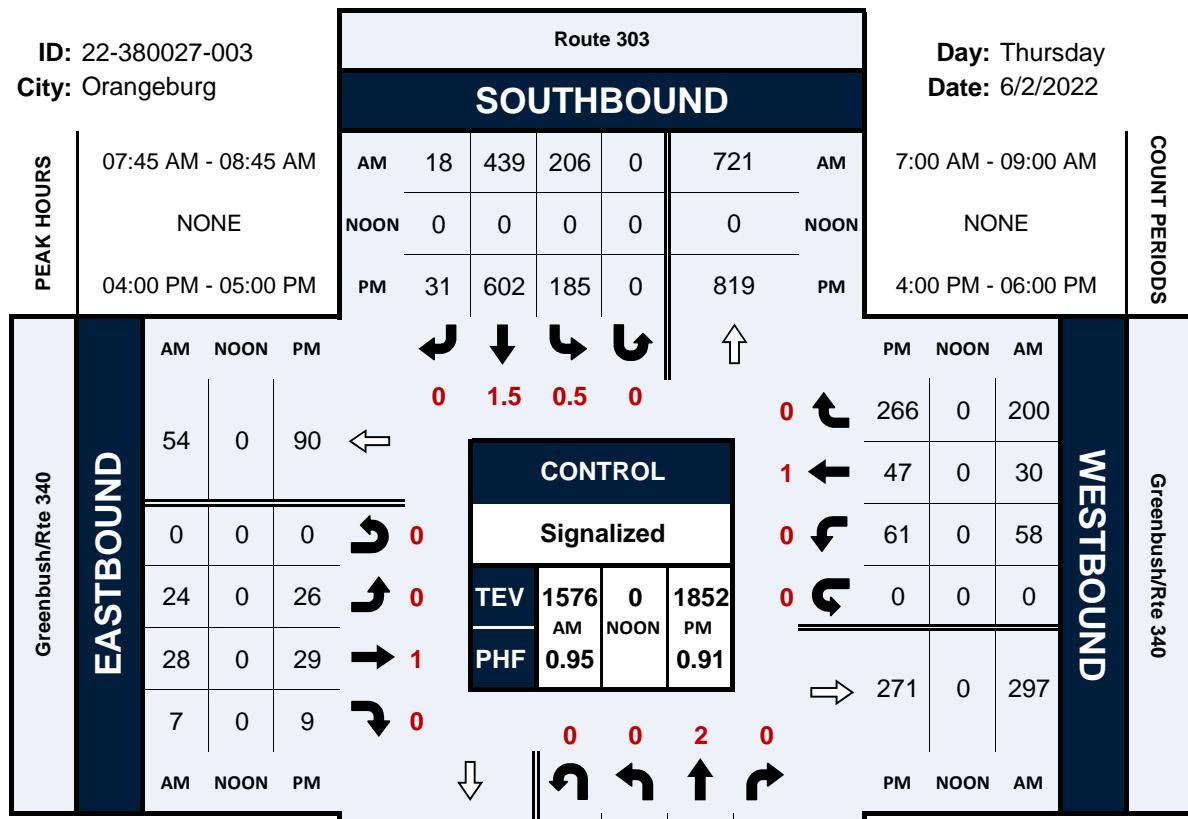
Data - Pedestrians (Crosswalks)

NS/EW Streets:	Route 303		Route 303		Greenbush/Rte 340		Greenbush/Rte 340		
AM	NORTH LEG		SOUTH LEG		EAST LEG		WEST LEG		TOTAL
	EB	WB	EB	WB	NB	SB	NB	SB	
7:00 AM	0	0	0	0	0	0	0	0	0
7:15 AM	0	0	0	0	0	0	0	0	0
7:30 AM	0	0	0	0	0	0	0	0	0
7:45 AM	0	0	0	0	0	0	0	0	0
8:00 AM	0	0	0	0	0	0	0	0	0
8:15 AM	0	0	0	0	0	0	0	0	0
8:30 AM	0	0	0	0	0	0	1	0	1
8:45 AM	0	0	0	1	0	1	0	0	2
TOTAL VOLUMES : APPROACH %'s :	EB 0	WB 0	EB 0	WB 1 0.00% 100.00%	NB 0	SB 1 0.00% 100.00%	NB 1 100.00%	SB 0 0.00%	TOTAL 3
PEAK HR :	07:45 AM - 08:45 AM								TOTAL
PEAK HR VOL :	0		0		0		1 0.250		1
PEAK HR FACTOR :							0.250		0.250
PM	NORTH LEG		SOUTH LEG		EAST LEG		WEST LEG		TOTAL
	EB	WB	EB	WB	NB	SB	NB	SB	
4:00 PM	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	0	0	0	0	0	0	0
4:30 PM	0	0	0	0	0	0	0	0	0
4:45 PM	0	0	0	0	0	0	0	0	0
5:00 PM	0	0	0	0	0	0	0	0	0
5:15 PM	0	0	0	0	0	0	0	0	0
5:30 PM	0	0	0	0	0	0	0	0	0
5:45 PM	0	0	0	0	0	0	0	0	0
TOTAL VOLUMES : APPROACH %'s :	EB 0	WB 0	EB 0	WB 0	NB 0	SB 0	NB 0	SB 0	TOTAL 0
PEAK HR :	04:00 PM - 05:00 PM								TOTAL 0
PEAK HR VOL :	0		0		0		0		0
PEAK HR FACTOR :									

Route 303 & Greenbush/Rte 340**Peak Hour Turning Movement Count**

ID: 22-380027-003
City: Orangeburg

Day: Thursday
Date: 6/2/2022



National Data & Surveying Services Intersection Turning Movement Count

Location: Route 303 & Glenshaw St
City: Orangeburg
Control: Signalized

Project ID: 22-380027-004
Date: 6/2/2022

Data - Total

NS/EW Streets:	Route 303				Route 303				Glenshaw St				Glenshaw St				
	0 NL	2 NT	0 NR	0 NU	0 SL	2 ST	0 SR	0 SU	0 EL	1 ET	0 ER	0 EU	0 WL	0 WT	0 WR	0 WU	
AM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
7:00 AM	7	74	0	0	0	96	6	0	0	0	2	0	0	0	0	0	185
7:15 AM	9	110	0	0	0	101	4	0	1	0	2	0	0	0	0	0	227
7:30 AM	18	145	0	0	0	124	10	0	2	0	3	0	0	0	0	0	302
7:45 AM	3	149	0	0	0	144	18	0	5	0	1	0	0	0	0	0	320
8:00 AM	12	121	0	0	0	150	14	0	11	0	9	0	0	0	0	0	317
8:15 AM	11	116	0	0	0	136	8	0	2	0	5	0	0	0	0	0	278
8:30 AM	9	120	0	0	0	150	10	0	6	0	6	0	0	0	0	0	301
8:45 AM	11	122	0	0	0	125	11	0	4	0	2	0	0	0	0	0	275
TOTAL VOLUMES : APPROACH %'s :	NL 80 7.71%	NT 957 92.29%	NR 0 0.00%	NU 0 0.00%	SL 0 0.00%	ST 1026 92.68%	SR 81 7.32%	SU 0 0.00%	EL 31 50.82%	ET 0 0.00%	ER 30 49.18%	EU 0 0.00%	WL 0 0	WT 0 0	WR 0 0	WU 0 0	TOTAL 2205
PEAK HR :	07:30 AM - 08:30 AM																TOTAL
PEAK HR VOL :	44 0.611	531 0.891	0 0.000	0 0.000	0 0.000	554 0.923	50 0.694	0 0.000	20 0.455	0 0.000	18 0.500	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	1217 0.951
PM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
4:00 PM	2	165	0	0	0	132	3	0	9	0	9	0	0	0	0	0	320
4:15 PM	2	171	0	0	0	147	4	0	2	0	4	0	0	0	0	0	330
4:30 PM	2	196	0	0	0	167	8	0	7	0	8	0	0	0	0	0	388
4:45 PM	3	164	0	0	0	206	3	0	12	0	11	0	0	0	0	0	399
5:00 PM	2	142	0	0	0	179	2	0	14	0	26	0	0	0	0	0	365
5:15 PM	3	149	0	0	0	166	1	0	10	0	22	0	0	0	0	0	351
5:30 PM	3	135	0	0	0	193	1	0	3	0	16	0	0	0	0	0	351
5:45 PM	1	147	0	0	0	176	2	0	5	0	10	0	0	0	0	0	341
TOTAL VOLUMES : APPROACH %'s :	NL 18 1.40%	NT 1269 98.60%	NR 0 0.00%	NU 0 0.00%	SL 0 0.00%	ST 1366 98.27%	SR 24 1.73%	SU 0 0.00%	EL 62 36.90%	ET 0 0.00%	ER 106 63.10%	EU 0 0.00%	WL 0 0	WT 0 0	WR 0 0	WU 0 0	TOTAL 2845
PEAK HR :	04:30 PM - 05:30 PM																TOTAL
PEAK HR VOL :	10 0.833	651 0.830	0 0.000	0 0.000	0 0.000	718 0.871	14 0.438	0 0.000	43 0.768	0 0.000	67 0.644	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	1503 0.942
PEAK HR FACTOR :	0.835				0.876				0.688								

National Data & Surveying Services Intersection Turning Movement Count

Location: Route 303 & Glenshaw St
City: Orangeburg
Control: Signalized

Project ID: 22-380027-004
Date: 6/2/2022

Data - Cars

NS/EW Streets:	Route 303				Route 303				Glenshaw St				Glenshaw St				
	0 NL	2 NT	0 NR	0 NU	0 SL	2 ST	0 SR	0 SU	0 EL	1 ET	0 ER	0 EU	0 WL	0 WT	0 WR	0 WU	TOTAL
AM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
7:00 AM	7	66	0	0	0	79	4	0	0	0	1	0	0	0	0	157	
7:15 AM	9	97	0	0	0	75	4	0	1	0	2	0	0	0	0	188	
7:30 AM	18	129	0	0	0	105	8	0	1	0	3	0	0	0	0	264	
7:45 AM	3	116	0	0	0	128	17	0	5	0	0	0	0	0	0	269	
8:00 AM	12	105	0	0	0	132	12	0	6	0	7	0	0	0	0	274	
8:15 AM	11	87	0	0	0	123	8	0	0	0	2	0	0	0	0	231	
8:30 AM	8	101	0	0	0	130	8	0	1	0	3	0	0	0	0	251	
8:45 AM	11	100	0	0	0	99	8	0	2	0	2	0	0	0	0	222	
TOTAL VOLUMES : APPROACH %'s :	NL 79 8.98%	NT 801 91.02%	NR 0 0.00%	NU 0 0.00%	SL 0 0.00%	ST 871 92.66%	SR 69 7.34%	SU 0 0.00%	EL 16 44.44%	ET 0 0.00%	ER 20 55.56%	EU 0 0.00%	WL 0 0	WT 0 0	WR 0 0	WU 0 0	TOTAL 1856
PEAK HR :	07:30 AM - 08:30 AM																TOTAL
PEAK HR VOL :	44	437	0	0	0	488	45	0	12	0	12	0	0	0	0	0	1038
PEAK HR FACTOR :	0.611	0.847	0.000	0.000	0.000	0.924	0.662	0.000	0.500	0.000	0.429	0.000	0.000	0.000	0.000	0.947	
PM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
4:00 PM	0	159	0	0	0	120	3	0	8	0	9	0	0	0	0	0	299
4:15 PM	1	161	0	0	0	131	3	0	2	0	3	0	0	0	0	0	301
4:30 PM	0	182	0	0	0	159	3	0	5	0	6	0	0	0	0	0	355
4:45 PM	2	150	0	0	0	199	1	0	9	0	10	0	0	0	0	0	371
5:00 PM	0	128	0	0	0	172	2	0	13	0	25	0	0	0	0	0	340
5:15 PM	1	143	0	0	0	160	1	0	8	0	21	0	0	0	0	0	334
5:30 PM	1	131	0	0	0	188	0	0	3	0	15	0	0	0	0	0	338
5:45 PM	1	140	0	0	0	168	1	0	4	0	9	0	0	0	0	0	323
TOTAL VOLUMES : APPROACH %'s :	NL 6 0.50%	NT 1194 99.50%	NR 0 0.00%	NU 0 0.00%	SL 0 0.00%	ST 1297 98.93%	SR 14 1.07%	SU 0 0.00%	EL 52 34.67%	ET 0 0.00%	ER 98 65.33%	EU 0 0.00%	WL 0 0	WT 0 0	WR 0 0	WU 0 0	TOTAL 2661
PEAK HR :	04:30 PM - 05:30 PM																TOTAL
PEAK HR VOL :	3	603	0	0	0	690	7	0	35	0	62	0	0	0	0	0	1400
PEAK HR FACTOR :	0.375	0.828	0.000	0.000	0.000	0.867	0.583	0.000	0.673	0.000	0.620	0.000	0.000	0.000	0.000	0.943	

National Data & Surveying Services Intersection Turning Movement Count

Location: Route 303 & Glenshaw St
City: Orangeburg
Control: Signalized

Project ID: 22-380027-004
Date: 6/2/2022

Data - HT

NS/EW Streets:	Route 303				Route 303				Glenshaw St				Glenshaw St				TOTAL
	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	
AM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
7:00 AM	0	7	0	0	0	14	2	0	0	0	1	0	0	0	0	0	24
7:15 AM	0	11	0	0	0	23	0	0	0	0	0	0	0	0	0	0	34
7:30 AM	0	12	0	0	0	16	2	0	0	0	0	0	0	0	0	0	30
7:45 AM	0	28	0	0	0	13	1	0	0	0	1	0	0	0	0	0	43
8:00 AM	0	15	0	0	0	15	2	0	4	0	2	0	0	0	0	0	38
8:15 AM	0	21	0	0	0	12	0	0	2	0	3	0	0	0	0	0	38
8:30 AM	1	13	0	0	0	19	2	0	5	0	3	0	0	0	0	0	43
8:45 AM	0	21	0	0	0	22	3	0	2	0	0	0	0	0	0	0	48
TOTAL VOLUMES : APPROACH %'s :	NL 1 0.78%	NT 128 99.22%	NR 0 0.00%	NU 0 0.00%	SL 0 0.00%	ST 134 91.79%	SR 12 8.22%	SU 0 0.00%	EL 13 56.52%	ET 0 0.00%	ER 10 43.48%	EU 0 0.00%	WL 0 0	WT 0 0	WR 0 0	WU 0 0	TOTAL 298
PEAK HR :	07:30 AM - 08:30 AM																TOTAL
PEAK HR VOL :	0	76	0	0	0	56	5	0	6	0	6	0	0	0	0	0	149
PEAK HR FACTOR :	0.000	0.679	0.000	0.000	0.000	0.875	0.625	0.000	0.375	0.000	0.500	0.000	0.000	0.000	0.000	0.866	
PM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
4:00 PM	0	2	0	0	0	10	0	0	1	0	0	0	0	0	0	0	17
4:15 PM	1	10	0	0	0	13	1	0	0	0	1	0	0	0	0	0	26
4:30 PM	2	12	0	0	0	8	5	0	2	0	2	0	0	0	0	0	31
4:45 PM	1	12	0	0	0	7	2	0	3	0	1	0	0	0	0	0	26
5:00 PM	2	13	0	0	0	6	0	0	1	0	1	0	0	0	0	0	23
5:15 PM	2	6	0	0	0	5	0	0	2	0	1	0	0	0	0	0	16
5:30 PM	2	4	0	0	0	5	1	0	0	0	1	0	0	0	0	0	13
5:45 PM	0	4	0	0	0	8	1	0	1	0	1	0	0	0	0	0	15
TOTAL VOLUMES : APPROACH %'s :	NL 12 15.58%	NT 65 84.42%	NR 0 0.00%	NU 0 0.00%	SL 0 0.00%	ST 62 86.11%	SR 10 13.89%	SU 0 0.00%	EL 10 55.56%	ET 0 0.00%	ER 8 44.44%	EU 0 0.00%	WL 0 0	WT 0 0	WR 0 0	WU 0 0	TOTAL 167
PEAK HR :	04:30 PM - 05:30 PM																TOTAL
PEAK HR VOL :	7	43	0	0	0	26	7	0	8	0	5	0	0	0	0	0	96
PEAK HR FACTOR :	0.875	0.827	0.000	0.000	0.000	0.813	0.350	0.000	0.667	0.000	0.625	0.000	0.000	0.000	0.000	0.774	

National Data & Surveying Services Intersection Turning Movement Count

Location: Route 303 & Glenshaw St
City: Orangeburg
Control: Signalized

Project ID: 22-380027-004
Date: 6/2/2022

Data - Buses

NS/EW Streets:	Route 303				Route 303				Glenshaw St				Glenshaw St				
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
AM	0 NL	2 NT	0 NR	0 NU	0 SL	2 ST	0 SR	0 SU	0 EL	1 ET	0 ER	0 EU	0 WL	0 WT	0 WR	0 WU	TOTAL
7:00 AM	0 NL	1 NT	0 NR	0 NU	0 SL	3 ST	0 SR	0 SU	0 EL	0 ET	0 ER	0 EU	0 WL	0 WT	0 WR	0 WU	4
7:15 AM	0 NL	2 NT	0 NR	0 NU	0 SL	3 ST	0 SR	0 SU	0 EL	0 ET	0 ER	0 EU	0 WL	0 WT	0 WR	0 WU	5
7:30 AM	0 NL	4 NT	0 NR	0 NU	0 SL	3 ST	0 SR	0 SU	1 EL	0 ET	0 ER	0 EU	0 WL	0 WT	0 WR	0 WU	8
7:45 AM	0 NL	5 NT	0 NR	0 NU	0 SL	3 ST	0 SR	0 SU	0 EL	0 ET	0 ER	0 EU	0 WL	0 WT	0 WR	0 WU	8
8:00 AM	0 NL	1 NT	0 NR	0 NU	0 SL	3 ST	0 SR	0 SU	1 EL	0 ET	0 ER	0 EU	0 WL	0 WT	0 WR	0 WU	5
8:15 AM	0 NL	8 NT	0 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	0 ET	0 ER	0 EU	0 WL	0 WT	0 WR	0 WU	9
8:30 AM	0 NL	6 NT	0 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	0 ET	0 ER	0 EU	0 WL	0 WT	0 WR	0 WU	7
8:45 AM	0 NL	1 NT	0 NR	0 NU	0 SL	4 ST	0 SR	0 SU	0 EL	0 ET	0 ER	0 EU	0 WL	0 WT	0 WR	0 WU	5
TOTAL VOLUMES :	NL 0	NT 28	NR 0	NU 0	SL 0	ST 21	SR 0	SU 0	EL 2	ET 0	ER 0	EU 0	WL 0	WT 0	WR 0	WU 0	TOTAL 51
APPROACH %'s:	0.00% 100.00%	0.00% 0.00%	0.00% 0.00%	0.00% 0.00%	0.00% 100.00%	0.00% 0.00%	0.00% 0.00%	0.00% 0.00%	100.00% 100.00%	0.00% 0.00%	0.00% 0.00%	0.00% 0.00%	0.0000	0.0000	0.0000	0.0000	TOTAL
PEAK HR :	07:30 AM - 08:30 AM																TOTAL
PEAK HR VOL :	0 0.000	18 0.563	0 0.000	0 0.000	0 0.000	10 0.833	0 0.000	0 0.000	2 0.500	0 0.000	0 0.000	0 0.000	0 0.0000	0 0.0000	0 0.0000	0 0.0000	30
PEAK HR FACTOR :	0.563 0.563				0.833 0.833				0.500 0.500				0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.833
PM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				TOTAL
	0 NL	2 NT	0 NR	0 NU	0 SL	2 ST	0 SR	0 SU	0 EL	1 ET	0 ER	0 EU	0 WL	0 WT	0 WR	0 WU	
4:00 PM	0 NL	2 NT	0 NR	0 NU	0 SL	2 ST	0 SR	0 SU	0 EL	0 ET	0 ER	0 EU	0 WL	0 WT	0 WR	0 WU	4
4:15 PM	0 NL	0 NT	0 NR	0 NU	0 SL	3 ST	0 SR	0 SU	0 EL	0 ET	0 ER	0 EU	0 WL	0 WT	0 WR	0 WU	3
4:30 PM	0 NL	2 NT	0 NR	0 NU	0 SL	0 ST	0 SR	0 SU	0 EL	0 ET	0 ER	0 EU	0 WL	0 WT	0 WR	0 WU	2
4:45 PM	0 NL	2 NT	0 NR	0 NU	0 SL	0 ST	0 SR	0 SU	0 EL	0 ET	0 ER	0 EU	0 WL	0 WT	0 WR	0 WU	2
5:00 PM	0 NL	1 NT	0 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	0 ET	0 ER	0 EU	0 WL	0 WT	0 WR	0 WU	2
5:15 PM	0 NL	0 NT	0 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	0 ET	0 ER	0 EU	0 WL	0 WT	0 WR	0 WU	1
5:30 PM	0 NL	0 NT	0 NR	0 NU	0 SL	0 ST	0 SR	0 SU	0 EL	0 ET	0 ER	0 EU	0 WL	0 WT	0 WR	0 WU	0
5:45 PM	0 NL	3 NT	0 NR	0 NU	0 SL	0 ST	0 SR	0 SU	0 EL	0 ET	0 ER	0 EU	0 WL	0 WT	0 WR	0 WU	3
TOTAL VOLUMES :	NL 0	NT 10	NR 0	NU 0	SL 0	ST 7	SR 0	SU 0	EL 0	ET 0	ER 0	EU 0	WL 0	WT 0	WR 0	WU 0	TOTAL 17
APPROACH %'s:	0.00% 100.00%	0.00% 0.00%	0.00% 0.00%	0.00% 0.00%	0.00% 100.00%	0.00% 0.00%	0.00% 0.00%	0.00% 0.00%	100.00% 100.00%	0.00% 0.00%	0.00% 0.00%	0.00% 0.00%	0.0000	0.0000	0.0000	0.0000	TOTAL
PEAK HR :	04:30 PM - 05:30 PM																TOTAL
PEAK HR VOL :	0 0.000	5 0.625	0 0.000	0 0.000	0 0.000	2 0.500	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.0000	0 0.0000	0 0.0000	0 0.0000	7
PEAK HR FACTOR :	0.625 0.625				0.500 0.500				0.500 0.500				0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.875

National Data & Surveying Services Intersection Turning Movement Count

Location: Route 303 & Glenshaw St
City: Orangeburg
Control: Signalized

Project ID: 22-380027-004
Date: 6/2/2022

Data - Bikes

NS/EW Streets:	Route 303				Route 303				Glenshaw St				Glenshaw St					
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND					
AM	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL	
7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		
7:15 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	1		
7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		
7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		
8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		
8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		
8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		
8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		
TOTAL VOLUMES : APPROACH %'s :	NL 0	NT 1	NR 0	NU 0	SL 0	ST 0	SR 0	SU 0	EL 0	ET 0	ER 0	EU 0	WL 0	WT 0	WR 0	WU 0	TOTAL 1	
APPROACH %'s :	0.00% 100.00%	0.00% 0.00%	0.00% 0.00%	0.00% 0.00%	PEAK HR : 07:30 AM - 08:30 AM												TOTAL 0	
PEAK HR VOL :	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	TOTAL 0	
PEAK HR FACTOR :	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000		
PM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND					
	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL	
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
TOTAL VOLUMES : APPROACH %'s :	NL 0	NT 0	NR 0	NU 0	SL 0	ST 0	SR 0	SU 0	EL 0	ET 0	ER 0	EU 0	WL 0	WT 0	WR 0	WU 0	TOTAL 0	
APPROACH %'s :	0.00% 0.00%	0.00% 0.00%	0.00% 0.00%	0.00% 0.00%	PEAK HR : 04:30 PM - 05:30 PM												TOTAL 0	
PEAK HR VOL :	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	TOTAL 0	
PEAK HR FACTOR :	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000		

National Data & Surveying Services Intersection Turning Movement Count

Location: Route 303 & Glenshaw St
City: Orangeburg

Project ID: 22-380027-004
Date: 6/2/2022

Data - Pedestrians (Crosswalks)

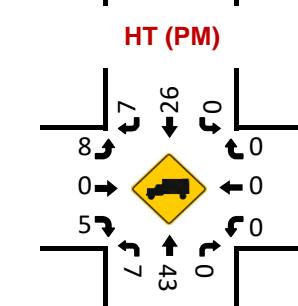
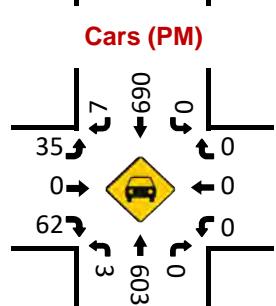
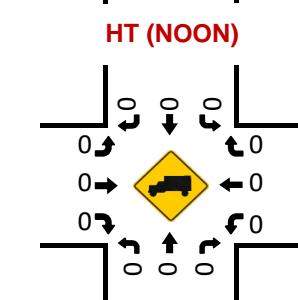
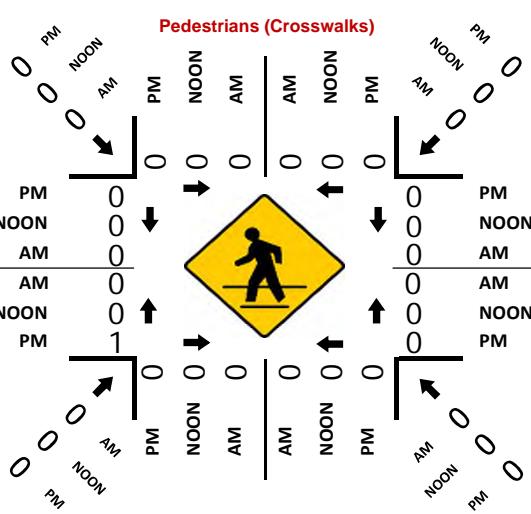
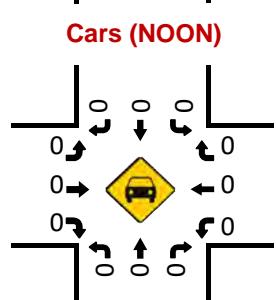
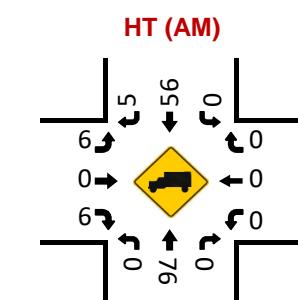
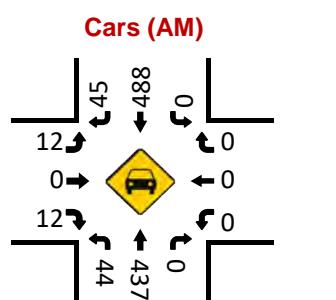
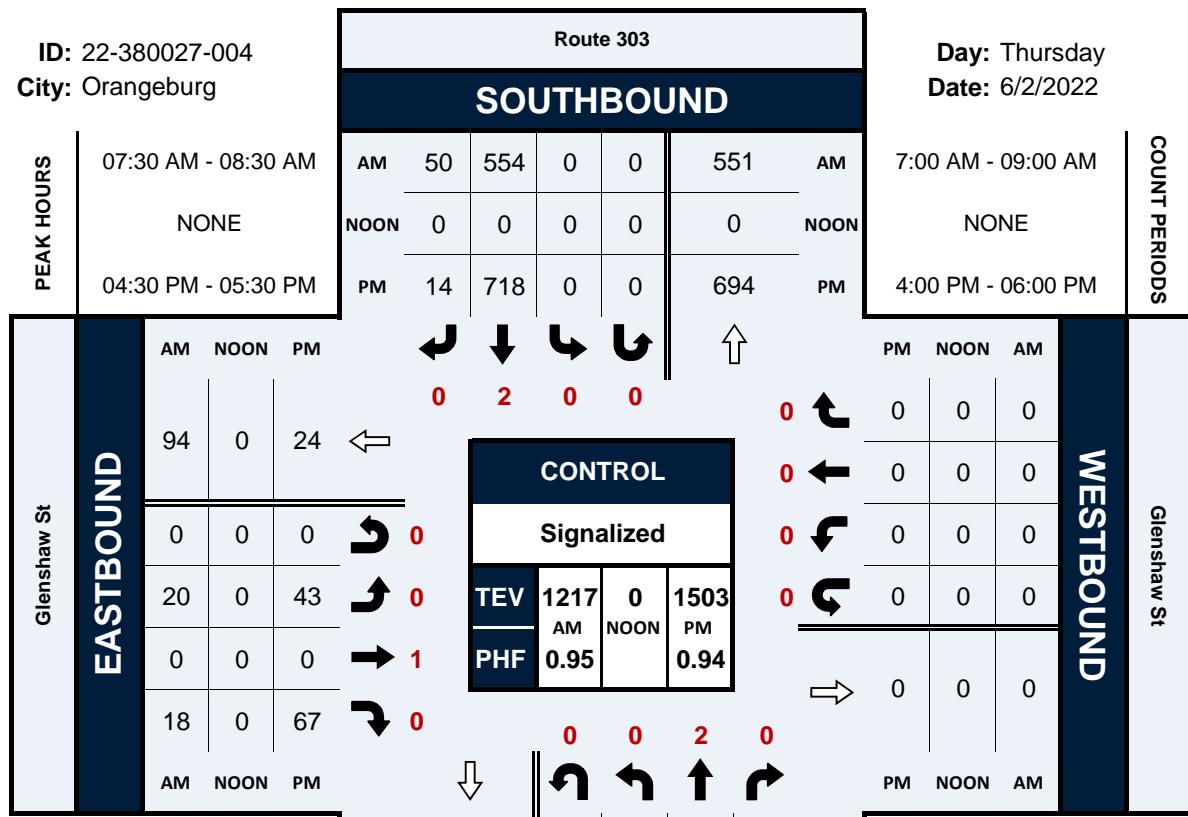
NS/EW Streets:	Route 303		Route 303		Glenshaw St		Glenshaw St		
AM	NORTH LEG		SOUTH LEG		EAST LEG		WEST LEG		TOTAL
	EB	WB	EB	WB	NB	SB	NB	SB	
7:00 AM	0	0	0	0	0	0	0	0	0
7:15 AM	0	0	0	0	0	0	0	0	0
7:30 AM	0	0	0	0	0	0	0	0	0
7:45 AM	0	0	0	0	0	0	0	0	0
8:00 AM	0	0	0	0	0	0	0	0	0
8:15 AM	0	0	0	0	0	0	0	0	0
8:30 AM	0	0	0	0	0	0	0	0	0
8:45 AM	0	0	0	0	0	0	0	0	0
TOTAL VOLUMES : APPROACH %'s :	EB 0	WB 0	EB 0	WB 0	NB 0	SB 0	NB 0	SB 0	TOTAL 0
PEAK HR :	07:30 AM - 08:30 AM								TOTAL 0
PEAK HR VOL : PEAK HR FACTOR :	0 0		0 0		0 0		0 0		TOTAL 0

NS/EW Streets:	Route 303		Route 303		Glenshaw St		Glenshaw St		
PM	NORTH LEG		SOUTH LEG		EAST LEG		WEST LEG		TOTAL
	EB	WB	EB	WB	NB	SB	NB	SB	
4:00 PM	0	0	0	0	0	0	1	0	1
4:15 PM	0	0	0	0	0	0	0	0	0
4:30 PM	0	0	0	0	0	0	0	0	0
4:45 PM	0	0	0	0	0	0	0	0	0
5:00 PM	0	0	0	0	0	0	1	0	1
5:15 PM	0	0	0	0	0	0	0	0	0
5:30 PM	0	0	0	0	0	0	0	0	0
5:45 PM	0	0	0	0	0	0	0	0	0
TOTAL VOLUMES : APPROACH %'s :	EB 0	WB 0	EB 0	WB 0	NB 0	SB 0	NB 2	SB 0	TOTAL 2
PEAK HR :	04:30 PM - 05:30 PM								TOTAL 1
PEAK HR VOL : PEAK HR FACTOR :	0 0		0 0		0 0		1 0 0.250 0.250		TOTAL 1 0.250

Route 303 & Glenshaw St**Peak Hour Turning Movement Count**

ID: 22-380027-004
City: Orangeburg

Day: Thursday
Date: 6/2/2022



STATE OF NEW YORK - DEPARTMENT OF TRANSPORTATION
TRAFFIC ENGINEERING & SAFETY DIVISION
TRAFFIC CONTROL SPECIFICATIONSSTUDY :
CONTRACT :
PIN :
FILE :

R-86

ROCKLAND
COUNTY

PAGE 1 OF 20 PAGES

INTERSECTION Route 303 @ Mountainview Ave. CITY VILLAGE TOWN OF ORANGEBURGDepartment Order filed 11/10/96 as Section 2039.15 Subdivision _____Prior specifications hereby superseded None August 14, 1996Purpose : REPLACE SIGNAL UNDER CONTRACT #D258360, INSTALL LEAD/LAG S.B./N.B
ARROWS ADD PEDS A & BThese specifications will be effective upon the Installation Modification of
the necessary traffic control device(s) required by and conforming to the State Manual
of Uniform Traffic Control Devices

I. This Signal shall

A. Operate in accordance with the Table of Operations and / or Change intervals as
shown on page(s) 2 as a :

- Pretimed Signal
- Semi-traffic actuated signal
- Full-traffic actuated signal
- Pedestrian actuated signal
- Other _____

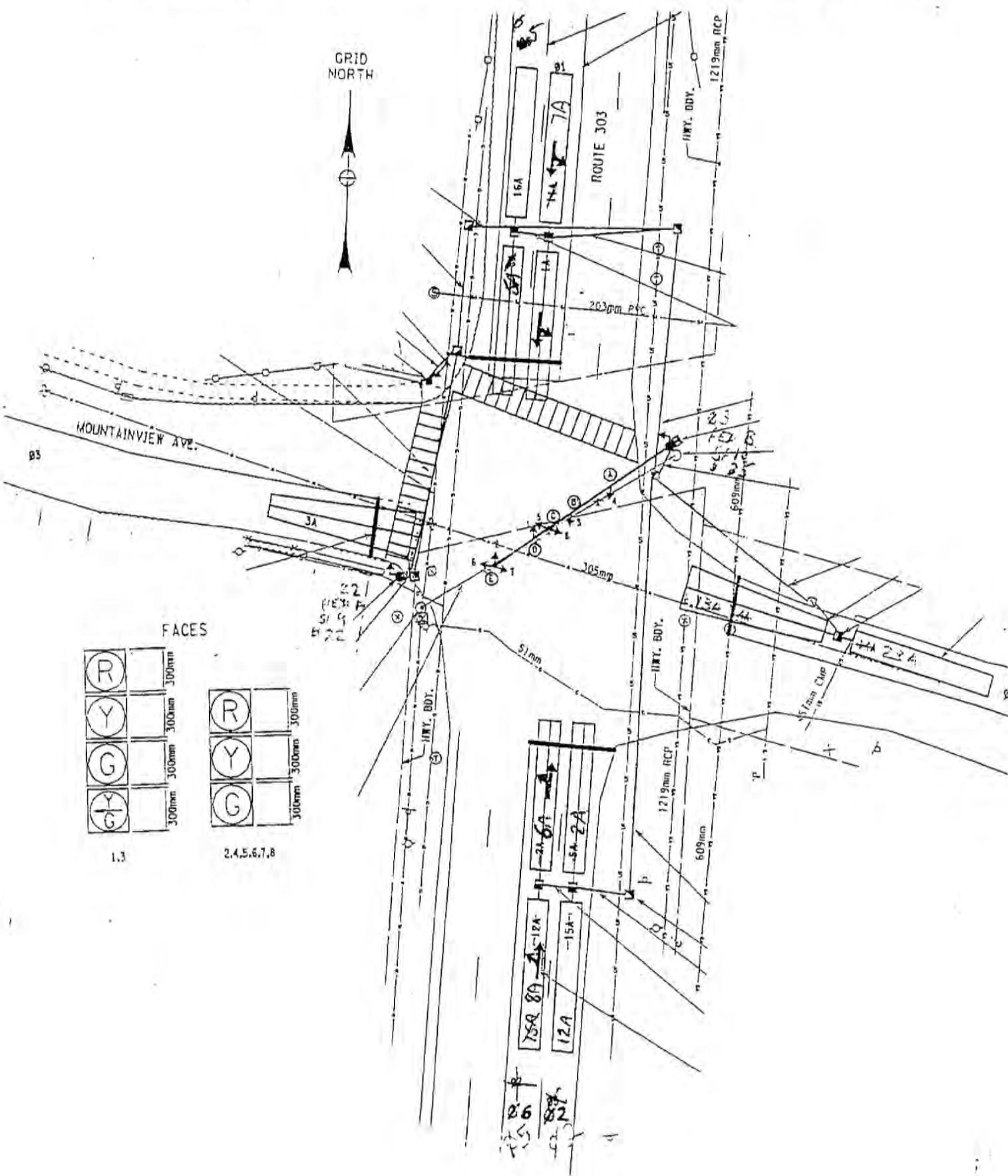
- B.
-
- Display vehicular indications
-
-
- Display pedestrian indications
-
-
- Be equipped with vehicle detectors
-
-
- Be equipped with Pedestrian pushbuttons

as shown in the schematic scaled drawing on page 3C. Be equipped with pre-emption which are described as follows interconnection and / or coordinationcc: (1) Main Office
(1) Region 8 Traffic Engineer
(1) MILLWOOD FIELD OFC.
(1) CONTRACT MAINTAINER

9/10/1	—	Signature	RTE
Date	—	—	Title
Installation Date			
Modification Date <u>September 10, 2001</u>			

MODEL 179 SIGNAL OPERATION
PROGRAMMABLE FEATURES
SIGNAL OPERATION SPECIFICATION

TAPS _____
STUDY # _____
FILE # _____
PAGE 29 OF 20

SIGNAL # R-86COUNTY ROCKLANDDATE 2GRID
NORTH

Overlap 1-16 Program Params & Parm# [1.5.2.1][1.5.2.2]
Coord Transition, CoorPhs [2.5]

Overlap Conflict Lock	OFF	Overlap Lock Inhibit	OFF	Parent Ph Clearance	ON	Extra Included Ph	OFF	Path# Short Long Dwell No Shortway Ø E-Yld Offset RailHld				Min Pwd Perm	
								Included Ø	Modifier Ø	Conflict Ø	Normal		
1	Modifier Ø			Gm				Gm	3	12	22		
	Conflict Ø			Yel	3.5			Yel	4	12	22		
A	Conflict Olap			Red 1.5	I	Conflict Olap		Red 1.5	LG	5	12	22	
	Conflict Ped			LG		Conflict Ped		NORMAL		6	12	22	
	Included Ø					Included Ø							
2	Modifier Ø			Gm	10	Modifier Ø		Gm	7	12	22		
	Conflict Ø			Yel	3.5	Conflict Ø		Yel	Red 1.5	LG	10	12	22
B	Conflict Olap			Red 1.5	J	Conflict Olap		Red 1.5					
	Conflict Ped			LG		Conflict Ped		NORMAL					
	Included Ø					Included Ø							
3	Modifier Ø			Gm	11	Modifier Ø		Gm	12	12	22		
	Conflict Ø			Yel	3.5	Conflict Ø		Yel	3.5	13	12	22	
C	Conflict Olap			Red 1.5	K	Conflict Olap		Red 1.5	LG	15	12	22	
	Conflict Ped			LG		Conflict Ped		NORMAL		16	12	22	
	Included Ø					Included Ø							
4	Modifier Ø			Gm	12	Modifier Ø		Gm	17	12	22		
	Conflict Ø			Yel	3.5	Conflict Ø		Yel	3.5	18	12	22	
D	Conflict Olap			Red 1.5	L	Conflict Olap		Red 1.5	LG	19	12	22	
	Conflict Ped			LG		Conflict Ped		NORMAL		20	12	22	
	Included Ø					Included Ø							
5	Modifier Ø			Gm	13	Modifier Ø		Gm	22	12	22		
	Conflict Ø			Yel	3.5	Conflict Ø		Yel	3.5	23	12	22	
E	Conflict Olap			Red 1.5	M	Conflict Olap		Red 1.5	LG	24	12	22	
	Conflict Ped			LG		Conflict Ped		NORMAL		25			
	Included Ø					Included Ø							
6	Modifier Ø			Gm	14	Modifier Ø		Gm	27				
	Conflict Ø			Yel	3.5	Conflict Ø		Yel	3.5				
F	Conflict Olap			Red 1.5	N	Conflict Olap		Red 1.5	LG	30			
	Conflict Ped			LG		Conflict Ped		NORMAL		31			
	Included Ø					Included Ø							
7	Modifier Ø			Gm	15	Modifier Ø		Gm	32				
	Conflict Ø			Yel	3.5	Conflict Ø		Yel	3.5				
G	Conflict Olap			Red 1.5	O	Conflict Olap		Red 1.5	LG	33			
	Conflict Ped			LG		Conflict Ped		NORMAL		34			
	Included Ø					Included Ø							
8	Modifier Ø			Gm	16	Modifier Ø		Gm	37				
	Conflict Ø			Yel	3.5	Conflict Ø		Yel	3.5				
H	Conflict Olap			Red 1.5	P	Conflict Olap		Red 1.5	LG	38			
	Conflict Ped			LG		Conflict Ped		NORMAL		39			
	Included Ø					Included Ø							
	Channel Settings [1.8.1]												
.....	Channel / Olap #	1	2	3	4	5	6	7	8	9	10	11	12
.....	Phase / VEH	VEH	VEH	VEH	VEH	VEH	VEH	VEH	VEH	VEH	VEH	VEH	VEH
.....	Channel Type	RED	RED	RED	RED	RED	RED	RED	RED	RED	RED	RED	RED
.....	Channel Flash												
.....	Alt Hz												
	Channel + Settings [1.8.4]												
.....	Channel ->	1	2	3	4	5	6	7	8	9	10	11	12
.....	Phase Red +												
.....	Flash Red +												
.....	Flash Yellow +												
.....	Flash Green +												
.....	Flash Inh Red +												
.....	Olap Ovrd												

Channel Params[1.8.3]

C1 IO Mode USER

: BLU Map SINGLE

Invert Rail Input OFF

Preemption Times [3.1], Options+ [3.6]						Track Clear Phases [3.2], Track Clear Overlaps+ [3.5]					
Pre #	Enable	Type	Output	Delay	MinDura	Pre #	Track Phases		Pre #	Track Phases	Track Overlaps
1	ON	RAIL	DWELL		1						
2	ON	RAIL	DWELL		2						
3	ON	EMERG	DWELL		3						
4	ON	EMERG	DWELL		4						
5	ON	EMERG	DWELL		5						
6	ON	EMERG	DWELL		6						
Pre #	MaxPres	MinGrn	MinWlk	PedClr	Co+Pre	Dwell Phases [3.2] and Overlaps+ [3.5]					
1					ON	1	Phases				
2					ON	1	Overlaps				
3					ON	1	Peds				
4					ON	2	Phases				
5					ON	2	Overlaps				
6					ON	2	Peds				
Pre #	Track Grn	Min Dwell	Ext Dwell	PedCir+	Yel	1	3	Phases			
1		2				2	3	Overlaps			
2		2				2	3	Peds			
3		2				2	4	Phases			
4		2				2	4	Overlaps			
5		2				2	4	Peds			
6		2				2	5	Phases			
Pre #	Red	Pattern	Skip			1	5	Overlaps			
1			OFF			1	5	Peds			
2			OFF			1	6	Phases			
3			OFF			1	6	Overlaps			
4			OFF			1	6	Peds			
5			OFF			1	6	Phases			
6			OFF			1	6	Overlaps			
						1	6	Peds			
Low Priority Preempts						Preemption Options+ [3.6]					
Pre #	Type	Min	Max			Pre #	Lock	Override	Auto Fish	Override	Fish Higher Dwell Link
7	OFF					1	ON	ON		ON	OFF
8	OFF					2	ON	ON		ON	OFF
9	OFF					3	ON	ON		ON	OFF
10	OFF					4	ON	ON		ON	OFF
						5	ON	ON		ON	OFF
						6	ON	ON		ON	OFF
Unit Parameters [1.2.1]						Exit Phases [3.2]					
Pre #	Type	Min	Max			Pre #	Exit Phase		Override		
7	OFF					1	1	ON	ON	ON	OFF
8	OFF					2	2	ON	ON	ON	OFF
9	OFF					3	3	ON	ON	ON	OFF
10	OFF					4	4	ON	ON	ON	OFF
						5	5	ON	ON	ON	OFF
						6	6	ON	ON	ON	OFF
Channel Parameters [1.8.3]						Stop Timer Over Preempt					
						1	OFF				
						2	PRE				
						3					
						4					
						5					
						6					
D Conn Mappings						Max Seek Track Time					
						1	NONE				
						2	OFF				
						3					
						4					
						5					
						6					
Pre Invert Rail Input						Max Seek Dwell Time					
						1					
						2					
						3					
						4					
						5					
						6					

Alt# 1 Times Table [1.1.6.1.2]								
Column#....>	1	2	3	4	5	6	7	8
Assign Ø								
Min Grm								
Gap, Ext								
Max 1								
Max 2								
Yel Cir								
Red Cir								
Walk								
Ped Cir								

Alt# 2 Times Table [1.1.6.1.2]								
Column#....>	1	2	3	4	5	6	7	8
Assign Ø								
Min Grm								
Gap, Ext								
Max 1								
Max 2								
Yel Cir								
Red Cir								
Walk								
Ped Cir								

Alt# 3 Times Table [1.1.6.1.3]								
Column#....>	1	2	3	4	5	6	7	8
Assign Ø								
Min Grm								
Gap, Ext								
Max 1								
Max 2								
Yel Cir								
Red Cir								
Walk								
Ped Cir								

Alt# 1 Options Table [1.1.6.2.1]								
Column # >	1	2	3	4	5	6	7	8
Assign Ø								
Lock Calls	On							
Soft Recall								
Dual Entry	On							
Enabl SimGap	On							
Guar Passage								
Rest In Walk								
Cond Service								
Reservice								
Non-Act 1								
Red Rest								
Max2								
Ped Delay								
Conflicting Ø1								

Alt# 1 Veh Parameters [5.5.1.1]

Column#....>	1	2	3	4	5	6	7	8
Assign Dft#								
Call								
Switch								
Delay								
Extend								
Queue								
No Activity								
Max Presence								
Erratic Count								
Fall Time								

Column#....>	1	2	3	4	5	6	7	8
Assign Dft#								
Call								
Extend								
Queue								
Added Initial								
Red Lock								
Yellow Lock								
Occupancy								
Volume								

Column#....>	1	2	3	4	5	6	7	8
Assign Dft#								
Occ-on-green								
Occ-on-yellow								
Occ-on-red								
Delay Phase 1								
Delay Phase 2								
Detector Mode	NORM							
Source								

Column#....>	1	2	3	4	5	6	7	8
Assign Dft#								
Call								
No Activity								
Max Presence								
Erratic Count								

Column # >	1	2	3	4	5	6	7	8
Assign Ø								
Cond Service								
Reservice								
Non-Act 1								
Red Rest								
Max2								
Ped Delay								
Conflicting Ø1								

Alt# 1 Veh Parameters [5.5.1.2]

4/1/2019 Page 5

ID: 5086 RTE 303 @ MOUNTAIN VIEW AVE

Alt# 2 Options Table [1.1.6.2.2]

Column #	>	1	2	3	4	5	6	7	8
Assign Ø									
Lock Calls	On								
Soft Recall									
Dual Entity									
Enabl SimGap	On								
Guar Passage									
Rest In Walk									
Cond Service									
Reservice									
Non-Act 1									
Red Rest									
Max2									
Ped Delay									
Conflicting Ø1									

Alt# 3 Options Table [1.1.6.2.3]

Column #	>	1	2	3	4	5	6	7	8
Assign Ø									
Lock Calls	On								
Soft Recall									
Dual Entity									
Enabl SimGap	On								
Guar Passage									
Rest In Walk									
Cond Service									
Reservice									
Non-Act 1									
Red Rest									
Max2									
Ped Delay									
Conflicting Ø1									

Alt# 2 Veh Parameters [5.5.2.1]

Column#.....>	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Assign Det# Call																
Switch																
Delay																
Extend																
Queue																
No Activity																
Max Presence																
Erratic Count																
Fail Time																

Alt# 2 Veh Options [5.5.2.2]

Column#.....>	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Assign Det# Call																
Extend																
Queue																
Added Initial																
Red Lock																
Yellow Lock																
Occupancy																
Volume																

Alt# 2 Veh Parameters+ [5.5.2.3]

Column#.....>	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Assign Det#																
Occ-on-green																
Occ-on-yellow																
Occ-on-red																
Delay Phase1																
Delay Phase2																
Detector Mode	NORM															
Source																

Alt# 2 Ped Parameters+ [5.5.2.4]

Column#.....>	1	2	3	4	5	6	7	8
Assign Det# Call								
Extend								
Queue								
No Activity								
Max Presence								
Erratic Count								

Alt# 4 Options Table [1.1.6.2.4]

Column #	>	1	2	3	4	5	6	7	8
Assign Ø									
Lock Calls	On								
Soft Recall									
Dual Entity									
Enabl SimGap	On								
Guar Passage									
Rest In Walk									
Cond Service									
Reservice									
Non-Act 1									
Red Rest									
Max2									
Ped Delay									
Conflicting Ø1									

Day Plans [4:4]

Day Plan 1

	Hour	Min	Act	Act#	Pad#	A1	A2	A3	S1	S2	S3	S4	S5	S6	S7	S8	Pad#	Off Dpt	On Time	Dft	Max2												
1	6	30	1	9	0	1	9	0	1	9	0	1	9	0	1	1													1			DFT	On
2	18	30	99	10	0	2	18	0	99	10	0	2	10	0	0	1	0	0	0	0	2	2										DFT	DFT
3	0	0	99	11	0	3	0	0	99	11	0	3	11	0	0	1	0	0	0	0	3	3									DFT	DFT	
4				12	0	4				12	0	4	12	0	0	4	4												4			DFT	DFT
5				13	0	5				13	0	5	13	0	0	5	5												5			DFT	DFT
6				14	0	6				14	0	6	14	0	0	6	6												6			DFT	DFT
7				15	0	7				15	0	7	15	0	0	7	7												7			DFT	DFT
8				16	0	8				16	0	8	16	0	0	8	8												8			DFT	DFT

Day Plan 4

	Hour	Min	Act	Pad#	Off Dpt	On Time	Dft	Max2																									
1	0	0	0	9	0	0	0	1	0	0	9	0	0	1	0	0	0	9	0	0	11	11									DFT	DFT	
2	0	0	10	0	0	2	0	0	10	0	0	2	0	0	2	0	0	0	0	0	12	12									DFT	DFT	
3	0	0	11	0	0	3	0	0	11	0	0	3	0	0	3	0	0	0	11	0	0	13	13									DFT	DFT
4	0	0	12	0	0	4	0	0	12	0	0	4	0	0	4	0	0	0	12	0	0	14	14									DFT	DFT
5	0	0	13	0	0	5	0	0	13	0	0	5	0	0	5	0	0	0	13	0	0	15	15									DFT	DFT
6	0	0	14	0	0	6	0	0	14	0	0	6	0	0	6	0	0	0	14	0	0	16	16									DFT	DFT
7	0	0	15	0	0	7	0	0	15	0	0	7	0	0	7	0	0	0	15	0	0	17	17									DFT	DFT
8	0	0	16	0	0	8	0	0	16	0	0	8	0	0	8	0	0	0	16	0	0	18	18									DFT	DFT

Day Plan 7

	Hour	Min	Act	Pad#	Off Dpt	On Time	Dft	Max2																								
1	0	0	0	9	0	0	0	1	0	0	9	0	0	1	0	0	0	9	0	0	11	11									DFT	DFT
2	0	0	10	0	0	2	0	0	10	0	0	2	0	0	2	0	0	0	10	0	0	21	21								DFT	DFT
3	0	0	11	0	0	3	0	0	11	0	0	3	0	0	3	0	0	0	11	0	0	23	23								DFT	DFT
4	0	0	12	0	0	4	0	0	12	0	0	4	0	0	4	0	0	0	12	0	0	24	24								DFT	DFT
5	0	0	13	0	0	5	0	0	13	0	0	5	0	0	5	0	0	0	13	0	0	25	25								DFT	DFT
6	0	0	14	0	0	6	0	0	14	0	0	6	0	0	6	0	0	0	14	0	0	26	26								DFT	DFT
7	0	0	15	0	0	7	0	0	15	0	0	7	0	0	7	0	0	0	15	0	0	27	27								DFT	DFT
8	0	0	16	0	0	8	0	0	16	0	0	8	0	0	8	0	0	0	16	0	0	28	28								DFT	DFT

Day Plan 8

	Hour	Min	Act	Pad#	Off Dpt	On Time	Dft	Max2																								
1	0	0	0	9	0	0	0	1	0	0	9	0	0	1	0	0	0	9	0	0	31	31								DFT	DFT	
2	0	0	10	0	0	2	0	0	10	0	0	2	0	0	2	0	0	0	10	0	0	32	32								DFT	DFT
3	0	0	11	0	0	3	0	0	11	0	0	3	0	0	3	0	0	0	11	0	0	33	33								DFT	DFT
4	0	0	12	0	0	4	0	0	12	0	0	4	0	0	4	0	0	0	12	0	0	34	34								DFT	DFT
5	0	0	13	0	0	5	0	0	13	0	0	5	0	0	5	0	0	0	13	0	0	35	35								DFT	DFT
6	0	0	14	0	0	6	0	0	14	0	0	6	0	0	6	0	0	0	14	0	0	36	36								DFT	DFT
7	0	0	15	0	0	7	0	0	15	0	0	7	0	0	7	0	0	0	15	0	0	37	37								DFT	DFT
8	0	0	16	0	0	8	0	0	16	0	0	8	0	0	8	0	0	0	16	0	0	38	38								DFT	DFT

Day Plan 9

	Hour	Min	Act	Pad#	Off Dpt	On Time	Dft	Max2																								
1	0	0	0	9	0	0	0	1	0	0	9	0	0	1	0	0	0	9	0	0	39	39								DFT	DFT	
2	0	0	10	0	0	2	0	0	10	0	0	2	0	0	2	0	0	0	10	0	0	40	40								DFT	DFT
3	0	0	11	0	0	3	0	0	11	0	0	3	0	0	3	0	0	0	11	0	0	41	41								DFT	DFT
4	0	0	12	0	0	4	0	0	12	0	0	4	0	0	4	0	0	0	12	0	0	42	42								DFT	DFT
5	0	0	13	0	0	5	0	0	13	0	0	5	0	0	5	0	0	0	13	0	0	43	43								DFT	DFT
6	0	0	14	0	0	6	0	0	14	0	0	6	0	0	6	0	0	0	14	0	0	44	44								DFT	DFT
7	0	0	15	0	0	7	0	0	15	0	0	7	0	0	7	0	0	0	15	0	0	45	45								DFT	DFT
8	0	0	16	0	0	8	0	0	16	0	0	8	0	0	8	0	0	0	16	0	0	46	46								DFT	DFT

Day Plan 10

| | Hour | Min | Act | Pad# | Off Dpt | On Time | Dft | Max2 |
| --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- |

<tbl_r

C1-USER IO Map [1.8.9.1In]		C1-USER IO Map [1.8.9.2Out]		IO Logic [1.8.7]		Fan Oper		Fan Oper		Fan Timer	
				Result							
11-1	1	Veh Call 1	01-1	1	Ch1 Red	01	=	0	---	0	---
11-2	2	Veh Call 2	01-2	49	Ch1 Green	07-2	115	Not Used	1	---	0
11-3	3	Veh Call 3	01-3	2	Ch2 Red	07-3	115	Not Used	1	---	0
11-4	189	Unused	01-4	26	Ch2 Yellow	07-4	115	Not Used	1	---	0
11-5	5	Veh Call 5	01-5	50	Ch2 Green	07-5	115	Not Used	1	---	0
11-6	6	Veh Call 6	01-6	3	Ch3 Red	07-6	115	Not Used	1	---	0
11-7	189	Unused	01-7	27	Ch3 Yellow	07-7	115	Not Used	1	---	0
11-8	189	Unused	01-8	51	Ch3 Green	07-8	115	Not Used	1	---	0
12-1	189	Unused	02-1	4	Ch4 Red	14-1	189	Unused	1	---	0
12-2	189	Unused	02-2	52	Ch4 Green	14-2	189	Unused	1	---	0
12-3	11	Veh Call 11	02-3	5	Ch5 Red	14-3	189	Unused	1	---	0
12-4	12	Veh Call 12	02-4	29	Ch5 Yellow	14-4	189	Unused	1	SWLOAD	43
12-5	13	Veh Call 13	02-5	53	Ch5 Green	17-1	189	Unused	2	SECURE	44
12-6	189	Unused	02-6	6	Ch6 Red	17-2	189	Unused	3	NONE	45
12-7	15	Veh Call 15	02-7	30	Ch6 Yellow	17-3	189	Unused	4	NONE	46
12-8	16	Veh Call 16	02-8	54	Ch6 Green	17-4	189	Unused	5	NONE	47
13-1	189	Unused	03-1	7	Ch7 Red	17-5	189	Unused	6	NONE	48
13-2	189	Unused	03-2	55	Ch7 Green	17-6	189	Unused	7	NONE	49
13-3	189	Unused	03-3	8	Ch8 Red	17-7	189	Unused	8	NONE	50
13-4	189	Unused	03-4	32	Ch8 Yellow	17-8	189	Unused	9	NONE	51
13-5	189	Unused	03-5	56	Ch8 Green	18-1	189	Unused	10	NONE	52
13-6	129	Ped Call 1	03-6	9	Ch9 Red	18-2	189	Unused	11	NONE	53
13-7	23	Veh Call 23	03-7	33	Ch9 Yellow	18-3	189	Unused	12	NONE	54
13-8	189	Unused	03-8	57	Ch9 Green	18-4	189	Unused	13	NONE	55
14-1	10	Ch10 Red	04-1	10	Ch10 Green	18-5	189	Unused	14	NONE	56
14-2	58	Ch10 Green	04-3	11	Ch11 Red	18-6	189	Unused	15	NONE	57
14-3	35	Ch11 Yellow	04-4	35	Ch11 Green	18-7	189	Unused	16	NONE	58
14-4	59	Ch11 Green	04-5	12	Ch12 Red	18-8	189	Unused	17	NONE	59
14-5	189	Unused	04-6	36	Ch12 Yellow	08-1	115	Not Used	18	NONE	60
14-6	189	Unused	04-7	60	Ch12 Green	08-2	115	Not Used	19	NONE	61
14-7	229	33xCMUStop	05-1	28	Ch4 Yellow	08-3	115	Not Used	20	NONE	62
14-8	228	33xFlashSns	05-2	34	Ch10 Yellow	08-4	115	Not Used	21	NONE	63
15-1	130	Ped Call 2	05-3	25	Ch11 Yellow	08-5	115	Not Used	22	NONE	64
15-2	189	Unused	05-4	31	Ch13 Red	08-6	115	Not Used	23	NONE	65
15-3	189	Unused	05-5	115	Not Used	08-7	115	Not Used	24	NONE	66
15-4	189	Unused	05-6	115	Not Used	08-8	115	Not Used	25	NONE	67
15-5	189	Unused	05-7	115	Not Used	08-9	115	Not Used	26	NONE	68
15-6	189	Unused	05-8	115	Not Used	08-10	115	Not Used	27	NONE	69
15-7	189	Unused	05-9	115	Not Used	08-11	115	Not Used	28	NONE	70
15-8	189	Unused	06-1	115	Not Used	08-12	115	Not Used	29	NONE	71
16-1	189	Unused	06-2	115	Not Used	08-13	115	Not Used	30	NONE	72
16-2	189	Unused	06-3	13	Ch13 Red	08-14	115	Not Used	31	NONE	73
16-3	189	Unused	06-4	37	Ch13 Yellow	08-15	115	Not Used	32	NONE	74
16-4	189	Unused	06-5	61	Ch13 Green	08-16	115	Not Used	33	NONE	75
16-5	189	Unused	06-6	14	Ch14 Red	08-17	115	Not Used	34	NONE	76
16-6	189	Unused	06-7	38	Ch14 Yellow	08-18	115	Not Used	35	NONE	77
16-7	189	Unused	06-8	62	Ch14 Green	08-19	115	Not Used	36	NONE	78

#	Event / Alarm	Ev / Air	Call Phases[1.1.5]	Redirect Phases[1.1.5]	From	To	From	To	From	To	From	To	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
1	Power Up Alarm.	On	On	∅	Phases Called By ∅	1							1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
2	Stop Timing	On	On	1		2							2															
3	TS1 Cabinet Door			2									3															
4	Coordination Failure	On	On	3									4															
5	External Alarm # 1	On	On	4									5															
6	External Alarm # 2	On	On	5									6															
7	External Alarm # 3			6									7															
8	External Alarm # 4			7									8															
9	Closed Loop Disabled	On	On	8									9															
10	External Alarm # 5			9									10															
11	External Alarm # 6			10									11															
12	Manual Control Enable	On	On	11									12															
13	Coord Free Input			12									13															
14	Local Flash Input	On	On	13									14															
15	MMU Flash			14									15															
16	CMU Flash			15									16															
17	Cycle Fault	On	On	16																								
18	Cycle Failure	On	On																									
19	Coordination Fault	On	On																									
20	Controller Fault	On	On																									
21	Detector SDLC Failure																											
22	MMU SDLC Failure																											
23	Critical SDLC Failure																											
24	Reserved																											
25	EEPROM CRC Fault	On	On																									
26	Defector Diagnostic Failure	On	On																									
27	BIU Defector Failure	On	On																									
28	Queue defector alarm	On	On																									
29	Ped Detector Fault	On	On																									
30	Coord Diagnostic Fault																											
41	TempAlert Probe Ch. A																											
42	TempAlert Probe Ch. B																											
47	Coord Active																											
48	Preempt Active	On	On																									
49	Preempt 1 Input	On	On																									
50	Preempt 2 Input	On	On																									
51	Preempt 3 Input	On	On																									
52	Preempt 4 Input	On	On																									
53	Preempt 5 Input	On	On																									
54	Preempt 6 Input	On	On																									
55	Preempt 7 Input	On	On																									
56	Preempt 8 Input	On	On																									
57	Preempt 9 Input	On	On																									
58	Preempt 10 Input	On	On																									
61	In Transition	On	On																									
81	FIO Status Alarm																											

Inhibit Phases[1.1.5]
Alt Inhibit Phases # 1 [1.1.6.3]
Alt Call & Redirect # 1 [1.1.6.3]
Alt Call & Redirect # 2 [1.1.6.3]
Unit Parameters [1.2.1]
Auto Flash Phase/Olap Settings [1.4.2]
ID: 5086 RTE 303 @ MOUNTAIN VIEW AVE
04/01/19 **Page 10**

**MODEL 179 SIGNAL OPERATION
PROGRAMMABLE FEATURES
SIGNAL OPERATION SPECIFICATION**

TAPS _____
STUDY # _____
FILE # _____
PAGE 18 OF 20

SIGNAL # R-86COUNTY # Rockland

DATE _____

SWITCH PACK	FUNCTION	INDICATIONS	FACE	TERMINAL WIRING BOARD		FACE	TERMINAL WIRING BOARD	
				TERMINAL	WIRE COLOR CODE		TERMINAL	WIRE COLOR CODE
1	Ø1	 Ground Wire	1	SP 1 R	-----	4	SP 1 R	
				SP 1 Y	14/10c-D-O/B		SP 1 Y	
				SP 1 G	-G/B		SP 1 G	
				Grnd Bus	-W/B		Grnd Bus	
2	Ø2	Red Yellow Green Ground Wire	3	SP 2 R	14/10c-B-R	6	SP 2 R	14/5c-A-R
				SP 2 Y	-O		SP 2 Y	-O
				SP 2 G	-G		SP 2 G	-G
				Grnd Bus	-W		Grnd Bus	-W
3	Ø3	Red Yellow Green Ground Wire	5	SP 3 R	14/5c-R	2	SP 3 R	14/15c-R/B
				SP 3 Y	-O		SP 3 Y	-O/B
				SP 3 G	-G		SP 3 G	-G/B
				Grnd Bus	-W		Grnd Bus	-W/B
4		Ground Wire		SP 4 R			SP 4 R	
				SP 4 Y			SP 4 Y	
				SP 4 G			SP 4 G	
				Grnd Bus			Grnd Bus	
5	Ø5	Red Yellow Green Ground Wire	1	SP 5 R	14/10c-D-R	2	SP 5 R	14/15c-E-R
				SP 5 Y	-O		SP 5 Y	-O
				SP 5 G	-G		SP 5 G	-G
				Grnd Bus	-W		Grnd Bus	-W
6	Ø6	 Ground Wire	3	SP 6 R	-----		SP 6 R	
				SP 6 Y	14/10c-B-O/B		SP 6 Y	
				SP 6 G	-G/B		SP 6 G	
				Grnd Bus	-W/B		Grnd Bus	
7		Ground Wire		SP 7 R			SP 7 R	
				SP 7 Y			SP 7 Y	
				SP 7 G			SP 7 G	
				Grnd Bus			Grnd Bus	
8		Ground Wire		SP 8 R			SP 8 R	
				SP 8 Y			SP 8 Y	
				SP 8 G			SP 8 G	
				Grnd Bus			Grnd Bus	
9	Ped "A" Ø2	Don't Walk ----- Walk Ground Wire	24	SP 9 R	14/5c-1P-R		SP 9 R	
				SP 9 Y	-----		SP 9 Y	
				SP 9 G	-G		SP 9 G	
				Grnd Bus	-W		Grnd Bus	
10	Ped "B" Ø3	Don't Walk ----- Walk Ground Wire	23	SP 10 R	14/5c-2P-R		SP 10 R	
				SP 10 Y	-----		SP 10 Y	
				SP 10 G	-G		SP 10 G	
				Grnd Bus	-W		Grnd Bus	
11		Ground Wire		SP 11 R			SP 11 R	
				SP 11 Y			SP 11 Y	
				SP 11 G			SP 11 G	
				Grnd Bus			Grnd Bus	
12		Ground Wire		SP 12 R			SP 12 R	
				SP 12 Y			SP 12 Y	
				SP 12 G			SP 12 G	
				Grnd Bus			Grnd Bus	
13	Ø3	Red Yellow Green Ground Wire	7	SP 13 R	14/15c-E-R/W	8	SP 13 R	14/10c-C-R/B
				SP 13 Y	-BL/W		SP 13 Y	-O/B
				SP 13 G	-G/W		SP 13 G	-G/B
				Grnd Bus	-B/W		Grnd Bus	-W/B
14		Ground Wire		SP 14 R			SP 14 R	
				SP 14 Y			SP 14 Y	
				SP 14 G			SP 14 G	
				Grnd Bus			Grnd Bus	

**MODEL 179 SIGNAL OPERATION
PROGRAMMABLE FEATURES
SIGNAL OPERATION SPECIFICATION**

TAPS _____
STUDY # _____
FILE # _____
PAGE 19 OF 20

SIGNAL # R-86COUNTY # Rockland

DATE _____

Conflict / Current Monitor Programming

Diodes to be Cut			CONFLICT MONITOR YELLOW JUMPERS TO BE INSTALLED	CURRENT MONITOR DIODES TO BE CUT
Sp1-Sp5			Sp-9	1,4,6-12,14
Sp1-Sp6			Sp-10	
Sp1-Sp9				
Sp2-Sp5				
Sp2-Sp6				
Sp2-Sp9				
Sp3-Sp13				
Sp3-Sp10				
Sp5-Sp9				
Sp6-Sp9				
Sp10-Sp13				

NOTE : _____

TE 262-14 (7/91)

**MODEL 179 SIGNAL OPERATION
PROGRAMMABLE FEATURES
SIGNAL OPERATION SPECIFICATION**

TAPS _____
 STUDY # _____
 FILE # _____
 PAGE 20 OF 20

SIGNAL # R-86COUNTY # Rockland

DATE _____

TABLE OF INPUT WIRING

TERM. NUMBER	FUNCTION	DET. NO	DET. TYPE	DET. AN OVER	REMARKS
1A, 1B	Ø1	1A,1B	Quadrupole		Presence Loop
2A, 2B	Ø2	2A,2B	Quadrupole		Presence Loop
3A, 3B	Ø3	3A,3B	Quadrupole		Presence Loop
4A, 4B					
5A, 5B	Ø5	5A,5B	Quadrupole		Presence Loop
6A, 6B	Ø6	6A,6B	Quadrupole		Presence Loop
7A, 7B					
8A, 8B					
9A, 9B					
10A, 10B					
11A, 11B	Ø1	11A,11B	Normal		Presence Loop
12A, 12B	Ø2	12A,12B	Normal		Presence Loop
13A, 13B	Ø3	13A,13B	Quadrupole		Presence Loop
14A, 14B					
15A, 15B					
16A, 16B					
17A, 17B					
18A, 18B					
19A, 19B					
20A, 20B					
21A, 21B					
22A, 22B	Ped "A" Ø2	22	Button		Pedestrian
23A, 23B	Ø3	23	Normal		Presence Loop
24A, 24B					
25A, 25B	Ped "B" Ø3	25	Button		Pedestrian
26A, 26B					
27A, 27B					
28A, 28B					

Office

STATE OF NEW YORK – DEPARTMENT OF TRANSPORTATION
TRAFFIC ENGINEERING SAFETY DIVISION
TRAFFIC CONTROL SPECIFICATION

R - 102

SIGNAL NO.

ROCKLAND

COUNTY

Study:

Contract:

PIN: HWP# 20110825913

File: 37.07-9

INTERSECTION

ROUTE 303 @ ORANGEBURGH ROAD

CITY VILLAGE TOWN OF ORANGETOWN

Department Order filed 08/15/75 as Section: 2039.15 Subdivision: (r)

Prior specification hereby superseded None Dated: JULY 29, 1983

Purpose: MODIFY BY ADDING CROSSWALKS AND BI MODAL ARROWS

These specifications will be effective upon the Installation Modification / Reinstallation of the necessary traffic control device(s) required by and conforming to the Federal Manual on Uniform Traffic Control Devices.

This signal shall

- A. Operate in accordance with the table of operations and / or change intervals as shown on the attached pages as a:

Pretimed Signal
 Semi-traffic actuated
 Full-traffic actuated
 Pedestrian actuated

- B. Display vehicular indications
 Display pedestrian indications
 Be equipped with vehicle detectors
 Be equipped with pedestrian buttons

as shown in the attached plans / drawings.

- C. Be equipped with Pre-emption Interconnection and/or coordination which are described as follows:

Description: SIGNAL CHANGING FROM P
TO PS SM # 80926

- cc: Region 8 Traffic Engineer
 Signal Shop
 Contract Maintainer
 Main Office

11/28/12

Date

Installation Date

Michael Cotton
R.T.E.

Signature

12/03
Title
11/28/12

Reinstallation/Modification

STATE OF NEW YORK - DEPARTMENT OF TRANSPORTATION
 TRAFFIC AND SAFETY DIVISION
 TRAFFIC CONTROL SIGNAL SPECIFICATIONS (CONTINUED)

STUDY:
 CONTRACT:
 PIN:
 FILE:

R-102

SIGNAL NO(S)

Rockland
COUNTY

10-2012
DATE

PAGE _____ OF _____ PAGES

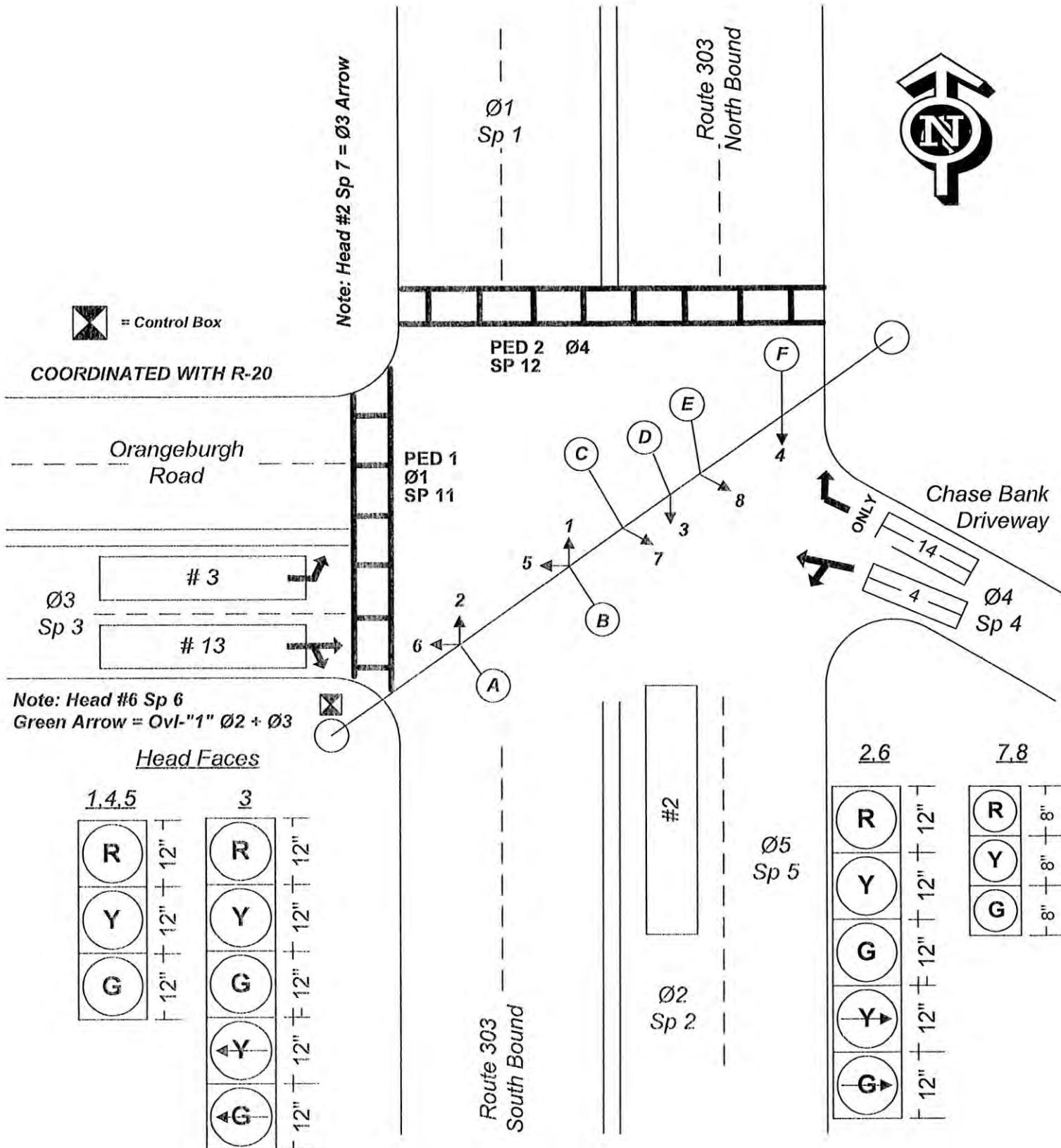
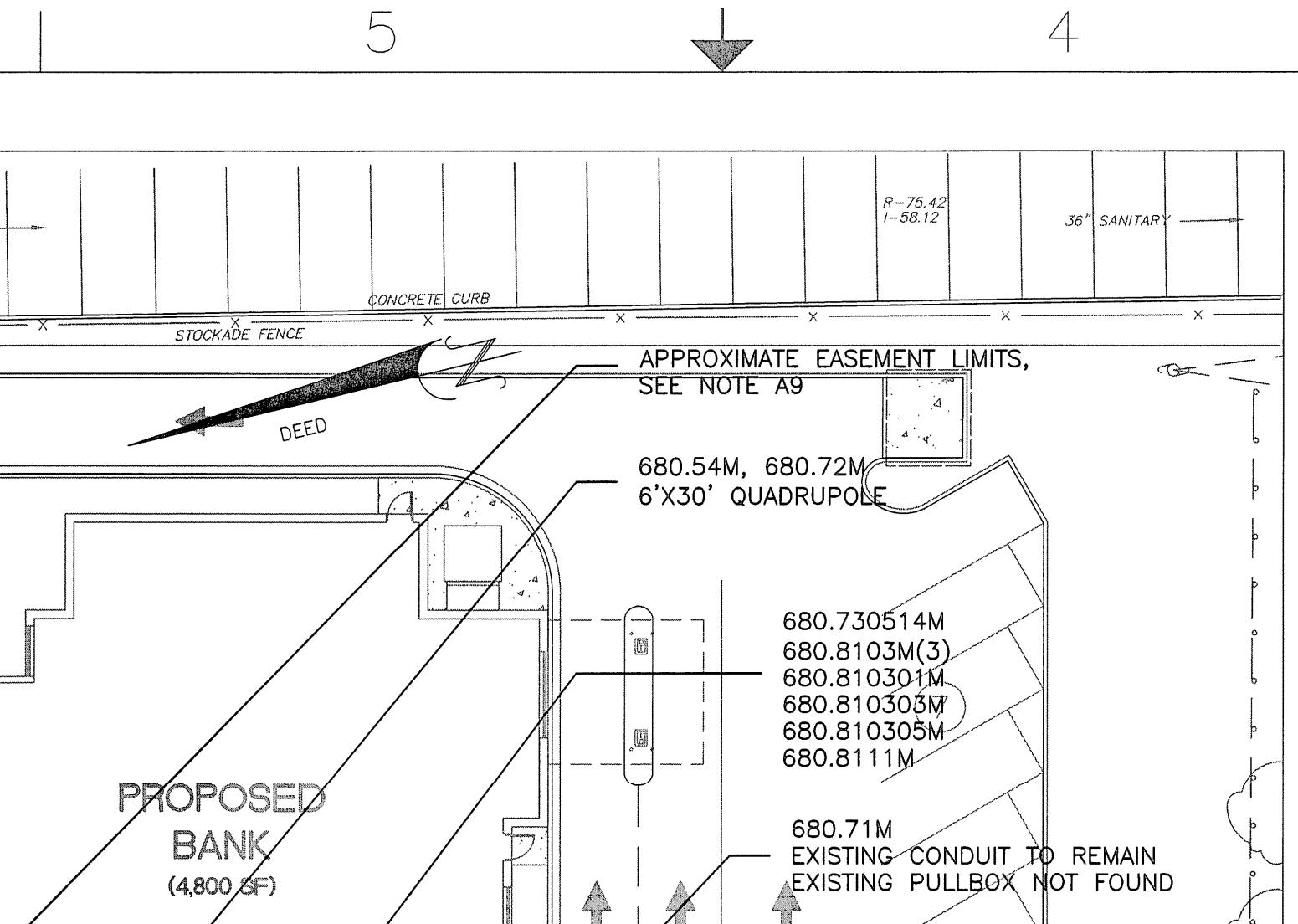
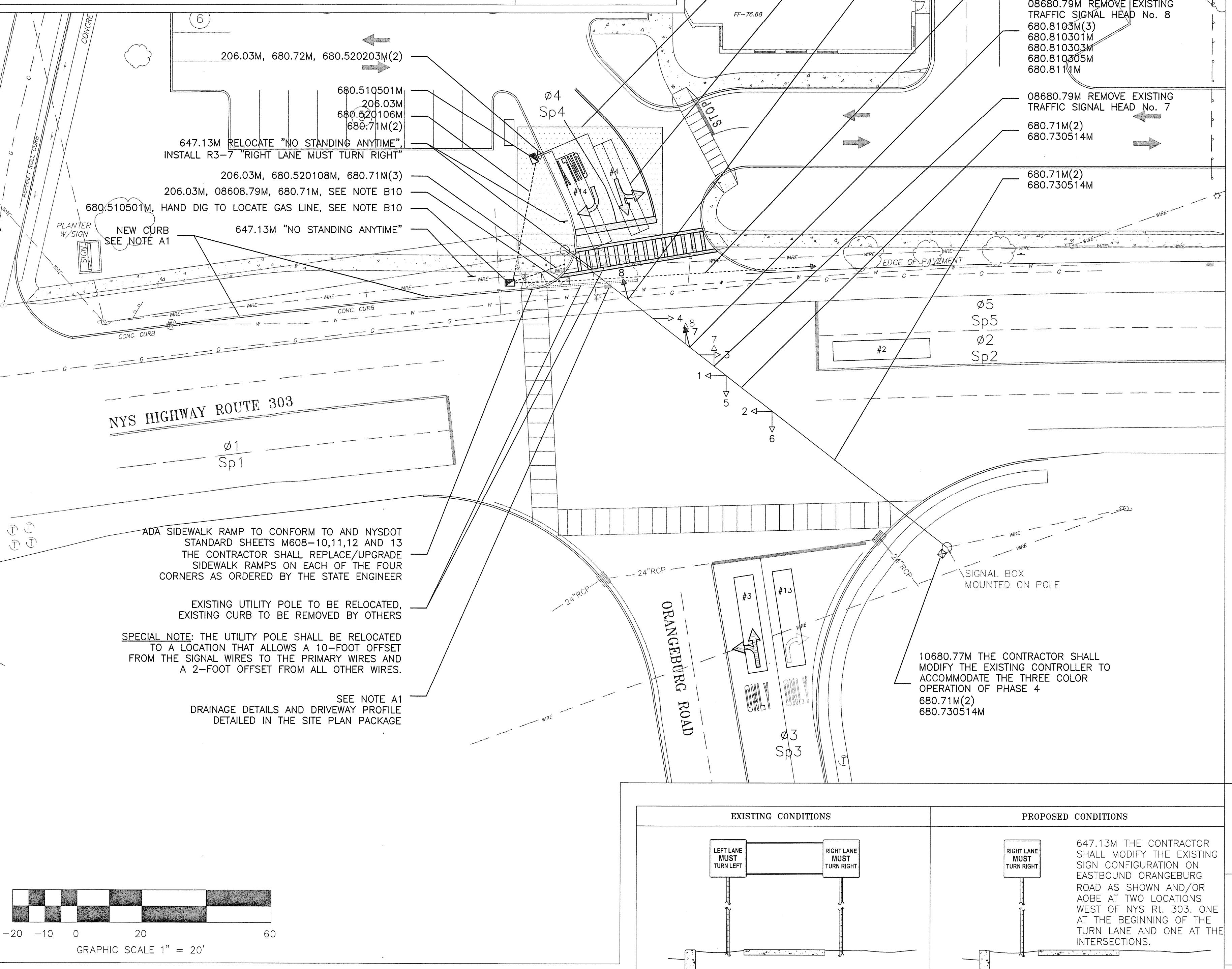


TABLE OF OPERATION

FACE	1 12"	2 12"	3 12"	4 12"	5 12"	6 12"	7,8 8"
	(R) G						
PHASE							
Ø1	G	G/→	R	R	R	R	R
Ø2	R	R	R/←	R	R	R/→	R
Ø5	R	R	G	G	R	R	R
Ø1 + Ø5	G	G/→	G	G	R	R	R
Ø2 + Ø5	R	R	G/←	G	R	R/→	R
Ø3	R	R/→	R	R	G	G/→	R
Ø4	R	R	R	R	R	R	G
FLASH	FL Y	FL Y	FL Y	FL R	FL R	FL R	FL R

O = 8" SIGNAL SECTION

□ = 12" SIGNAL SECTION



1. THIS PLAN HAS BEEN PREPARED TO SUPPLEMENT INFORMATION SHOWN ON THE SITE PLAN BY PENNONI ENGINEERING AND SURVEYING OF NEW YORK, P.C., DATED AUGUST 04, 2006 LAST REVISED SEPTEMBER 03, 2008. THE CONTRACTOR SHALL REFER TO THE SITE PLAN PACKAGE FOR CONSTRUCTION DETAILS.
2. THIS PLAN PROVIDES CONSTRUCTION DETAILS RELATED TO THE TRAFFIC SIGNAL AND PAVEMENT MARKING MODIFICATIONS. TRAFFIC ENGINEERING ANALYSIS FOR THIS PROJECT WAS PREPARED BY JOSEPH STAIGAR ENGINEERING, LLC.
3. THE SITE IS LOCATED THE SUBJECT SITE IS LOCATED AT MILE MARKER: 303-8501-1022.
4. ALL WORK SHALL BE DONE IN ACCORDANCE WITH THE NYSDOT STANDARD AND DETAIL SHEETS. THE CONTRACTOR IS RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS.
5. THE CONTRACTOR SHALL VERIFY THE LOCATION OF PROPERTY LINES ADJACENT TO THE PROPOSED WORK PRIOR TO THE START OF CONSTRUCTION.
6. PRIOR TO THE START OF ANY WORK THE CONTRACTOR SHALL REQUEST A UTILITY MARK-OUT. THE CONTRACTOR SHALL VERIFY THE LOCATION OF UNDERGROUND AND OVERHEAD UTILITIES AND NOTIFY THE NEW YORK STATE DEPARTMENT OF TRANSPORTATION ENGINEER IN CHARGE OF ANY CONFLICTS PRIOR TO THE START OF CONSTRUCTION.
7. THE PERMITTEE SHALL CONTACT MR. RAY NOVAK FROM NYSDOT AT 914.941.4475 FOR A MARK OUT OF THE TRAFFIC SIGNAL EQUIPMENT PRIOR TO WORKING IN THE NYSDOT RIGHT OF WAY.
8. THE CONTRACTOR SHALL RESTORE ALL DISTURBED AREAS IN-KIND (INCLUDING: CURB, SIDEWALK, ASPHALT, SOD, ETC.).
9. THE APPLICANT SHALL PROVIDE AN EASEMENT TO THE NYSDOT FOR THE MAINTAIN OF THE TRAFFIC SIGNAL EQUIPMENT LOCATED ON PRIVATE PROPERTY WITHIN THE DRIVEWAY AREA.
10. THE CONTRACTOR SHALL COORDINATE A MEETING WITH THE NEW YORK STATE DEPARTMENT OF TRANSPORTATION AND O&R AND ALL OTHER INVOLVED UTILITIES PRIOR TO THE RELOCATION OF THE UTILITY POLE.

B. TRAFFIC SIGNAL NOTES:

1. ALL WORK SHALL BE DONE IN ACCORDANCE WITH THE NYSDOT REGIONAL TRAFFIC SIGNAL DETAIL SHEETS AND NYSDOT STANDARD SHEETS.
2. PRIOR TO BEGINNING ANY WORK ON THE CONTRACT, THE CONTRACTOR SHALL NOTIFY THE REGIONAL TRAFFIC SIGNAL SECTION TO PERFORM AN INSPECTION OF THE EXISTING TRAFFIC SIGNAL EQUIPMENT. AFTER THE INSPECTION, THE CONTRACTOR MUST THEN SUBMIT A WRITTEN NOTIFICATION WITH THE DATE THEY WILL ASSUME TRAFFIC SIGNAL MAINTENANCE. NO WORK SHALL PROCEED ON THE CONTRACT UNTIL TRAFFIC SIGNAL MAINTENANCE IS ASSUMED. THE EXISTING TRAFFIC SIGNAL SHALL BE MAINTAINED UNDER ITEM 619.1613 M EXCEPT FOR THE CONTROLLER, PROGRAMMING AND TIMING WHICH SHALL BE MAINTAINED BY THE STATE.
3. ALL TRAFFIC SIGNALS SHALL BE MAINTAINED IN A TRAFFIC RESPONSIVE OPERATION AND ALL INTERCONNECT, WHERE EXISTING, SHALL BE MAINTAINED. THE CONTRACTOR SHALL PAY A LIQUIDATED DAMAGE CHARGE OF \$500.00 PER CALENDAR DAY IF ACTUATION AND COORDINATION IS NOT MAINTAINED.
4. ALL VEHICLE DETECTION AND SIGNAL COORDINATION MUST BE MAINTAINED AT ALL TIMES. SPLICING OF INDUCTANCE LOOP WIRE, SHIELDED LEAD-IN CABLE, AND INTERCONNECT CABLE SHALL BE ALLOWED FOR TEMPORARY REPAIRS DURING CONSTRUCTION ONLY. TEMPORARY LOOP DETECTORS, IF NECESSARY, SHALL BE 1.8 X 1.8 M, BE CENTERED IN THEIR RESPECTIVE LANE, AND PAID FOR UNDER ITEMS 680.54 M AND 680.72 M.
5. TRAFFIC SIGNAL TURN-ON SHALL BE DONE BY NYSDOT TRAFFIC SIGNAL PERSONNEL ONLY. THE CONTRACTOR SHALL PAY A LIQUIDATED DAMAGE CHARGE OF \$10,000.00 IF THE TRAFFIC SIGNAL IS TURNED ON WITHOUT NYSDOT TRAFFIC SIGNAL PERSONNEL PRESENT. THE CONTRACTOR SHALL NOTIFY THE NYSDOT TRAFFIC SIGNAL SECTION TWO WEEKS PRIOR TO THE REQUESTED DATE OF TURN-ON.
6. ALL DETECTORS SHALL BE CENTERED IN THEIR RESPECTIVE LANES AND SPACED 3 M APART UNLESS OTHERWISE INDICATED IN THE PLAN.
7. WHERE PAVEMENT IS RESURFACED, ALL INDUCTANCE LOOPS SHALL BE INSTALLED PRIOR TO THE INSTALLATION OF THE FINAL ASPHALT TOP COURSE.
8. EQUIPMENT SHALL BE DISPOSED OF BY THE CONTRACTOR AS DIRECTED BY THE NEW YORK STATE DEPARTMENT OF TRANSPORTATION ENGINEER IN CHARGE.
9. EXISTING TRAFFIC SIGNAL EQUIPMENT SHALL REMAIN UNCHANGED UNLESS OTHERWISE NOTED.
10. THE CONTRACTOR SHALL REMOVE AND REPLACE THE EXISTING PULLBOX. THE CONTRACTOR SHALL INTERCEPT THE EXISTING CONDUIT FROM THE POLE FOUNDATION AND RUN NEW CONDUIT TO THE RELOCATED PULLBOX. THE CONTRACTOR SHALL EXTEND THE EXISTING CONDUIT AND REINSTALL SHIELDED LEAD-IN CABLE RUNNING TO THE NORTHBOUND LOOP DETECTORS. THE SHIELDED LEAD-IN MAY NOT BE SPLICED.
11. THE CONTRACTOR SHALL SUPPLY TWO (2) ENCOM 5270 SPREAD SPECTRUM RADIOS TO COORDINATE SIGNALS R-102 AND R-20.
12. ALL MATERIALS INCORPORATED IN THE SIGNAL INSTALLATION SHALL CONFORM TO THE CURRENT NYSDOT REQUIREMENTS: NYSDOT STANDARD SPECIFICATIONS, REGION 8 SIGNAL DETAILS SHEETS, AND STANDARD STRUCTURE SHEETS. CONFORMANCE SHALL BE MET BY NYSDOT APPROVAL OF THE FOLLOWING SUBMISSION BY THE PERMITTEE/CONTRACTOR:

A. TRAFFIC SIGNAL POLES AND PEDESTRIAN POLES:
MANUFACTURER'S SHOP DRAWINGS AND CALCULATIONS MUST BE SUBMITTED TO NYSDOT FOR EACH SIGNAL POLE. THE SHOP DRAWINGS AND CALCULATIONS MUST BE STAMPED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF NEW YORK.

B. TRAFFIC SIGNAL HEADS (SECTIONS) AND BRACKET ASSEMBLIES, PEDESTRIAN HEADS & BRACKET ASSEMBLIES, CONCRETE PULLBOXES, CAST ALUMINUM JUNCTION BOXES, AND OVERHEAD SIGN ASSEMBLIES: MANUFACTURER'S CATALOG CUTS MUST BE SUBMITTED TO NYSDOT, AS WELL AS MANUFACTURER'S CERTIFICATION OF COMPLIANCE WITH "NYSDOT STANDARD SPECIFICATIONS".

C. TRAFFIC SIGNAL CONDUIT, CABLE, WIRE:
MANUFACTURER'S CERTIFICATION OF COMPLIANCE WITH "NYSDOT STANDARD SPECIFICATIONS".

D. TRAFFIC SIGNAL LOOP EMBEDDING SEALER:
ONLY THOSE PRODUCTS INCLUDED ON THE LATEST NYSDOT MATERIALS BUREAU "APPROVED LIST" SHALL BE USED.

C. MPT NOTES:

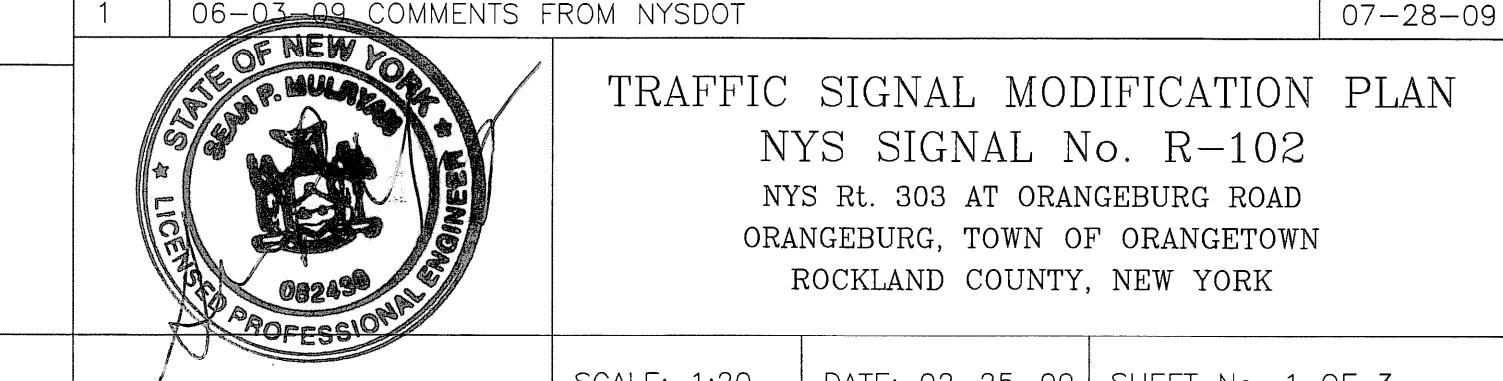
1. MAINTENANCE AND PROTECTION OF TRAFFIC SCHEMES ARE TO BE IN ACCORDANCE WITH THE FEDERAL MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES AND THE NEW YORK STATE SUPPLEMENT.
2. THE DESIGN, SPACING AND LOCATION OF SIGNS, CONES, FLAGGER, DRUMS, BARRICADES, FLASHING BEACONS, ETC., SHALL CONFORM TO THE REQUIREMENTS OF THE LATEST EDITION OF THE FEDERAL MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES AND THE NEW YORK STATE SUPPLEMENT.
3. THE CONTRACTOR IS RESPONSIBILITY TO SAFELY MAINTAIN AND PROTECT TRAFFIC IN ACCORDANCE WITH ITEM 619.01M.
4. NO DRUMS, CONES, BARRICADES AND OTHER TRAFFIC CONTROL EQUIPMENT SHALL REMAIN IN A LOCATION WHERE THEY INTERFERE WITH OR BE DISTURBED BY A SNOW PLOWING OPERATION. THE WORK MUST BE SCHEDULED TO AFFORD THE SAFE REMOVAL OF SUCH DEVICES WHEN NECESSARY.
5. DRAINAGE FRAMES, GRATES AND COVERS SHALL NOT BE ADJUSTED IN A TRAVEL LANE UNLESS THE FINAL PAVEMENT COURSE IS PLACED PRIOR TO THE ONSET OF SNOW AND ICE WEATHER. STEEL PLATES, ETC. SHALL NOT PROTRUDE ABOVE THE ADJACENT PAVEMENT. IF ANY OF THESE PROTRUSIONS EXIST IN A NON-TRAVEL LANE PRIOR TO A SNOW AND ICE CONDITION, THEN TEMPORARY ASPHALT RAMPS MUST BE PLACED SO THAT FOR EVERY ONE INCH OF RISE, THERE IS A SIX FOOT RUN OF RAMP.
6. IN ADDITION TO THE SIGNING REQUIRED IN THE MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES, THE CONTRACTOR SHALL USE A FLASHING ARROW BOARD, FOR ALL MAINLINE TRAVEL LANE CLOSING AND WILL POSITION IT AT THE BEGINNING OF THE TAPER.
7. THE CONTRACTOR SHALL PROVIDE FLAGGERS WHEN SIGHT DISTANCES ARE IMPAIRED BY HIS OPERATION OR AOE.
8. THE CONTRACTOR SHALL NOT PARK HIS EQUIPMENT OR STORE MATERIAL OVERNIGHT WHERE IT IS DEEMED BY THE ENGINEER TO BE A SAFETY HAZARD TO TRAFFIC. STORAGE WITHIN THE NYSDOT WILL NOT BE PERMITTED.
9. THE CONTRACTOR IS ADVISED THAT NO PAVEMENT EDGE DROP-OFFS WILL BE PERMITTED WITHIN TEN (10) FEET OF TRAVEL LANES. THE CONTRACTOR SHALL SCHEDULE HIS OPERATIONS SO THAT ALL EXCAVATED WORK AREAS SHALL BE BACKFILLED TO GRADE AT CONCLUSION OF THE WORK DAY.
10. THE CONTRACTOR SHALL NOT RESTRICT THE INGRESS/EGRESS REQUIRED FOR DRIVEWAYS ALONG NYS ROUTE 303.
11. ALL LANES MUST BE OPEN TO TRAFFIC BEFORE 9AM AND AFTER 3PM. NO LANE CLOSURES ARE PERMITTED ON WEEKENDS OR HOLIDAYS. NIGHTTIME CLOSINGS WILL NOT BE PERMITTED WITHOUT PRIOR FROM THE STATE PERMIT INSPECTOR. AT OTHER TIMES, ONE LANE MAY BE CLOSED IN EACH DIRECTION. THE CONTRACTOR WILL NOT BE PERMITTED TO CLOSE MORE THAN ONE LANE IN ANY DIRECTION.

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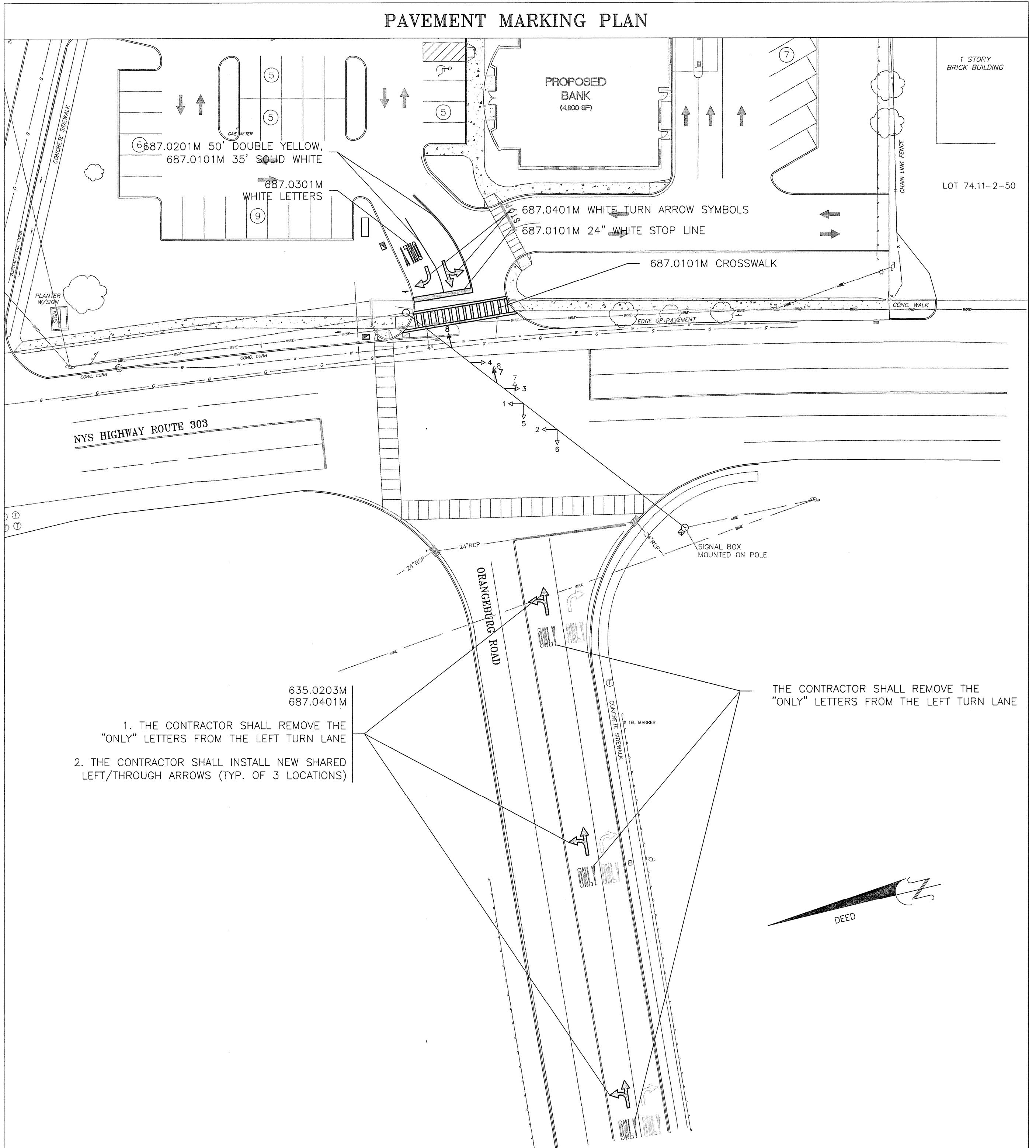
PROJECT No. MO8-056 JSE No. 00000

No.	REVISION	DATE
4	07-15-10 COMMENTS FROM NYSDOT	08-04-10
3	06-07-10 COMMENTS FROM NYSDOT	06-15-10
2	05-06-10 COMMENTS FROM NYSDOT	05-13-10
1	06-03-09 COMMENTS FROM NYSDOT	07-28-09



8 7 6 5 4 3 2 1

PAVEMENT MARKING PLAN

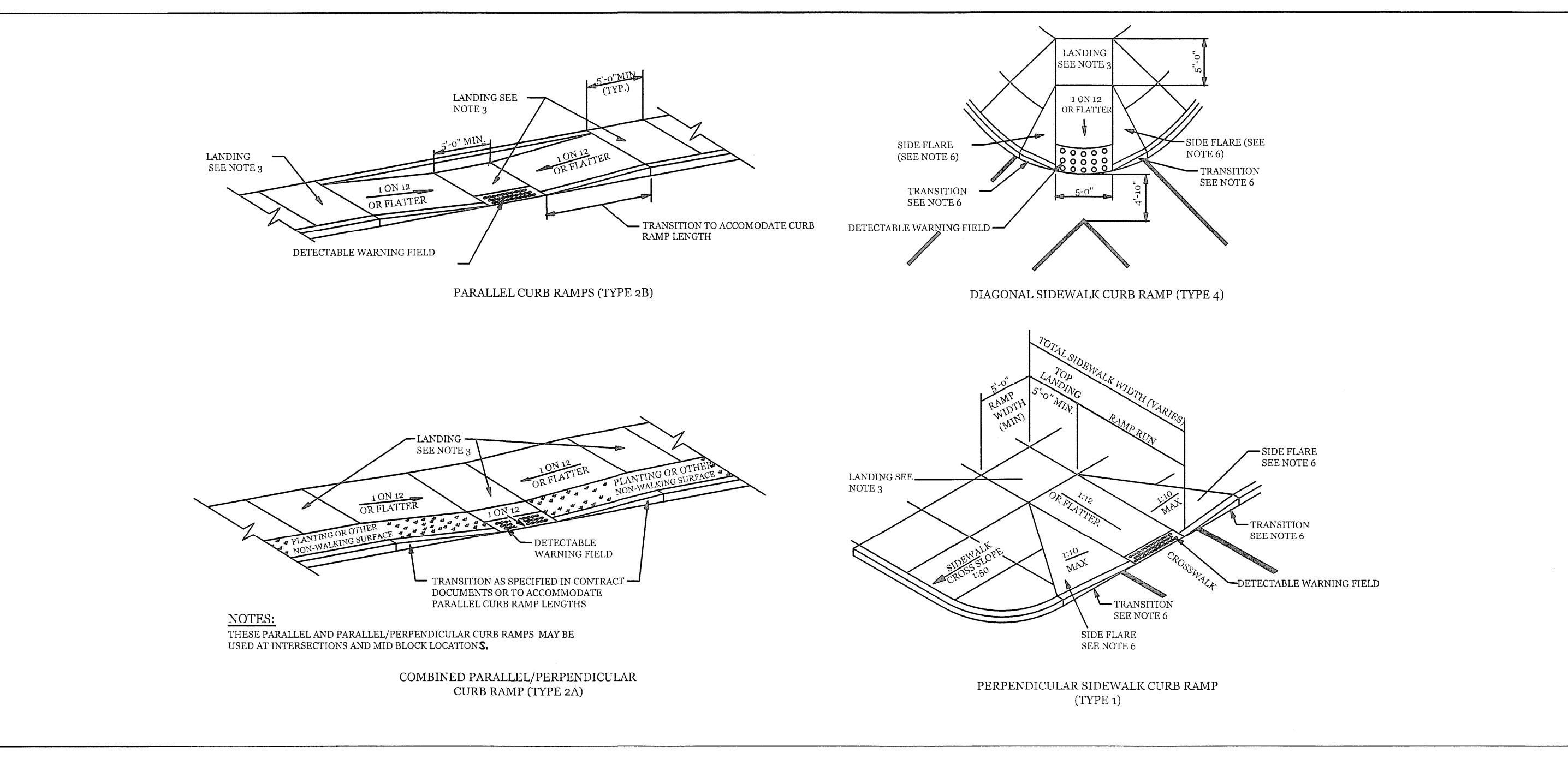
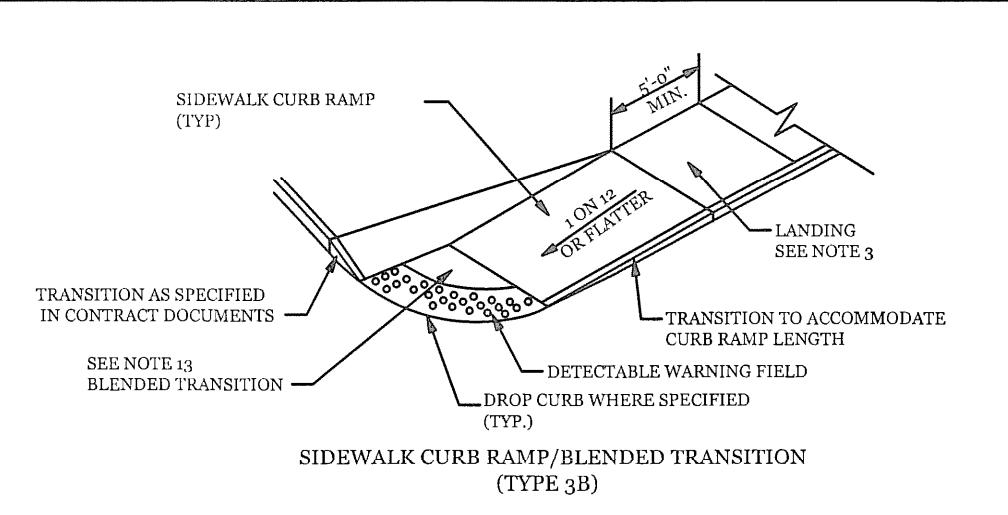
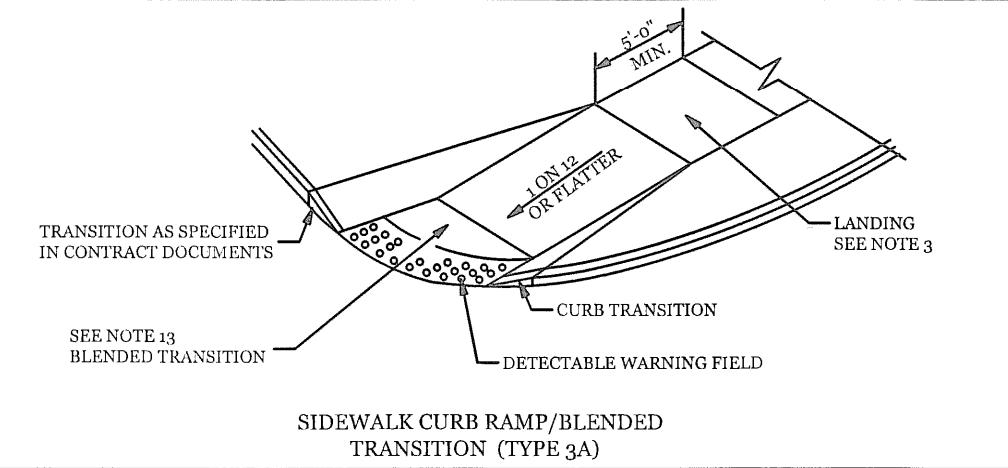
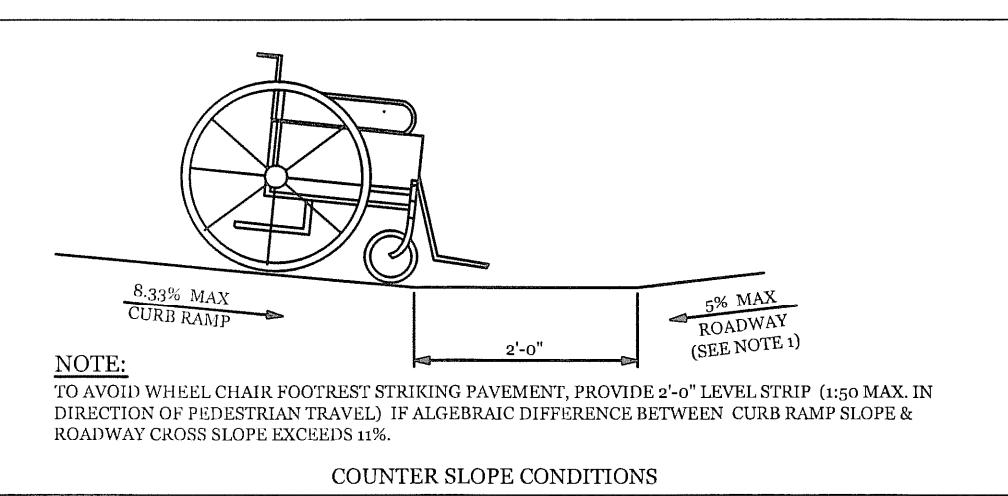


- D. PAVEMENT MARKING NOTES:
- ALL PAVEMENT MARKINGS SHALL BE PLACED IN ACCORDANCE WITH THE FEDERAL MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES.
 - THE CONTRACTOR SHALL REMOVE ANY AND ALL PAVEMENT MARKINGS THAT CONFLICT WITH THE PROPOSED PAVEMENT MARKINGS.
 - ALL DIMENSIONS AND PLACEMENT OF ARROWS, SYMBOLS, AND TEXT ARE SHOWN TYPICAL AND SHALL APPLY UNLESS OTHERWISE INDICATED IN THE CONTRACT DOCUMENTS.
 - THE REGIONAL TRAFFIC ENGINEER WILL REVIEW AND APPROVE ANY CHANGES TO THE PAVEMENT MARKING PLANS PRIOR TO FINAL INSTALLATION, CHANGES SHALL BE SUBMITTED TWO WEEKS PRIOR TO INSTALLATION.

HANDICAP RAMP DETAILS PER ADA REQUIREMENT

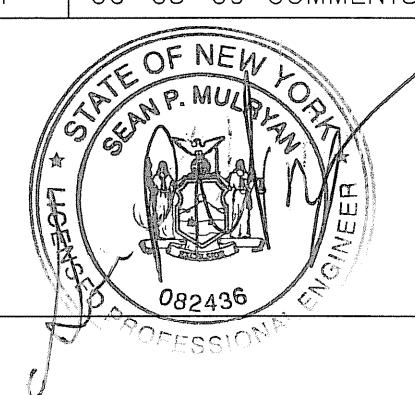
HANDICAP RAMP AND SIDEWALK NOTES:

- THE PUBLIC SIDEWALK CURB RAMP STANDARDS DEPICTED HERE MAY NOT BE APPROPRIATE FOR ALL LOCATIONS. FIELD CONDITIONS AT INDIVIDUAL LOCATIONS MAY REQUIRE SPECIFIC DESIGNS. DESIGNS MUST BE CONSISTENT WITH THE PROVISIONS OF THIS SHEET TO THE MAXIMUM EXTENT FEASIBLE ON ALTERATION PROJECTS AND WHEN STRUCTURALLY PRACTICABLE ON NEW CONSTRUCTION PROJECTS AS REQUIRED BY THE AMERICAN'S WITH DISABILITIES ACT ACCESSIBILITY GUIDELINES.
- THERE SHALL BE A LANDING AT THE TOP OF EACH CURB RAMP. THERE SHALL BE A LANDING AT THE TOP AND AT THE BOTTOM OF EACH PARALLEL AND PARALLEL/PERPENDICULAR RAMP.
- LANDINGS SHALL HAVE A MINIMUM CLEAR DIMENSION OF A 5'-0" X 5'-0" SQUARE. THE MAXIMUM CROSS SLOPE AT LANDINGS IS 2 PERCENT IN ANY DIRECTION. LANDINGS MAY OVERLAP WITH ADJACENT LANDINGS OR A SINGLE LANDING MAY SERVE MULTIPLE CURB RAMPS OR PARALLEL OR PARALLEL/PERPENDICULAR RAMPS. LANDINGS MAY OVERLAP WITH THE CLEAR GROUND SPACE REQUIRED AT PEDESTRIAN SIGNAL PUSH BUTTONS.
- CROSS SLOPES. THE MAXIMUM CROSS SLOPE OF CURB RAMPS SHALL BE 2 PERCENT. CURB RAMP SURFACES SHALL GENERALLY LIE IN CONTINUOUS PLANES WITH A MINIMUM OF SURFACE WARP.
- THE RUNNING GRADE OF CURB RAMPS SHOULD BE AS FLAT AS PRACTICABLE. THE MAXIMUM RUNNING GRADE OF ANY PORTION OF ANY CURB RAMP SHALL BE 1:12 (8.3%). CURB RAMPS ARE NOT REQUIRED TO BE LONGER THAN 15'-0"
- CURB RAMPS LOCATED WHERE PEDESTRIANS MAY WALK ACROSS THE CURB RAMP SHALL HAVE FLARED SIDES. THE LENGTH OF THE FLARES SHALL BE AT LEAST TEN (10) TIMES THE CURB HEIGHT, MEASURED ALONG THE CURB LINE. WHEN INFESTABLE OR IMPRACTICABLE TO PROVIDE A LANDING THAT IS AT LEAST 5'-0" WIDE (MEASURED FROM THE TOP OF THE RAMP TO THE BACK OF THE SIDEWALK), THE LENGTH OF THE FLARES SHALL BE TWELVE (12) TIMES THE CURB HEIGHT MEASURED ALONG THE CURB LINE.
- THE SURFACE OF ALL CURB RAMPS SHALL BE STABLE, FIRM AND SLIP RESISTANT. A COARSE BROOM FINISH RUNNING PERPENDICULAR TO THE SLOPE IS RECOMMENDED ON CONCRETE RAMP SURFACES, EXCLUSIVE OF THE DETECTABLE WARNING FIELDS.
- RAMP TRANSITIONS BETWEEN WALKS, GUTTERS, OR STREETS SHALL BE FLUSH AND FREE OF ABRUPT VERTICAL CHANGES 1/4" MAX.
- COORDINATE ALL TRAFFIC CONTROL DEVICES, UTILITY LOCATIONS, SIGNS, STREET FURNITURE AND DRAINAGE TO ENSURE A CONTINUOUS PEDESTRIAN ACCESS ROUTE AT ALL CURB RAMP LOCATIONS. GUIDANCE FOR CROSSWALK MARKINGS AND TRAFFIC CONTROL DEVICES IS PROVIDED IN THE MUTCD. DRAINAGE GRATES AND UTILITY ACCESS COVERS ARE NOT ALLOWED IN RAMP WALKING SURFACES OR LANDINGS.
- WHERE FEASIBLE, E.G. WHERE R.O.W. WIDTH PROVIDES SUFFICIENT SPACE TO INSTALL SIDEWALKS SET BACK FROM THE CURBS, RAMP TYPES 2A AND 3A SHOULD BE INSTALLED AS THE SEPARATION PROVIDED BETWEEN SIDEWALK AND CURB OR TRAVELWAY MAKE FOR GREATER PEDESTRIAN SAFETY AND COMFORT.
- AT MARKED CROSSINGS, THE FULL WIDTH OF THE RAMP SHALL BE WHOLLY CONTAINED WITHIN THE MARKINGS. THE SIDES OF THE RAMPS (THE FLARES) NEED NOT BE WITHIN THE WIDTH OF THE MARKINGS.
- DETAILS ILLUSTRATE THAT DETECTABLE WARNINGS ARE REQUIRED. SEE THE CURRENT DETECTABLE WARNING STANDARD SHEET FOR SPECIFIC DETECTABLE WARNING REQUIREMENTS. DETAILS DO NOT SHOW DROPPED CURBS AT BOTTOMS OF CURB RAMPS. DROPPED CURBS MAY BE SPECIFIED.
- SLOPES ON BLENDED TRANSITIONS SHALL NOT BE STEEPER THAN 2% (1 ON 50) IN ANY DIRECTION.



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PROJECT No. MO8-056 JSE No. 00000



TRAFFIC SIGNAL MODIFICATION PLAN
NYS Rt. 303 AT ORANGEBURG ROAD
ORANGEBURG, TOWN OF ORANGETOWN
ROCKLAND COUNTY, NEW YORK

SCALE: 1:30 DATE: 02-25-09 SHEET No. 2 OF 3

8 7 6 5 4 3 2 1

SIGNAL OPERATION SPECIFICATIONS TABLE OF SWITCH PACKS				
SIGNAL NO. NYS Rt. 303 & ORANGEBURG RD. COUNTY ROCKLAND				
SWITCH PACK	FUNCTION	FACE	INDICATIONS	TERMINAL WIRING BOARD
				TERMINAL WIRE COLOR CODE
SP 1	Ø1	1,2	R Y G GRND. WIRE	SP 1R SP 1Y SP 1G EXISTING WIRING TO REMAIN
SP 2	Ø2	3	R Y G GRND. WIRE	SP 2R SP 2Y SP 2G EXISTING WIRING TO REMAIN
SP 3	Ø3	5,6	R Y G GRND. WIRE	SP 3R SP 3Y SP 3G EXISTING WIRING TO REMAIN
SP 4	Ø4	7,8	R Y G GRND. WIRE	SP 4R 14/05C-1-R/B SP 4Y 14/05C-1-O/B SP 4G 14/05C-1-G/B GRND. BUS 14/05C-1-W/B
SP 5	Ø5	3,4	R Y G GRND. WIRE	SP 5R SP 5Y SP 5G EXISTING WIRING TO REMAIN
SP 6	OVL A Ø2+Ø5	6	R Y G GRND. WIRE	SP 6R SP 6Y SP 6G EXISTING WIRING TO REMAIN
SP 7	OVL B Ø3	2	R Y G GRND. WIRE	SP 7R SP 7Y SP 7G EXISTING WIRING TO REMAIN
SP 8			R Y G GRND. WIRE	SP 8R SP 8Y SP 8G
SP 9			R Y G GRND. WIRE	SP 9R SP 9Y SP 9G
SP 10			R Y G GRND. WIRE	SP 10R SP 10Y SP 10G
SP 11			R Y G GRND. WIRE	SP 11R SP 11Y SP 11G
SP 12			R Y G GRND. WIRE	SP 12R SP 12Y SP 12G
				GRND. BUS

KEY FOR WIRE COLOR CODE
14/ 19C - I - R/B
XX / X XC - X - X / X
AWG No. OF CABLE No. COLOR TRACER COLOR
COND- FOR THE COLOR OF COLOR
UCTORS GIVEN CONDUCTOR SIZE

COLOR: R-RED, O-ORANGE, G-GREEN, BL-BLUE, W-WHITE, B-BLACK

LEGEND

G = CIRCULAR GREEN	Y = CIRCULAR YELLOW	R = CIRCULAR RED
← = LEFT GREEN ARROW	→ = LEFT YELLOW ARROW	↔ = LEFT RED ARROW
→ = RIGHT GREEN ARROW	↔ = RIGHT YELLOW ARROW	→ = RIGHT RED ARROW
↑ = VERTICAL GREEN ARROW		

SIGNAL OPERATION SPECIFICATIONS TABLE OF INPUT WIRING			
SIGNAL NO. NYS Rt. 303 & ORANGEBURG RD. COUNTY ROCKLAND			
FUNCTION	DETECTOR NUMBER	TYPE	TERMINAL BOARD WIRING
Ø2	D2	NB LEFT TURN LOOP	1A , 1B
Ø3	D3	EB LOOP	2A , 2B
Ø4	D4	WB LOOP	3A , 3B
Ø4	D14	WB RIGHT TURN LOOP	4A , 4B
			5A , 5B
			6A , 6B
			7A , 7B
			8A , 8B
			9A , 9B
			10A , 10B
			11A , 11B
			12A , 12B
OVL A Ø2	D13	EB RIGHT TURN LOOP	13A , 13B
			14A , 14B
			15A , 15B
			16A , 16B

POLE DATA TABLE		
NYS Rt. 303 & ORANGEBURG ROAD		
POLE LOCATION	NW	SE
ITEM	EXISTING	EXISTING
LENGTH	30 FT	30 FT
DESIGN LOAD	7000 LBS	7000 LBS
MAX. FOOTING MOMENT	199.5 FT-K	199.5 FT-K

MICROCOMPUTER SIGNAL OPERATION SPECIFICATION

STATE OF NEW YORK
DEPARTMENT OF TRANSPORTATION

DRAWING No. M08-056	SCALE NO SCALE	DATE 07-28-09	REGION 08
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LIST OF ITEMS		
ITEM No.	DESCRIPTION	UNIT
203.03 M	Conduit Excavation and Backfill including Surface Restoration	LF
647.13 M	Ground Mounted Sign	EA
635.0203 M	Cleaning and Preparation of Pavement Surface Letters	EA
680.51 M	Reinforced Concrete Pullbox	EA
680.520106 M	Conduit - 2 NPS Steel	M
680.520108 M	Conduit - 3 NPS Steel	M
680.520203 M	Conduit - 1 NPS Flexible Liquid Tight Steel	M
680.54 M	Inductance Loop Installation	M
680.71 M	Shielded Lead-in Cable	M
680.72 M	Inductance Loop Wire	M
680.730514 M	5 Conductor Signal Cable 14 AWG	M
10680.77 M	Modify Traffic Signal Equipment	LS
08680.79 M	Remove Traffic Signal Equipment	EA
680.8103 M	Traffic Signal Section 200 mm	EA
680.810301 M	Traffic Signal Module 200 mm LED Red Ball	EA
680.810303 M	Traffic Signal Module 200 mm LED Yellow Ball	EA
680.810305 M	Traffic Signal Module 200 mm LED Green Ball	EA
680.8111 M	Traffic Signal Bracket Assembly 1 Way	EA
687.0101 M	White Thermoplastic Reflectorized Pavement Stripes	M
687.0201 M	Yellow Thermoplastic Reflectorized Pavement Stripes	M
687.0301 M	White Thermoplastic Reflectorized Pavement Letters	EA
687.0401 M	White Thermoplastic Reflectorized Pavement Symbols	EA
---	ENCOM 5270 SPREAD SPECTRUM RADIOS	(2) EA
---	SEE NOTE B11	

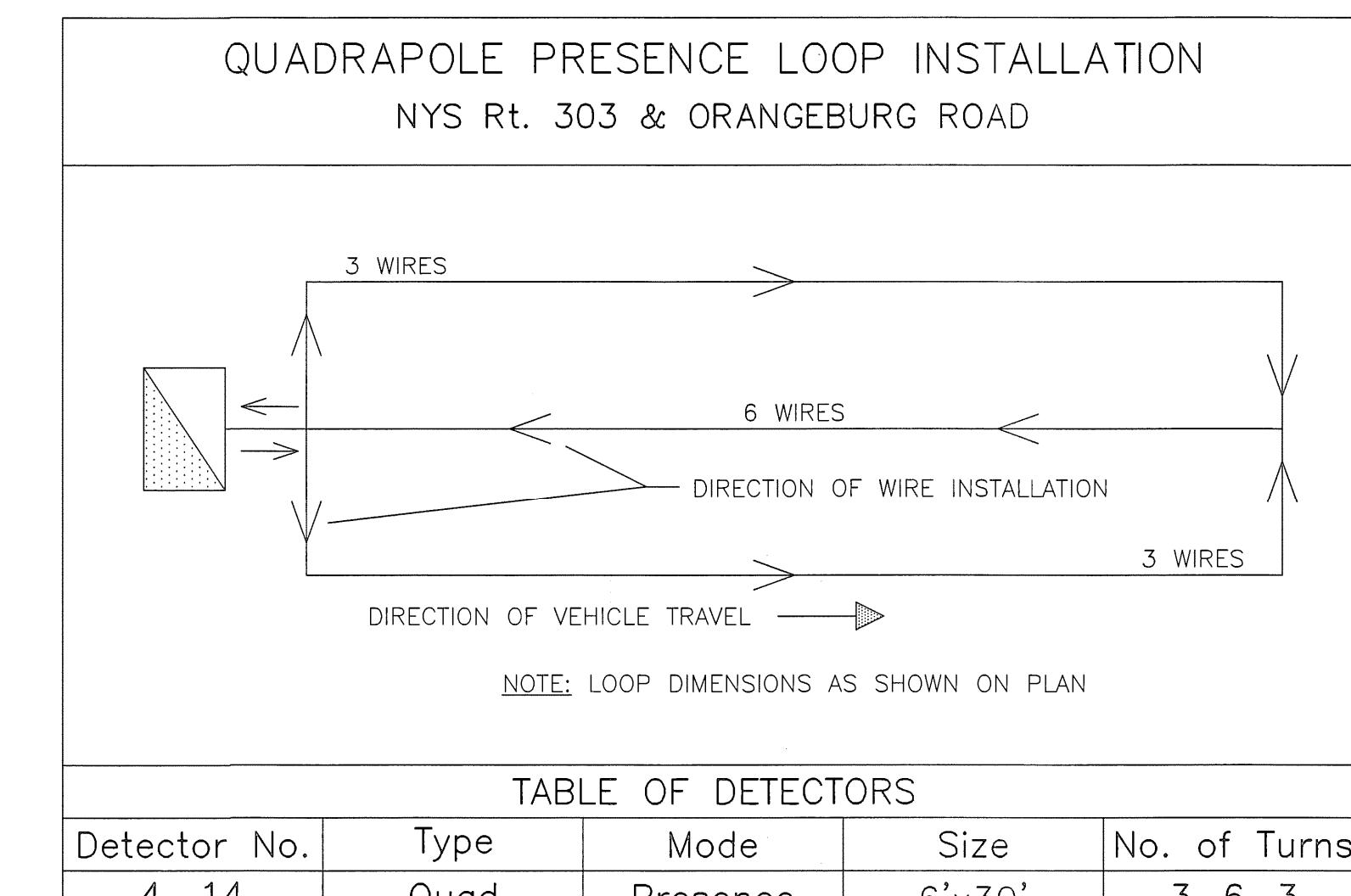
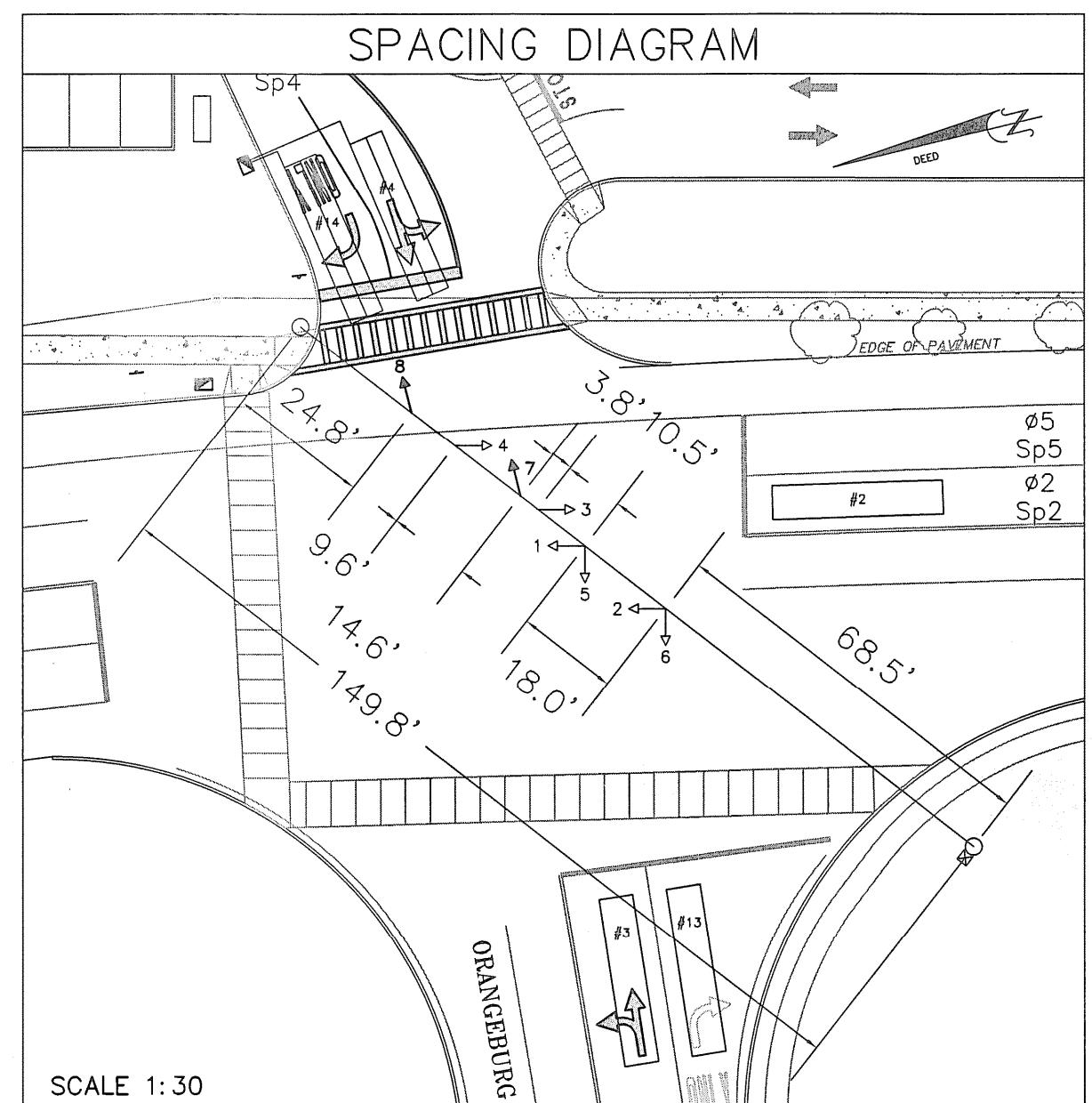


TABLE OF DETECTORS				
Detector No.	Type	Mode	Size	No. of Turns
4, 14	Quad	Presence	6'x30'	3-6-3

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No. REVISION DATE
4 07-15-10 COMMENTS FROM NYSDOT 08-04-10
3 06-07-10 COMMENTS FROM NYSDOT 06-15-10
2 05-06-10 COMMENTS FROM NYSDOT 05-13-10
1 06-03-09 COMMENTS FROM NYSDOT 07-28-09

TRAFFIC SIGNAL MODIFICATION PLAN
NYS Rt. 303 AT ORANGEBURG ROAD
ORANGEBURG, TOWN OF ORANGETOWN
ROCKLAND COUNTY, NEW YORK

SCALE: NTS DATE: 02-25-09 SHEET No. 3 OF 3

Phase Times [1.1.1]								Coordination Patterns [2.4] and Coordination Split Tables [2.7.1]																STD8							
Times [1.1.1]	1	2	3	4	5	6	7	8	Pat#	Cyc	Off	Split	Seq	Pat#	Cyc	Off	Split	Seq	Pat#	Cyc	Off	Split	Seq	Pat#	Cyc	Off	Split	Seq			
Min Green	10	5	5	5	10				1	110	0	1	1	13	0	0	13	1	25	0	0		1	37	0	0		1			
Gap, Ext		2	2	2					2	0	0	2	1	14	0	0	14	1	26	0	0		1	38	0	0		1			
Max 1	30	15	35	20	30				3	0	0	3	1	15	0	0	15	1	27	0	0		1	39	0	0		1			
Max 2									4	0	0	4	1	16	0	0	16	1	28	0	0		1	40	0	0		1			
Yel Clearance	4	4	4	4	4				5	0	0	5	1	17	0	0	17	1	29	0	0		1	41	0	0		1			
Red Clearance	1	1	1	1	1				6	0	0	6	1	18	0	0	18	1	30	0	0		1	42	0	0		1			
Walk	7		7						7	0	0	7	1	19	0	0	19	1	31	0	0		1	43	0	0		1	1	1	GREEN
Ped Clearance	33		23						8	0	0	8	1	20	0	0	20	1	32	0	0		1	44	0	0		1	3	1	RED
Red Revert									9	0	0	9	1	21	0	0	21	1	33	0	0		1	45	0	0		1	4	1	RED
Add Initial									10	0	0	10	1	22	0	0	22	1	34	0	0		1	46	0	0		1	5	2	GREEN
Max Initial									11	0	0	11	1	23	0	0	23	1	35	0	0		1	47	0	0		1	6	2	RED
Time B4 Reduct									12	0	0	12	1	24	0	0	24	1	36	0	0		1	48	0	0		1	7	2	RED
Cars B4 Reduct																															
Time To Reduce																															
Reduce By																															
Min Gap																															
DyMaxLim																															
Max Step																															
Options [1.1.2]	1	2	3	4	5	6	7	8																							
Enable	1	1	1	1	1																										
Min Recall																															
Max Recall	1			1																											
Ped Recall																															
Soft Recall																															
Lock Calls																															
Auto Flash Entry																															
Auto Flash Exit																															
Dual Entry																															
Enable Simul Gap																															
Guarantee Passage																															
Rest In Walk																															
Conditon Service																															
Non-Actuated 1																															
Non-Actuated 2																															
Add Init Calc																															
Options+ [1.1.3]	1	2	3	4	5	6	7	8																							
Reserve																															
PedCler Thru Yel																															
Skip Red No Call																															
Red Rest																															
Max II																															
Conflicting Phase																															
Red Rest On Gap																															
Omit Yellow																															
Ped Delay																															
Grn/Ped Delay																															
RTE 303 @ ORANGEBURGH RD (ID 5102) (Standard File)																															

STD8

Ring/Startup [1.1.4]

Phs Ring Start Enable

Coord Modes [2.1]

Test OpMode 0

Correction SHRT/LNG

Maximum MAX 1

Force-Off FLOAT

Closed Loop ON

Stop-in-Walk ON

Auto Reset ON

Expand Splt OFF

Ped Recycle NO_RECYLE

Before TIMED

After TIMED

Auto Flash [1.4.1]

Auto Flash PH OVER

Flash Yel 4.5

Flash Red 2

Unit Params [1.2.1]

Phase Mode STD8

IO Mode USER

Loc Flsh Start ON

Start Flsh(s) 0

Start AllRed(s) 0

Yellow < 3° OFF

Display Time 20

Red Revert 0

MCE Timeout 0

Feature Profile 0

Free Ring Seq 1

Auxswitch STOPTM

SDLC Retry 0

TS2 Det Faults ON

Auto Ped Clear OFF

SDLC Retry 0

08/01/13 Page 1A

Concurrency [1.1.4]

Phs	Concurrent Phases								
1	5	6	0	0	0	0	0	0	0
2	5	6	0	0	0	0	0	0	0
3	7	8	0	0	0	0	0	0	0
4	7	8	0	0	0	0	0	0	0
5	1	2	0	0	0	0	0	0	0
6	1	2	0	0	0	0	0	0	0
7	3	4	0	0	0	0	0	0	0
8	3	4	0	0	0	0	0	0	0
9	0	0	0	0	0	0	0	0	0
10	0	0	0	0	0	0	0	0	0
11	0	0	0	0	0	0	0	0	0
12	0	0	0	0	0	0	0	0	0
13	0	0	0	0	0	0	0	0	0
14	0	0	0	0	0	0	0	0	0
15	0	0	0	0	0	0	0	0	0
16	0	0	0	0	0	0	0	0	0

Sequence [1.2.4]

Seq	Rng	Concurrent Phases									Seq	Rng	Concurrent Phases				
1	1	1	2	3	4	0	0	0	0	0	9	1	2	4	3	0	0
1	2	5	6	7	8	0	0	0	0	0	9	2	5	6	7	8	0
1	3	0	0	0	0	0	0	0	0	0	9	3	0	0	0	0	0
1	4	0	0	0	0	0	0	0	0	0	9	4	0	0	0	0	0
2	1	1	2	3	4	0	0	0	0	0	10	1	1	2	4	3	0
2	2	6	5	7	8	0	0	0	0	0	10	2	6	5	7	8	0
2	3	0	0	0	0	0	0	0	0	0	10	3	0	0	0	0	0
2	4	0	0	0	0	0	0	0	0	0	10	4	0	0	0	0	0
3	1	2	1	3	4	0	0	0	0	0	11	1	2	1	4	3	0
3	2	5	6	7	8	0	0	0	0	0	11	2	5	6	7	8	0
3	3	0	0	0	0	0	0	0	0	0	11	3	0	0	0	0	0
3	4	0	0	0	0	0	0	0	0	0	11	4	0	0	0	0	0
4	1	2	1	3	4	0	0	0	0	0	12	1	2	1	4	3	0
4	2	6	5	7	8	0	0	0	0	0	12	2	6	5	7	8	0
4	3	0	0	0	0	0	0	0	0	0	12	3	0	0	0	0	0
4	4	0	0	0	0	0	0	0	0	0	12	4	0	0	0	0	0
5	1	1	2	3	4	0	0	0	0	0	13	1	1	2	4	3	0
5	2	5	6	8	7	0	0	0	0	0	13	2	5	6	8	7	0
5	3	0	0	0	0	0	0	0	0	0	13	3	0	0	0	0	0
5	4	0	0	0	0	0	0	0	0	0	13	4	0	0	0	0	0
6	1	1	2	3	4	0	0	0	0	0	14	1	1	2	4	3	0
6	2	6	5	8	7	0	0	0	0	0	14	2	6	5	8	7	0
6	3	0	0	0	0	0	0	0	0	0	14	3	0	0	0	0	0
6	4	0	0	0	0	0	0	0	0	0	14	4	0	0	0	0	0
7	1	2	1	3	4	0	0	0	0	0	15	1	2	1	4	3	0
7	2	5	6	8	7	0	0	0	0	0	15	2	5	6	8	7	0
7	3	0	0	0	0	0	0	0	0	0	15	3	0	0	0	0	0
7	4	0	0	0	0	0	0	0	0	0	15	4	0	0	0	0	0
8	1	2	1	3	4	0	0	0	0	0	16	1	2	1	4	3	0
8	2	6	5	8	7	0	0	0	0	0	16	2	6	5	8	7	0
8	3	0	0	0	0	0	0	0	0	0	16	3	0	0	0	0	0
8	4	0	0	0	0	0	0	0	0	0	16	4	0	0	0	0	0

Overlap 1-16 Program Params & Parm+ [1.5.2,1] [1.5.2,2]															
Overlap Conflict Lock		OFF		Overlap Lock Inhibit		OFF		Parent Ph Clearance		ON		Extra Included Ph		ON	
A	Included Ø	2	3					NORMAL		Included Ø					NORMAL
	Modifier Ø							Grn		Modifier Ø					Grn
	Conflict Ø							Yel	4	Conflict Ø					Yel 3.5
	Conflict Olap							Red	1	Conflict Olap					Red 1.5
	Conflict Ped							LG		Conflict Ped					LG
B	Included Ø							NORMAL		Included Ø					NORMAL
	Modifier Ø							Grn		Modifier Ø					Grn
	Conflict Ø							Yel		Conflict Ø					Yel 3.5
	Conflict Olap							Red		Conflict Olap					Red 1.5
	Conflict Ped							LG		Conflict Ped					LG
C	Included Ø							NORMAL		Included Ø					NORMAL
	Modifier Ø							Grn		Modifier Ø					Grn
	Conflict Ø							Yel	3.5	Conflict Ø					Yel 3.5
	Conflict Olap							Red	1.5	Conflict Olap					Red 1.5
	Conflict Ped							LG		Conflict Ped					LG
D	Included Ø							NORMAL		Included Ø					NORMAL
	Modifier Ø							Grn		Modifier Ø					Grn
	Conflict Ø							Yel	3.5	Conflict Ø					Yel 3.5
	Conflict Olap							Red	1.5	Conflict Olap					Red 1.5
	Conflict Ped							LG		Conflict Ped					LG
E	Included Ø							NORMAL		Included Ø					NORMAL
	Modifier Ø							Grn		Modifier Ø					Grn
	Conflict Ø							Yel	3.5	Conflict Ø					Yel 3.5
	Conflict Olap							Red	1.5	Conflict Olap					Red 1.5
	Conflict Ped							LG		Conflict Ped					LG
F	Included Ø							NORMAL		Included Ø					NORMAL
	Modifier Ø							Grn		Modifier Ø					Grn
	Conflict Ø							Yel	3.5	Conflict Ø					Yel 3.5
	Conflict Olap							Red	1.5	Conflict Olap					Red 1.5
	Conflict Ped							LG		Conflict Ped					LG
G	Included Ø							NORMAL		Included Ø					NORMAL
	Modifier Ø							Grn		Modifier Ø					Grn
	Conflict Ø							Yel	3.5	Conflict Ø					Yel 3.5
	Conflict Olap							Red	1.5	Conflict Olap					Red 1.5
	Conflict Ped							LG		Conflict Ped					LG
H	Included Ø							NORMAL		Included Ø					NORMAL
	Modifier Ø							Grn		Modifier Ø					Grn
	Conflict Ø							Yel	3.5	Conflict Ø					Yel 3.5
	Conflict Olap							Red	1.5	Conflict Olap					Red 1.5
	Conflict Ped							LG		Conflict Ped					LG
Channel Settings [1.8.11]															
... Channel -->		1	2	3	4	5	6	7	8	9	10	11	12	13	14
Phase / Olap #		1	2	3	4	5	1	3				1	4		
Channel Type		VEH	VEH	VEH	VEH	OLP	VEH	VEH	VEH	VEH	VEH	PED	PED	VEH	VEH
Channel Flash		YEL	DRK	RED	RED	YEL	DRK	DRK	RED	RED	RED	DRK	DRK	DRK	DRK

Coord Transition, CoorPhs [2.5]											
Pat#	Short	Long	Dwell	No Shortway	Ø	E-Yld	Offset	RetHld	Float	Min Veh Perm	Min Ped Perm
1	12	22					EndGRN				
2	12	22					EndGRN				
3	12	22					EndGRN				
4	12	22					EndGRN				
5	12	22					EndGRN				
6	12	22					EndGRN				
7	12	22					EndGRN				
8	12	22					EndGRN				
9	12	22					EndGRN				
10	12	22					EndGRN				
11	12	22					EndGRN				
12	12	22					EndGRN				
13	12	22					EndGRN				
14	12	22					EndGRN				
15	12	22					EndGRN				
16	12	22					EndGRN				
17	12	22					EndGRN				
18	12	22					EndGRN				
19	12	22					EndGRN				
20	12	22					EndGRN				
21	12	22					EndGRN				
22	12	22					EndGRN				
23	12	22					EndGRN				
24	12	22					EndGRN				
25	0	0					BegGRN				
26	0	0					BegGRN				
27	0	0					BegGRN				
28	0	0					BegGRN				
29	0	0					BegGRN				
30	0	0					BegGRN				
31	0	0					BegGRN				
32	0	0					BegGRN				
33	0	0					BegGRN				
34	0	0					BegGRN				
35	0	0					BegGRN				
36	0	0					BegGRN				
37	0	0					BegGRN				
38	0	0					BegGRN				
39	0	0					BegGRN				
40	0	0					BegGRN				
41	0	0					BegGRN				
42	0	0					BegGRN				
43	0	0					BegGRN				
44	0	0					BegGRN				
45	0	0					BegGRN				
46	0	0					BegGRN				
47	0	0					BegGRN				
48	0	0					BegGRN				

Channel Settings [1.8.1]

Channel >> 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 | 13 | 14 | 15 | 16 | 17 | 18 | 19 | 20 | 21 | 22 | 23 | 24

Phase / Olap # 1 2 3 4 5 1 3 1 4

Channel Type VEH VEH VEH VEH VEH OLP VEH VEH VEH VEH PED PED VEH VEH

Alt Hz

Channel+ Settings [1.8.4]

... Channel ->| 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 | 13 | 14 | 15 | 16 | 17 | 18 | 19 | 20 | 21 | 22 | 23 | 24 |

Flash Green+ [View Details](#)

Veh Par 1-64 [5.1]										Veh Par 1-64 [5.1]										Vehicle Options 1-64 [5.2]										Vehicle Options 1-64 [5.2]										Parameters+ 1-64 [5.3]									
Det #	Call Ø	Šwi Ø	Dlay Ext	Que	No Act	Max Pres	Err Cnt	Fail Time	Det #	Call Ø	Šwi Ø	Dlay Ext	Que	No Act	Max Pres	Err Cnt	Fail Time	Det #	Call	Ext	Que	Add Init	Red Lock	Yell Lock	occ	vol	Det #	Call	Ext	Que	Add Red	Red Lock	Yell Lock	occ	vol	Det #	oc G	oc Y	oc R	Dlay 1	Dlay 2	Type	Src						
1					45	50		33						45	50		31	1	1		1						33	1	1		1						1									NORM			
2	2	8			45	50	10	34						45	50		32	1	1		1						34	1	1		1					2									NORM				
3	3				45	50	20	35						45	50		33	1	1		1						35	1	1		1					3									NORM				
4	4				45	50	10	36						45	50		34	1	1		1						36	1	1		1					4									NORM				
5					45	50		37						45	50		35	1	1		1						37	1	1		1					5									NORM				
6					45	50		38						45	50		36	1	1		1						38	1	1		1					6									NORM				
7					45	50		39						45	50		37	1	1		1						39	1	1		1					7									NORM				
8					45	50		40						45	50		38	1	1		1						40	1	1		1					8									NORM				
9					45	50		41						45	50		39	1	1		1						41	1	1		1					9									NORM				
10					45	50		42						45	50		40	1	1		1						42	1	1		1					10									NORM				
11					45	50		43						45	50		41	1	1		1						43	1	1		1					11									NORM				
12					45	50		44						45	50		42	1	1		1						44	1	1		1					12									NORM				
13	3	5			45	50	20	45						45	50		43	1	1		1						45	1	1		1					13									NORM				
14	4	5			45	50	10	46						45	50		44	1	1		1						46	1	1		1					14									NORM				
15					45	50		47						45	50		45	1	1		1						47	1	1		1					15									NORM				
16					45	50		48						45	50		46	1	1		1						48	1	1		1					16									NORM				
17					45	50		49						45	50		47	1	1		1						49	1	1		1					17									NORM				
18					45	50		50						45	50		48	1	1		1						50	1	1		1					18									NORM				
19					45	50		51						45	50		49	1	1		1						51	1	1		1					19									NORM				
20					45	50		52						45	50		50	20	1	1		1					52	1	1		1					20									NORM				
21					45	50		53						45	50		51	21	1	1		1					53	1	1		1					21									NORM				
22					45	50		54						45	50		52	22	1	1		1					54	1	1		1					22									NORM				
23					45	50		55						45	50		53	23	1	1		1					55	1	1		1					23									NORM				
24					45	50		56						45	50		54	24	1	1		1					56	1	1		1					24									NORM				
25					45	50		57						45	50		55	25	1	1		1					57	1	1		1					25									NORM				
26					45	50		58						45	50		56	26	1	1		1					58	1	1		1					26									NORM				
27					45	50		59						45	50		57	27	1	1		1					59	1	1		1					27									NORM				
28					45	50		60						45	50		58	28	1	1		1					60	1	1		1					28									NORM				
29					45	50		61						45	50		59	29	1	1		1					61	1	1		1					29									NORM				
30					45	50		62						45	50		60	30	1	1		1					62	1	1		1					30									NORM				
31					45	50		63						45	50		61	31	1	1		1					63	1	1		1					31									NORM				
32					45	50		64						45	50		62	32	1	1		1					64	1	1		1					32									NORM				

Ped Det Parms [5.4]

Det #	Call Ø	No Act	Max Pres	Err Cnt
1	1	0	15	0
2	4	0	15	0
3	0	15	0	
4	0	15	0	
5	0	15	0	
6	0	15	0	
7	0	15	0	
8	0	15	0	

Unit Paramters [1.2.1]

TS2 Det Faults	ON
Vol/Occ Report Parm [1.5.8]	
Vol/Occ Period Minutes	0
Vol/Occ Period Minutes	15

Preemption Times [3.1], Options+ [3.6]					
Pre #	Enable	Type	Output	Delay	MinDura
1	ON	RAIL	DWELL		
2	ON	RAIL	DWELL		
3	ON	EMERG	DWELL		
4	ON	EMERG	DWELL		
5	ON	EMERG	DWELL		
6	ON	EMERG	DWELL		
Pre #	MaxPres	MinGmn	MinWlk	PedClr	Co+Pre
1					ON
2					ON
3					ON
4					ON
5					ON
6					ON
Pre #	Track Grp	Min Dwell	Ext Dwell	PedClr	Yel
1		2			
2		2			
3		2			
4		2			
5		2			
6		2			
Pre #	Red	Pattern	Skip		
1				OFF	
2				OFF	
3				OFF	
4				OFF	
5				OFF	
6				OFF	
Low Priority Preempts					
Pre #	Type	Min	Max		
7	OFF	0	0		
8	OFF	0	0		
9	OFF	0	0		
10	OFF	0	0		
Unit Parameters [1.2.1]					
Stop Timer Over Preempt		OFF			
Preempt or Ext Output		PRE			
Max Seek Track Time		0			
Max Seek Dwell Time		0			
Channel Parameters [1.8.3]					
D Conn Mappings		NONE			
Pre Invert Rail Input					

Track Clear Phases [3.2], Track Clear Overlaps+ [3.5]							
Pre #	Track Phases		Track Overlaps				
1							
2							
3							
4							
5							
6							
Dwell Phases [3.2] and Overlaps+ [3.5]							
Pre #							
1	Phases						
	Overlaps						
	Peds						
2	Phases						
	Overlaps						
	Peds						
3	Phases						
	Overlaps						
	Peds						
4	Phases						
	Overlaps						
	Peds						
5	Phases						
	Overlaps						
	Peds						
6	Phases						
	Overlaps						
	Peds						
Preemption 1, Options+ [3.6]							
Exit Phases [3.2]			Pre #	Lock	Override	Override	
Pre #	Exit Phase			Auto Flsh	Higher	Flsh Dwel Link	
1				1 ON	ON	ON OFF	
2				2 ON	ON	ON ON	
3				3 ON	ON	ON OFF	
4				4 ON	ON	ON OFF	
5				5 ON	ON	ON OFF	
6				6 ON	ON	ON OFF	

C1-USER IO Map [1.8.9.1 In]		
11-1	189	Unused
11-2	2	Veh Call 2
11-3	3	Veh Call 3
11-4	4	Veh Call 4
11-5	189	Unused
11-6	189	Unused
11-7	189	Unused
11-8	189	Unused
12-1	189	Unused
12-2	189	Unused
12-3	189	Unused
12-4	189	Unused
12-5	13	Veh Call 13
12-6	14	Veh Call 14
12-7	189	Unused
12-8	189	Unused
13-1	189	Unused
13-2	189	Unused
13-3	189	Unused
13-4	189	Unused
13-5	129	Ped Call 1
13-6	130	Ped Call 2
13-7	189	Unused
13-8	189	Unused
14-1	C11S Connector	
14-2		
14-3		
14-4		
14-5	189	Unused
14-6	189	Unused
14-7	229	33xCMUStop
14-8	228	33xFlashSns
15-1	189	Unused
15-2	189	Unused
15-3	189	Unused
15-4	189	Unused
15-5	189	Unused
15-6	189	Unused
15-7	189	Unused
15-8	189	Unused
16-1	189	Unused
16-2	189	Unused
16-3	189	Unused
16-4	189	Unused
16-5	189	Unused
16-6	189	Unused
16-7	189	Unused
16-8	189	Unused

C1-USER IO Map [1.8.9.2 Out]		
O1-1	1	Ch1 Red
O1-2	49	Ch1 Green
O1-3	2	Ch2 Red
O1-4	26	Ch2 Yellow
O1-5	50	Ch2 Green
O1-6	3	Ch3 Red
O1-7	27	Ch3 Yellow
O1-8	51	Ch3 Green
O2-1	4	Ch4 Red
O2-2	52	Ch4 Green
O2-3	5	Ch5 Red
O2-4	29	Ch5 Yellow
O2-5	53	Ch5 Green
O2-6	6	Ch6 Red
O2-7	30	Ch6 Yellow
O2-8	54	Ch6 Green
O3-1	7	Ch7 Red
O3-2	55	Ch7 Green
O3-3	8	Ch8 Red
O3-4	32	Ch8 Yellow
O3-5	56	Ch8 Green
O3-6	9	Ch9 Red
O3-7	33	Ch9 Yellow
O3-8	57	Ch9 Green
O4-1	10	Ch10 Red
O4-2	58	Ch10 Green
O4-3	11	Ch11 Red
O4-4	35	Ch11 Yellow
O4-5	59	Ch11 Green
O4-6	12	Ch12 Red
O4-7	36	Ch12 Yellow
O4-8	60	Ch12 Green
O5-1	28	Ch13 Yellow
O5-2	34	Ch10 Yellow
O5-3	25	Ch1 Yellow
O5-4	31	Ch7 Yellow
O5-5	115	Not Used
O5-6	115	Not Used
O5-7	115	Not Used
O5-8	114	Watchdog
O6-1	115	Not Used
O6-2	115	Not Used
O6-3	13	Ch13 Red
O6-4	37	Ch13 Yellow
O6-5	61	Ch13 Green
O6-6	14	Ch14 Red
O6-7	38	Ch14 Yellow
O6-8	62	Ch14 Green

C1-USER IO Map [1.8.9.2 Out]		
O7-1	115	Not Used
O7-2	115	Not Used
O7-3	115	Not Used
O7-4	115	Not Used
O7-5	115	Not Used
O7-6	115	Not Used
O7-7	115	Not Used
O7-8	115	Not Used

IO Logic [1.8.7]		
I 0 =	Fcn	Oper
I 0 =	Fcn	Oper
I 0 =	Fcn	Oper
I 0 =	Fcn	Oper
I 0 =	Fcn	Oper
I 0 =	Fcn	Oper
I 0 =	Fcn	Oper
I 0 =	Fcn	Oper
I 0 =	Fcn	Oper
I 0 =	Fcn	Oper
I 0 =	Fcn	Oper
I 0 =	Fcn	Oper
I 0 =	Fcn	Oper
I 0 =	Fcn	Oper
Security Access Levels [8.2]		
1 SWLOAD	22	NONE
2 SECURE	23	NONE
3 NONE	24	NONE
4 NONE	25	NONE
5 NONE	26	NONE
6 NONE	27	NONE
7 NONE	28	NONE
8 NONE	29	NONE
9 NONE	30	NONE
10 NONE	31	NONE
11 NONE	32	NONE
12 NONE	33	NONE
13 NONE	34	NONE
14 NONE	35	NONE
15 NONE	36	NONE
16 NONE	37	NONE
17 NONE	38	NONE
18 NONE	39	NONE
19 NONE	40	NONE
20 NONE	41	NONE
21 NONE	42	NONE

43	NONE	Com Parameters [6.1]
44	NONE	Station ID 5102
45	NONE	Group ID
46	NONE	Master ID 0
47	NONE	Backup Time 900
48	NONE	SysUp Modem [6.1]
49	NONE	Enable Modem OFF
50	NONE	Idle Time 15
51	NONE	Dial Time 5
52	NONE	Tel: 0,0-000-000-0000
53	NONE	All: 0,0-000-000-0000
54	NONE	2070 Port parms [6.2]
55	NONE	Port Baud Rate FCM
56	NONE	SP1 9600 6
57	NONE	SP2 9600 6
58	NONE	SP3 19200 6
59	NONE	SP4 38400 6
60	NONE	SP5 1200
61	NONE	SP6 1200
62	NONE	SP7 1200
63	NONE	SP8 1200

2070 IP 1 Addressing [6.5]		
Addr	Addressing	
Mask		
Brdct		
GtWay		
Port		

2070 IP 2 Addressing [6.5]		
Addr	Addressing	
Mask		
Brdct		
GtWay		
Port		

2070 Port Binding Ports [6.6]			
	Port	Echo	Mode
ASYNC1	SP1	NONE	
ASYNC2	SP2	NONE	
ASYNC3	SP3	NONE	
ASYNC4	SP4	NONE	
SYNC1	SP5		
SYNC2	OFF		

2070 Port Binding Functions [6.6]			
Function	Channel	Function	Channel
TS2/CVM	NONE	SYSUp	ASYNC2
CMU/MMU	NONE	SYSDown	ASYNC1
Opticom	NONE	Shell	NONE
Loop Det.	NONE		
GPS	NONE		

#	Event / Alarm	Ev	Alr	Call Phases[1.1.5]		Redirect Phases[1.1.5]								Inhibit Phases[1.1.5]																
				Ø	Phases Called By Ø	From	To	From	To	From	To	From	To	From	To	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15
1	Power Up Alarm.	1	1			1																								
2	Stop Timing	1	1			2																								
3	TS1 Cabinet Door					3																								
4	Coordination Failure	1	1			4																								
5	External Alarm # 1	1	1			5																								
6	External Alarm # 2	1	1			6																								
7	External Alarm # 3					7																								
8	External Alarm # 4					8																								
9	Closed Loop Disabled	1				9																								
10	External Alarm # 5					10																								
11	External Alarm # 6					11																								
12	Manual Control Enable	1	1			12																								
13	Coord Free Input					13																								
14	Local Flash Input	1	1			14																								
15	MMU Flash					15																								
16	CMU Flash					16																								
17	Cycle Fault	1																												
18	Cycle Failure	1																												
19	Coordination Fault	1																												
20	Controller Fault	1	1																											
21	Detector SDLC Failure																													
22	MMU SDLC Failure																													
23	Critical SDLC Failure																													
24	Reserved																													
25	EEPROM CRC Fault	1	1																											
26	Detector Diagnostic Failure																													
27	BIU Detector Failure	1	1																											
28	Queue detector alarm	1																												
29	Ped Detector Fault	1																												
30	Coord Diagnostic Fault																													
41	TempAlert Probe Ch. A																													
42	TempAlert Probe Ch. B																													
47	Coord Active																													
48	Preempt Active	1																												
49	Preempt 1 Input	1																												
50	Preempt 2 Input	1																												
51	Preempt 3 Input	1																												
52	Preempt 4 Input	1																												
53	Preempt 5 Input	1																												
54	Preempt 6 Input	1																												
55	Preempt 7 Input	1																												
56	Preempt 8 Input	1																												
57	Preempt 9 Input	1																												
58	Preempt 10 Input	1																												
61	In Transition	1																												
81	FIO Status Alarm																													

Yel Ø
Yel (olaps)

Auto Flash Phase/Olap Settings [1.4.2]

Allow Skip Yellow	OFF	TOD Dim Enable	OFF	Max Cycle Time	0
2	OFF	Tone Disable	OFF	Cycle Fault Action	ALARM
3	OFF	Diamond Mode	4Ph		
4	OFF	Backup Time (s)	900		
		Disable Init Ped	OFF		
		Cycle Fault Action	ALARM		
		Enable Run Timer	ON	RTE 303 @ ORANGEBURGH RD (ID 5102) (Standard Fil	

RTE 303 @ ORANGEBURGH RD (ID 5102) (Standard Fil

Page _____

8/1/2013

MODEL 2070 SIGNAL OPERATION
PROGRAMMABLE FEATURES
SIGNAL OPERATION SPECIFICATION

TAPS _____
STUDY # _____
FILE # _____
PAGE 1 OF 3

SIGNAL #

R - 102

COUNTY

ROCKLAND

DATE 10/03/12

TABLE OF SWITCH PACKS

SWITCH PACK	FUNCTION	INDICATIONS	FACE	TERMINAL WIRING BOARD		FACE	TERMINAL WIRING BOARD	
				TERMINAL	WIRE COLOR CODE		TERMINAL	WIRE COLOR CODE
1	$\emptyset 1$	Red	1	SP 1 R	14 / 10C - B - R	2	SP 1 R	14 / 15C - A - R
		Yellow		SP 1 Y	- O		SP 1 Y	- O
		Green		SP 1 G	- G		SP 1 G	- G
		Ground Wire		Grnd Bus	- W		Grnd Bus	- W
2	$\emptyset 2$	-----	3	SP 2 R	-----		SP 2 R	
		()		SP 2 Y	14 / 10C - D - O/B		SP 2 Y	
		()		SP 2 G	- G/B		SP 2 G	
		Ground Wire		Grnd Bus	- W/B		Grnd Bus	
3	$\emptyset 3$	Red	5	SP 3 R	14 / 10C - B - R/B	6	SP 3 R	14 / 15C - A - R / B
		Yellow		SP 3 Y	- O/B		SP 3 Y	- O / B
		Green		SP 3 G	- G/B		SP 3 G	- G / B
		Ground Wire		Grnd Bus	- W/B		Grnd Bus	- W / B
4	$\emptyset 4$	Red	7	SP 4 R	14 / 5C - C - R	8	SP 4 R	14 / 5C - E - R
		Yellow		SP 4 Y	- O		SP 4 Y	- O
		Green		SP 4 G	- G		SP 4 G	- G
		Ground Wire		Grnd Bus	- W		Grnd Bus	- W
5	$\emptyset 5$	Red	3	SP 5 R	14 / 10C - D - R	4	SP 5 R	14 / 5C - F - R
		Yellow		SP 5 Y	- O		SP 5 Y	- O
		Green		SP 5 G	- G		SP 5 G	- G
		Ground Wire		Grnd Bus	- W		Grnd Bus	- W
6	$OVL1,$ $\emptyset 2 + \emptyset 3$	-----	6	SP 6 R	-----		SP 6 R	
		()		SP 6 Y	14 / 15C - A - R / W		SP 6 Y	
		()		SP 6 G	- BL		SP 6 G	
		Ground Wire		Grnd Bus	- B		Grnd Bus	
7	$\emptyset 3$	-----	2	SP 7 R	-----		SP 7 R	
		()		SP 7 Y	14 / 15C - A - BL / W		SP 7 Y	
		()		SP 7 G	- G / W		SP 7 G	
		Ground Wire		Grnd Bus	- B / W		Grnd Bus	
8		-----		SP 8 R			SP 8 R	
		-----		SP 8 Y			SP 8 Y	
		-----		SP 8 G			SP 8 G	
		Ground Wire		Grnd Bus			Grnd Bus	
9		-----		SP 9 R			SP 9 R	
		-----		SP 9 Y			SP 9 Y	
		-----		SP 9 G			SP 9 G	
		Ground Wire		Grnd Bus			Grnd Bus	
10		-----		SP 10 R			SP 10 R	
		-----		SP 10 Y			SP 10 Y	
		-----		SP 10 G			SP 10 G	
		Ground Wire		Grnd Bus			Grnd Bus	
11	PED 1 $\emptyset 1$	HAND	21	SP 11 R	14 / 5C - 1P - R		SP 11 R	
		-----		SP 11 Y	-----		SP 11 Y	
		MAN		SP 11 G	14 / 5C - 1P - G		SP 11 G	
		Ground Wire		Grnd Bus	14 / 5C - 1P - W		Grnd Bus	
12	PED 2 $\emptyset 4$	HAND	22	SP 12 R	14 / 5C - 1P - R		SP 12 R	
		-----		SP 12 Y	-----		SP 12 Y	
		MAN		SP 12 G	14 / 5C - 1P - G		SP 12 G	
		Ground Wire		Grnd Bus	14 / 5C - 1P - W		Grnd Bus	
13		-----		SP 13 R			SP 13 R	
		-----		SP 13 Y			SP 13 Y	
		-----		SP 13 G			SP 13 G	
		Ground Wire		Grnd Bus			Grnd Bus	
14		-----		SP 14 R			SP 14 R	
		-----		SP 14 Y			SP 14 Y	
		-----		SP 14 G			SP 14 G	
		Ground Wire		Grnd Bus			Grnd Bus	

SIGNAL # R - 102

COUNTY # ROCKLAND DATE 11/08/12

DATE 11/08/12

CONFLICT / CURRENT MONITOR PROGRAMMING

NOTES:

Red Monitor on SP1, SP3, SP4, SP5

TE 262-12 (7/91)

MODEL 179 SIGNAL OPERATION
PROGRAMMABLE FEATURES
SIGNAL OPERATION SPECIFICATIONTAPS _____
STUDY # _____
FILE # _____
PAGE 3 OF 3SIGNAL # R - 102COUNTY ROCKLANDDATE 10/03/12

TABLE OF INPUT WIRING

TERM. NUMBER	FUNCTION	DET. NO.	DET. TYPE	DET. AN OVER	REMARKS
1A, 1B					
2A, 2B	$\emptyset 2$	2	NORMAL		PRESENCE LOOP
3A, 3B	$\emptyset 3$	3	NORMAL		PRESENCE LOOP
4A, 4B	$\emptyset 4$	4	QUAD		PRESENCE LOOP
5A, 5B					
6A, 6B					
7A, 7B					
8A, 8B					
9A, 9B					
10A, 10B					
11A, 11B					
12A, 12B					
13A, 13B	$\emptyset 3$	13	NORMAL		PRESENCE LOOP
14A, 14B	$\emptyset 4$	14	QUAD		PRESENCE LOOP
15A, 15B					
16A, 16B					
17A, 17B					
18A, 18B					
19A, 19B					
20A, 20B					
21A, 21B	PED 1, $\emptyset 1$	21	BUTTON		PED DETECTOR
22A, 22B	PED 2, $\emptyset 4$	22	BUTTON		PED DETECTOR
23A, 23B					
24A, 24B					
25A, 25B					
26A, 26B					
27A, 27B					
28A, 28B					

R-20

Signal #

STATE OF NEW YORK – DEPARTMENT OF TRANSPORTATION
TRAFFIC ENGINEERING SAFETY DIVISION
TRAFFIC CONTROL SPECIFICATION

Study:

Contract:

D262321

County of ROCKLAND

PIN:

8811.96

File:

39.15-303

INTERSECTION: RT 303 @ RT 340CITY VILLAGE TOWN OF: ORANGETOWNDepartment Order filed 8-15-1975 as Section: 2039.15 Subdivision: (d)Prior specification hereby superseded None Dated: November 2, 2011

Purpose: TRAFFIC SIGNAL REPLACEMENT UNDER CONTRACT D262321.

These specifications will be effective upon the Installation Modification / Reinstallation of the necessary traffic control device(s) required by and conforming to the Federal Manual on Uniform Traffic Control Devices.

This signal shall

- A. Operate in accordance with the table of operations and / or change intervals as shown on the attached pages as a:

Pretimed Signal

Semi-traffic actuated

Full-traffic actuated

Pedestrian actuated

Other

- B. Display vehicular indications

Display pedestrian indications

Be equipped with vehicle detectors

Be equipped with pedestrian buttons

as shown in the attached plans / drawings.

upgrade contract 4/14

- C. Be equipped with Pre-emption Interconnection and/or coordination which are described as follows:

Description:

IN COORDINATION WITH R-102.

cc: Region 8 Traffic Engineer
 Signal Shop
 Contract Maintainer
 Main Office

Date

Installation Date

NICHOLAS CHOUBAH R.T.E.

Signature

Title

Reinstallation/Modification

R-20

Signal #

STATE OF NEW YORK - DEPARTMENT OF TRANSPORTATION
TRAFFIC AND SAFETY DIVISION
TRAFFIC CONTROL SPECIFICATIONS (CONTINUED)

Signal:

R-20

Contract:

D262321

County of ROCKLAND

PIN:

8811.96

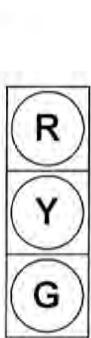
File:

39.15-303

FACES



12" + 12" + 12" + 12"



12" + 12" + 12" + 12"

Sp 6 Ø6
16
6
1Sp 1 Ø1
7
1

303



Old Orangeburgh Road

Ø3

Sp 3
13
3PED 1
SP 9 Ø3PED 4
Ø6
SP 12#24
A
C
#225
2
6
N
1
J
C
D
3
W
4
7
E
8#21
#23PED 3
Ø2
SP 11

340

#23

#22

SP 10
PED 2 Ø315
5
2
12Ø2
Sp 2

SIGN PANELS



Coordination Patterns [2.4] and Coordination Split Tables [2.7.1]																STD8										
Phase Times [1.1.1]								Path# Cyc Off Split Seq								Ring/Startup [1.1.4]										
Min Green	5	5	5	5	5	5	5	1	110	0	1	13	0	0	13	1	25	0	0	1	37	0	0	0	1	
Cap_Ext	2	2	2	2	2	2	2	2	0	0	2	1	14	0	0	14	1	26	0	0	1	38	0	0	1	
Max 1	15	30	35	30	30	30	30	3	0	0	3	1	15	0	0	15	1	27	0	0	1	39	0	0	1	
Max 2								4	0	0	4	1	16	0	0	16	1	28	0	0	1	40	0	0	1	
Yel Clearance	4	4	4	4	4	4	4	5	0	0	5	1	17	0	0	17	1	29	0	0	1	41	0	0	1	
Red Clearance	1	1	1	1	1	1	1	6	0	0	6	1	18	0	0	18	1	30	0	0	1	42	0	0	1	
Walk	7	7	7	7	7	7	7	7	0	0	7	1	19	0	0	19	1	31	0	0	1	43	0	0	1	
Ped Clearance	19	23	19	19	8	8	8	8	0	0	8	1	20	0	0	20	1	32	0	0	1	44	0	0	1	
Red Revert								9	0	0	9	1	21	0	0	21	1	33	0	0	1	45	0	0	1	
Add Initial								10	0	0	10	1	22	0	0	22	1	34	0	0	1	46	0	0	1	
Max Initial								11	0	0	11	1	23	0	0	23	1	35	0	0	1	47	0	0	1	
Time B4 Reduct								12	0	0	12	1	24	0	0	24	1	36	0	0	1	48	0	0	1	
Cars B4 Reduct								Split	1	2	3	4	5	6	7	8	Split	1	2	3	4	5	6	7	8	
Time To Reduce								1	Coord	20	50	40	0	20	50	40	0	13	Coord	0	0	0	0	0	0	0
Reduce By									2		MAX		MAX												Coord Modes [2.1]	
Min Gap									2	Coord	0	0	0	0	0	0	0	0	14	Coord	0	0	0	0	0	Test OptMode 0
DyMaxLim									3	Coord	0	0	0	0	0	0	0	0	15	Coord	0	0	0	0	0	SHRT/LNG MAX 1
Max Step									4	Coord	0	0	0	0	0	0	0	0	16	Coord	0	0	0	0	0	FLOAT
Options [1.1.2]	1	2	3	4	5	6	7	8																		
Enable	1	1	1	1	1	1	1	1	5	Coord	0	0	0	0	0	0	0	0	17	Coord	0	0	0	0	0	Closed Loop ON
Min Recall	1								6	Coord	0	0	0	0	0	0	0	0	18	Coord	0	0	0	0	0	Stop-in-Walk OFF
Max Recall									7	Coord	0	0	0	0	0	0	0	0	19	Coord	0	0	0	0	0	Auto Reset ON
Ped Recall									8	Coord	0	0	0	0	0	0	0	0	20	Coord	0	0	0	0	0	Expand Split OFF
Soft Recall									9	Coord	0	0	0	0	0	0	0	0	21	Coord	0	0	0	0	0	Ped Recycle NO_RECYLE
Lock Calls									10	Coord	0	0	0	0	0	0	0	0	22	Coord	0	0	0	0	0	Flash Red Before TIMED
Auto Flash Entity	1								11	Coord	0	0	0	0	0	0	0	0	23	Coord	0	0	0	0	0	Flash Red After Auto Flash PH OVER
Dual Entry	1	1	1	1	1	1	1	1	12	Coord	0	0	0	0	0	0	0	0	24	Coord	0	0	0	0	0	Flash Red 4.5
Enable Simul Gap	1	1	1	1	1	1	1	1	13	Coord	0	0	0	0	0	0	0	0	25	Coord	0	0	0	0	0	Unit Params [1.2.1]
Guarantee Passag									14	Coord	0	0	0	0	0	0	0	0	26	Coord	0	0	0	0	0	Phase Mode STD8
Rest In Walk									15	Coord	0	0	0	0	0	0	0	0	27	Coord	0	0	0	0	0	IO Mode USER
Condition Service									16	Coord	0	0	0	0	0	0	0	0	28	Coord	0	0	0	0	0	Loc Fish Start ON
Non Actuated 1									17	Coord	0	0	0	0	0	0	0	0	29	Coord	0	0	0	0	0	Start Flash(s) 0
Non Actuated 2									18	Coord	0	0	0	0	0	0	0	0	30	Coord	0	0	0	0	0	Start All(Red)s 0
Add Init Calc									19	Coord	0	0	0	0	0	0	0	0	31	Coord	0	0	0	0	0	Display Time 20
Options+ [1.1.3]	1	2	3	4	5	6	7	8	20	Coord	0	0	0	0	0	0	0	0	32	Coord	0	0	0	0	0	Red Revert 0
Reservice									21	Coord	0	0	0	0	0	0	0	0	33	Coord	0	0	0	0	0	MCE Timeout 0
PedCir Thru Yel									22	Coord	0	0	0	0	0	0	0	0	34	Coord	0	0	0	0	0	Feature Profile 0
Skip Red No Call									23	Coord	0	0	0	0	0	0	0	0	35	Coord	0	0	0	0	0	Free Ring Seq 1
Red Rest									24	Coord	0	0	0	0	0	0	0	0	36	Coord	0	0	0	0	0	Auxswitch STOPTM
Max II									25	Coord	0	0	0	0	0	0	0	0	37	Coord	0	0	0	0	0	SDLC Retry 0
Conflicting Phase									26	Coord	0	0	0	0	0	0	0	0	38	Coord	0	0	0	0	0	TS2 Det/Faults ON
Red Rest On Gap									27	Coord	0	0	0	0	0	0	0	0	39	Coord	0	0	0	0	0	Auto Ped Clear OFF
Omit Yellow									28	Coord	0	0	0	0	0	0	0	0	40	Coord	0	0	0	0	0	SDLC Retry 0
Ped Delay									29	Coord	0	0	0	0	0	0	0	0	41	Coord	0	0	0	0	0	RTE 303 @ RTE 340 (ID 5020) (Standard File)
Gm/Ped Delay									30	Coord	0	0	0	0	0	0	0	0	42	Coord	0	0	0	0	0	Page # 8 Misc - Events/Alarms; Call/Inhibit/Redirect; P/O/LAP Auto Flash; CIC; Misc Unit Param 04/02/14 Page 1A

Concurrency [11.14]

Sequence [11.24]

Phs	Concurrency Phases			
1	5	6	0	0
2	5	6	0	0
3	7	8	0	0
4	7	8	0	0
5	1	2	0	0
6	1	2	0	0
7	3	4	0	0
8	3	4	0	0
9	0	0	0	0
10	0	0	0	0
11	0	0	0	0
12	0	0	0	0
13	0	0	0	0
14	0	0	0	0
15	0	0	0	0
16	0	0	0	0

Seq	Ring	Concurrent Phases				Seq	Ring	Concurrent Phases			
1	1	1	1	2	3	4	0	0	0	0	0
2	1	2	5	6	7	8	0	0	0	0	0
3	1	3	0	0	0	0	0	0	0	0	0
4	1	4	0	0	0	0	0	0	0	0	0
5	2	1	1	2	3	4	0	0	0	0	0
6	2	2	6	5	7	8	0	0	0	0	0
7	3	4	0	0	0	0	0	0	0	0	0
8	3	4	0	0	0	0	0	0	0	0	0
9	0	0	0	0	0	0	0	0	0	0	0
10	0	0	0	0	0	0	0	0	0	0	0
11	0	0	0	0	0	0	0	0	0	0	0
12	0	0	0	0	0	0	0	0	0	0	0
13	0	0	0	0	0	0	0	0	0	0	0
14	0	0	0	0	0	0	0	0	0	0	0
15	0	0	0	0	0	0	0	0	0	0	0
16	0	0	0	0	0	0	0	0	0	0	0

Overland 1-16 Program Parms & Parmt [1521][1522]

Overlap 1-16 Program Params & Parm# [1.5.2.1][1.5.2.2]												Coord Transition, CoorPhs [2.5]														
Overlap Conflict Lock			OFF	Overlap Lock Inhibit	OFF	Parent Ph Clearance	ON	Extra Included Ph	ON	Path#			Short Long Dwell	No Shortway Ø	E-Yld	Offset	ReHld	Float	Min Ven Perm	Min Ped Perm						
Included Ø						Included Ø				1	12	22														
Modifier Ø						Gm	9	Modifier Ø		2	12	22														
Conflict Ø						Yel	3.5	Conflict Ø		3	12	22														
A	Conflict Olap					Red	1.5	-	Conflict Olap	4	12	22														
Conflict Ped						LG		Conflict Ped		5	12	22														
Included Ø						NORMAL		Included Ø		6	12	22														
2	Modifier Ø					Gm	10	Modifier Ø		7	12	22														
Conflict Ø						Yel	3.5	Conflict Ø		8	12	22														
B	Conflict Olap					Red	1.5	J	Conflict Olap	9	12	22														
Conflict Ped						LG		Conflict Ped		10	12	22														
Included Ø						NORMAL		Included Ø		11	12	22														
3	Modifier Ø					Gm	11	Modifier Ø		12	12	22														
Conflict Ø						Yel	3.5	Conflict Ø		13	12	22														
C	Conflict Olap					Red	1.5	K	Conflict Olap	14	12	22														
Conflict Ped						LG		Conflict Ped		15	12	22														
Included Ø						NORMAL		Included Ø		16	12	22														
4	Modifier Ø					Gm	12	Modifier Ø		17	12	22														
Conflict Ø						Yel	3.5	Conflict Ø		18	12	22														
D	Conflict Olap					Red	1.5	L	Conflict Olap	19	12	22														
Conflict Ped						LG		Conflict Ped		20	12	22														
Included Ø						NORMAL		Included Ø		21	12	22														
5	Modifier Ø					Gm	13	Modifier Ø		22	12	22														
Conflict Ø						Yel	3.5	Conflict Ø		23	12	22														
E	Conflict Olap					Red	1.5	M	Conflict Olap	24	12	22														
Conflict Ped						LG		Conflict Ped		25	0	0														
Included Ø						NORMAL		Included Ø		26	0	0														
6	Modifier Ø					Gm	14	Modifier Ø		27	0	0														
Conflict Ø						Yel	3.5	Conflict Ø		28	0	0														
F	Conflict Olap					Red	1.5	N	Conflict Olap	29	0	0														
Conflict Ped						LG		Conflict Ped		30	0	0														
Included Ø						NORMAL		Included Ø		31	0	0														
7	Modifier Ø					Gm	15	Modifier Ø		32	0	0														
Conflict Ø						Yel	3.5	Conflict Ø		33	0	0														
G	Conflict Olap					Red	1.5	O	Conflict Olap	34	0	0														
Conflict Ped						LG		Conflict Ped		35	0	0														
Included Ø						NORMAL		Included Ø		36	0	0														
8	Modifier Ø					Gm	16	Modifier Ø		37	0	0														
Conflict Ø						Yel	3.5	Conflict Ø		38	0	0														
H	Conflict Olap					Red	1.5	P	Conflict Olap	39	0	0														
Conflict Ped						LG		Conflict Ped		40	0	0														
Channel Settings [1.8.1]												41	0	0												
Channel ->	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24		
Phase / Olap #	1	2	3	6	3	2	6	3	Yel	3.5	Red	1.5	LG	5	12	22	23	24	42	0	0					
Channel Type	VEH	VEH	VEH	VEH	VEH	VEH	VEH	VEH	PEO	VEH	VEH	VEH	VEH	VEH	VEH	VEH	VEH	VEH	VEH	VEH	VEH	VEH	VEH	VEH		
Channel Flash	DRK	YEL	RED	RED	RED	YEL	RED	RED	DRK	DRK	DRK	DRK	DRK	DRK	DRK	DRK	DRK	DRK	DRK	DRK	DRK	DRK	DRK	DRK	DRK	
All Hz																			46	0	0					
Channel+ Settings [1.8.4]																			47	0	0					
Channel ->	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24		
Phase GRN																			43	0	0					
Conflict GRN																			44	0	0					
Conflict Ped																			45	0	0					
Conflict Ped																			46	0	0					
Conflict Ped																			47	0	0					

Flash Bed+

11

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10

NG666

10

Cap Ovrd

10244

100

RTE 303 RE KITE 300 LTD Statute of Frauds

Page 2

104

卷之三

10

Preemption Times [3.1], Options+ [3.6]			
Pre #	Enable	Type	Output
1	ON	RAIL	DWELL
2	ON	RAIL	DWELL
3	ON	EMERG	DWELL
4	ON	EMERG	DWELL
5	ON	EMERG	DWELL
6	ON	EMERG	DWELL

Pre #	MaxPres	MinGm	MinWik	PedCir	Co+Pre
1	ON				
2	ON				
3	ON				
4	ON				
5	ON				
6	ON				

Low Priority Preempts	Pre #	Type	Min	Max
7	OFF	0	0	
8	OFF	0	0	
9	OFF	0	0	
10	OFF	0	0	

Unit Parameters [1.2.1]	Stop Timer Over Preempt	OFF	PRE
	Preempt or Ext Output		
	Max Seek Track Time	0	
	Max Seek Dwell Time	0	
Channel Parameters [1.8.3]	D Conn Mappings	NONE	
	Pre Invert Rail Input		

Adv Timers [3.8]	enterYellowChange	
	enterRedCir	trackRedCir
	trackYellowCir	AllRedB4Dwell

Track Clear Phases [3.2], Track Clear Overlaps+ [3.5]

Pre #	Track	Gm	Min Dwell	Ext Dwell	PedCir	Co+Pre
1	1	ON				
2	2	ON				
3	3	ON				
4	4	ON				
5	5	ON				
6	6	ON				

Pre #	Lock	Override	Higher	Fish	Dwell	Link
1	ON	ON	ON	ON	OFF	
2	ON	ON	ON	ON	ON	
3	ON	ON	ON	ON	ON	
4	ON	ON	ON	ON	ON	
5	ON	ON	ON	ON	ON	
6	ON	ON	ON	ON	ON	

Pre #	Exit Phase	Pre #	Lock	Override	Higher	Fish	Dwell	Link
1		1	ON	ON	ON	ON	OFF	
2		2	ON	ON	ON	ON	ON	
3		3	ON	ON	ON	ON	ON	
4		4	ON	ON	ON	ON	ON	
5		5	ON	ON	ON	ON	ON	
6		6	ON	ON	ON	ON	ON	

Init' Dwell [3.9]	Phases	Peds	Overlap

Column#....>	1	2	3	4	5	6	7	8
Assign Ø								
Min Gm								
Gap, Ext								
Max 1								
Max 2								
Yel CIR								
Red CIR								
Walk								
Ped CIR								

Column#....>	1	2	3	4	5	6	7	8
Assign Ø								
Min Gm								
Gap, Ext								
Max 1								
Max 2								
Yel CIR								
Red CIR								
Walk								
Ped CIR								

Column#....>	1	2	3	4	5	6	7	8
Assign Ø								
Dual Entry								
Enabl SimGap	1	1	1	1	1	1	1	1
Gaur Passage								
Rest In Walk								
Cond Service								
Reservice								
Non-Act1								
Red Res1								

Column # >	1	2	3	4	5	6	7	8
Lock Calls	1	1	1	1	1	1	1	1
Soft Recall								
Dual Entry								
Enabl SimGap	1	1	1	1	1	1	1	1
Gaur Passage								
Rest In Walk								
Cond Service								
Reservice								
Non-Act1								
Red Res1								

Annual Schedule [4.3] Month of Year												Date								1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31	Link To	Day Plan	1
1	1	1	1	1	1	1	1	1	1	1	1		1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1										
2	J	F	M	A	M	J	J	A	S	O	N		S	M	T	W	T	F	S	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31			1
3	J	F	M	A	M	J	J	A	S	O	N		S	M	T	W	T	F	S	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31			1
4	J	F	M	A	M	J	J	A	S	O	N		S	M	T	W	T	F	S	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31			1
5	J	F	M	A	M	J	J	A	S	O	N		S	M	T	W	T	F	S	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31			1
6	J	F	M	A	M	J	J	A	S	O	N		S	M	T	W	T	F	S	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31			1
7	J	F	M	A	M	J	J	A	S	O	N		S	M	T	W	T	F	S	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31			1
8	J	F	M	A	M	J	J	A	S	O	N		S	M	T	W	T	F	S	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31			1
9	J	F	M	A	M	J	J	A	S	O	N		S	M	T	W	T	F	S	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31			1
10	J	F	M	A	M	J	J	A	S	O	N		S	M	T	W	T	F	S	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31			1
11	J	F	M	A	M	J	J	A	S	O	N		S	M	T	W	T	F	S	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31			1
12	J	F	M	A	M	J	J	A	S	O	N		S	M	T	W	T	F	S	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31			1
13	J	F	M	A	M	J	J	A	S	O	N		S	M	T	W	T	F	S	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31			1
14	J	F	M	A	M	J	J	A	S	O	N		S	M	T	W	T	F	S	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31			1
15	J	F	M	A	M	J	J	A	S	O	N		S	M	T	W	T	F	S	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31			1
16	J	F	M	A	M	J	J	A	S	O	N		S	M	T	W	T	F	S	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31			1
17	J	F	M	A	M	J	J	A	S	O	N		S	M	T	W	T	F	S	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31			1
18	J	F	M	A	M	J	J	A	S	O	N		S	M	T	W	T	F	S	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31			1
19	J	F	M	A	M	J	J	A	S	O	N		S	M	T	W	T	F	S	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31			1
20	J	F	M	A	M	J	J	A	S	O	N		S	M	T	W	T	F	S	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31			1
21	J	F	M	A	M	J	J	A	S	O	N		S	M	T	W	T	F	S	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31			1
22	J	F	M	A	M	J	J	A	S	O	N		S	M	T	W	T	F	S	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31			1
23	J	F	M	A	M	J	J	A	S	O	N		S	M	T	W	T	F	S	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31			1
24	J	F	M	A	M	J	J	A	S	O	N		S	M	T	W	T	F	S	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31			1

Day Plans [4.4]

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C1-USER IO Map [1.8.9.1 In]		C1-USER IO Map [1.8.9.2 Out]		IO Logic [1.8.9.2 Out]	
11-1	1	Veh Call 1	0-1	1	Ch1 Red
11-2	2	Veh Call 2	0-2	49	Ch1 Green
11-3	3	Veh Call 3	0-3	2	Ch2 Red
11-4	4	Veh Call 4	0-4	26	Ch2 Yellow
11-5	5	Veh Call 5	0-5	50	Ch2 Green
11-6	6	Veh Call 6	0-6	3	Ch3 Red
11-7	7	Veh Call 7	0-7	27	Ch3 Yellow
11-8	189	Unused	0-8	51	Ch3 Green
12-1	189	Unused	0-1	4	Ch4 Red
12-2	189	Unused	0-2	52	Ch4 Green
12-3	189	Unused	0-3	5	Ch5 Red
12-4	12	Veh Call 12	0-4	29	Ch5 Yellow
12-5	13	Veh Call 13	0-5	53	Ch5 Green
12-6	14	Veh Call 14	0-6	6	Ch6 Red
12-7	15	Veh Call 15	0-7	30	Ch6 Yellow
12-8	16	Veh Call 16	0-8	54	Ch6 Green
13-1	189	Unused	0-1	7	Ch7 Red
13-2	189	Unused	0-2	55	Ch7 Green
13-3	189	Unused	0-3	8	Ch8 Red
13-4	189	Unused	0-4	32	Ch8 Yellow
13-5	129	Ped Call 1	0-5	56	Ch8 Green
13-6	130	Ped Call 2	0-6	9	Ch9 Red
13-7	131	Ped Call 3	0-7	33	Ch9 Yellow
13-8	132	Ped Call 4	0-8	57	Ch9 Green
14-1	10	33xCMUStop	0-1	10	Ch10 Red
14-2	58	33xFlashSns	0-2	58	Ch10 Green
14-3	11	Ch11 Red	0-3	11	Ch11 Yellow
14-4	35	Ch11 Green	0-4	35	Ch11 Green
14-5	189	Unused	0-5	59	Ch12 Red
14-6	189	Unused	0-6	12	Ch12 Green
14-7	229	33xCMUStop	0-7	36	Ch13 Yellow
14-8	228	33xFlashSns	0-8	60	Ch13 Green
15-1	189	Unused	0-9	28	Ch4 Yellow
15-2	189	Unused	0-10	34	Ch10 Yellow
15-3	189	Unused	0-11	25	Ch11 Red
15-4	189	Unused	0-12	31	Ch11 Green
15-5	189	Unused	0-13	54	Ch12 Red
15-6	189	Unused	0-14	36	Ch12 Green
15-7	189	Unused	0-15	115	Not Used
15-8	189	Unused	0-16	114	Watchdog
16-1	189	Unused	0-17	115	Not Used
16-2	189	Unused	0-18	115	Not Used
16-3	189	Unused	0-19	13	Ch13 Red
16-4	189	Unused	0-20	37	Ch13 Yellow
16-5	189	Unused	0-21	61	Ch13 Green
16-6	189	Unused	0-22	14	Ch14 Red
16-7	189	Unused	0-23	38	Ch14 Yellow
16-8	189	Unused	0-24	62	Ch14 Green

C1-USER IO Map [1.8.9.1 In]		C1-USER IO Map [1.8.9.2 Out]		IO Logic [1.8.9.2 Out]	
07-1	115	Not Used	07-2	115	Not Used
07-3	115	Not Used	07-4	115	Not Used
07-5	115	Not Used	07-6	115	Not Used
07-7	115	Not Used	07-8	115	Not Used
C11S-USER IO Map [1.8.9.1 In]		C11S-USER IO Map [1.8.9.2 Out]		IO Logic [1.8.9.2 Out]	
14-1	0	=	14-2	0	=
14-3	0	=	14-4	189	Unused
14-5	0	=	14-6	189	Unused
14-7	0	=	14-8	189	Unused
14-9	0	=	14-10	189	Unused
14-11	0	=	14-12	189	Unused
14-13	0	=	14-14	189	Unused
14-15	0	=	14-16	189	Unused
14-17	0	=	14-18	189	Unused
14-19	0	=	14-20	189	Unused
14-21	0	=	14-22	189	Unused
C11S Connector		C11S-USER IO Map [1.8.9.2 Out]		IO Logic [1.8.9.2 Out]	
18-1	115	Not Used	18-2	115	Not Used
18-3	115	Not Used	18-4	115	Not Used
18-5	115	Not Used	18-6	115	Not Used
18-7	189	Unused	18-8	189	Unused
18-9	189	Unused	18-10	189	Unused
18-11	189	Unused	18-12	189	Unused
18-13	189	Unused	18-14	189	Unused
18-15	189	Unused	18-16	189	Unused
18-17	189	Unused	18-18	189	Unused
18-19	189	Unused	18-20	189	Unused
18-21	189	Unused	18-22	189	Unused
2070 Port Binding Functions [6.6]		2070 IP 1 Addressing [6.5]		2070 IP 2 Addressing [6.5]	
Port	Echo	Port	Addressing	Port	Addressing
SP1	None	SP1	None	SP1	None
SP2	None	SP2	None	SP2	None
SP3	None	SP3	None	SP3	None
SP4	None	SP4	None	SP4	None
SP5	None	SP5	None	SP5	None
SP6	None	SP6	None	SP6	None
SP7	None	SP7	None	SP7	None
SP8	None	SP8	None	SP8	None
2070 Port Binding Functions [6.6]		2070 IP 1 Addressing [6.5]		2070 IP 2 Addressing [6.5]	
Function	Channel	Function	Channel	Function	Channel
TS2/CVM	NONE	TS2/CVM	NONE	SYSUp	ASYNc2
CNUJM/JU	NONE	CNUJM/JU	NONE	SYSDown	ASYNc1
Ojicom	NONE	Ojicom	NONE	Shell	NONE
Loop Det.	NONE	Loop Det.	NONE	GPS	NONE
SPS	NONE	SPS	NONE	SPS	NONE
SYNC2	OFF	SYNC2	OFF	SYNC2	OFF

#	Event / Alarm	Ev All	Call Phases [1.1.5]		Redirect Phases [1.1.5]		Inhibit Phases [1.1.5]		
			Phases Called By Ø	From	To	From	To	From	To
1	Power Up Alarm.	1 1	Ø	1	2	3	4	5	6
2	Stop Timing	1 1	2	3	4	5	6	7	8
3	TS1 Cabinet Door	1 1	2	3	4	5	6	7	8
4	Coordination Failure	1 1	3	4	5	6	7	8	9
5	External Alarm # 1	1 1	4	5	6	7	8	9	10
6	External Alarm # 2	1 1	5	6	7	8	9	10	11
7	External Alarm # 3	1	6	7	8	9	10	11	12
8	External Alarm # 4	1	7	8	9	10	11	12	13
9	Closed Loop Disabled	1	8	9	10	11	12	13	14
10	External Alarm # 5	1	9	10	11	12	13	14	15
11	External Alarm # 6	1	10	11	12	13	14	15	16
12	Manual Control Enable	1 1	11	12	13	14	15	16	
13	Coord Free Input	1	12	13	14	15	16		
14	Local Flash Input	1 1	13	14	15	16			
15	MMU Flash	1	14	15	16				
16	CMU Flash	1							
17	Cycle Fault	1							
18	Cycle Failure	1	Alt Call & Redirect # 1 [1.1.6.3]		From To From To From To		Alt Inhibit Phases # 1 [1.1.6.3]		
19	Coordination Fault	1	Ø	1	2	3	4	5	6
20	Controller Fault	1 1	1	2	3	4	5	6	7
21	Detector SDLC Failure	1	2	3	4	5	6	7	8
22	MMU SDLC Failure	1	3	4	5	6	7	8	
23	Critical SDLC Failure	1	4	5	6	7	8		
24	Reserved	1	5	6	7	8			
25	EEPROM CRC Fault	1 1	6	7	8				
26	Detector Diagnostic Failure	1	7	8					
27	BIU Detector Failure	1 1							
28	Queue detector alarm	1	Alt Call & Redirect # 2 [1.1.6.3]		From To From To From To		Alt Inhibit Phases # 2 [1.1.6.3]		
29	Ped Detector Fault	1	Ø	1	2	3	4	5	6
30	Coord Diagnostic Fault	1	1	2	3	4	5	6	7
41	TempAlert Probe Ch. A	1	2	3	4	5	6	7	8
42	TempAlert Probe Ch. B	1	3	4	5	6	7	8	
47	Coord Active	1	4	5	6	7	8		
48	Preempt Active	1	5	6	7	8			
49	Preempt 1 Input	1	6	7	8				
50	Preempt 2 Input	1	7	8					
51	Preempt 3 Input	1							
52	Preempt 4 Input	1	Coord, CIC Plans [2.3]		From To From To		Unit Parameters [1.2.1]		
53	Preempt 5 Input	1	CIC Co@	Grow	1 2 3 4 5 6 7 8	Allow Skip Yellow	0	Max Cycle Time	0
54	Preempt 6 Input	1	1 OFF			TOD Dim Enable	OFF	Cycle Fault Action	ALARM
55	Preempt 7 Input	1	2 OFF			Tone Disable	OFF		
56	Preempt 8 Input	1	3 OFF			Diamond Mode	4Ph		
57	Preempt 9 Input	1	4 OFF			Backup Time (s)	900		
58	Preempt 10 Input	1				Disable Init Ped	OFF		
61	In Transition	1	Yel Ø			Cycle Fault Action	ALARM		
81	FIO Status Alarm	1	Yel (olaps)			Enable Run Timer	ON	RTE 303 @ RTE 340 (ID 5020) (Standard File)	

R-20

Signal #

**MODEL 2070 SIGNAL OPERATION
PROGRAMMABLE FEATURES
SIGNAL OPERATION SPECIFICATION**

R-20

Signal:

R-20

Contract:

D262321

PIN:

8811.96

File:

39.15-303

Date: 7/9/2015

County of ROCKLAND**TABLE OF SWITCH PACKS**

SWITCH PACK	FUNCTION	INDICATIONS	FACE	TERMINAL WIRING BOARD		FACE	TERMINAL WIRING BOARD	
				TERMINAL	WIRE COLOR CODE		TERMINAL	WIRE COLOR CODE
1	Ø1	-----	1	SP 1 R	-----		SP 1 R	
				SP 1 Y	14 / 15C - B - O / B		SP 1 Y	
				SP 1 G	- G / B		SP 1 G	
		Ground Wire		Grnd Bus	- W / B		Grnd Bus	
		Red	3	SP 2 R	14 / 10C - C - R	4	SP 2 R	14 / 10C - D - R
2	Ø2	Yellow		SP 2 Y	- O		SP 2 Y	- O
		Green		SP 2 G	- G		SP 2 G	- G
		Ground Wire		Grnd Bus	- W		Grnd Bus	- W
3	Ø3	Red	5	SP 3 R	14 / 15C - B - R / W	6	SP 3 R	14 / 10C - A - R / B
		Yellow		SP 3 Y	- BL / W		SP 3 Y	- O / B
		Green		SP 3 G	- G / W		SP 3 G	- G / B
		Ground Wire		Grnd Bus	- B / W		Grnd Bus	- W / B
4				SP 4 R			SP 4 R	
				SP 4 Y			SP 4 Y	
				SP 4 G			SP 4 G	
		Ground Wire		Grnd Bus			Grnd Bus	
5				SP 5 R			SP 5 R	
				SP 5 Y			SP 5 Y	
				SP 5 G			SP 5 G	
		Ground Wire		Grnd Bus			Grnd Bus	
6	Ø6	Red	1	SP 6 R	14 / 15C - B - R	2	SP 6 R	14 / 10C - A - R / B
		Yellow		SP 6 Y	- O		SP 6 Y	- O / B
		Green		SP 6 G	- G		SP 6 G	- G / B
		Ground Wire		Grnd Bus	- W		Grnd Bus	- W / B
7				SP 7 R			SP 7 R	
				SP 7 Y			SP 7 Y	
				SP 7 G			SP 7 G	
		Ground Wire		Grnd Bus			Grnd Bus	
8				SP 8 R			SP 8 R	
				SP 8 Y			SP 8 Y	
				SP 8 G			SP 8 G	
		Ground Wire		Grnd Bus			Grnd Bus	
9	PED 1 Ø3	HAND	21	SP 9 R	14 / 5C - 1P - R		SP 9 R	
		-----		SP 9 Y			SP 9 Y	
		MAN		SP 9 G	14 / 5C - 1P - G		SP 9 G	
		Ground Wire		Grnd Bus	14 / 5C - 1P - W		Grnd Bus	
10	PED 2 Ø3	HAND	22	SP 10 R	14 / 5C - 2P - R		SP 10 R	
		-----		SP 10 Y	-----		SP 10 Y	
		MAN		SP 10 G	14 / 5C - 2P - G		SP 10 G	
		Ground Wire		Grnd Bus	14 / 5C - 2P - W		Grnd Bus	
11	PED 3 Ø2	HAND	23	SP 11 R	14 / 5C - 3P - R		SP 11 R	
		-----		SP 11 Y	-----		SP 11 Y	
		MAN		SP 11 G	14 / 5C - 3P - G		SP 11 G	
		Ground Wire		Grnd Bus	14 / 5C - 3P - W		Grnd Bus	
12	PED 4 Ø6	HAND	24	SP 12 R	14 / 5C - 4P - R		SP 12 R	
		-----		SP 12 Y	-----		SP 12 Y	
		MAN		SP 12 G	14 / 5C - 4P - G		SP 12 G	
		Ground Wire		Grnd Bus	14 / 5C - 4P - W		Grnd Bus	
13	Ø3	Red	7	SP 13 R	14 / 10C - C - R / B	8	SP 13 R	14 / 10C - D - R / B
		Yellow		SP 13 Y	- O / B		SP 13 Y	- O / B
		Green		SP 13 G	- G / B		SP 13 G	- G / B
		Ground Wire		Grnd Bus	- W / B		Grnd Bus	- W / B
14				SP 14 R			SP 14 R	
				SP 14 Y			SP 14 Y	
				SP 14 G			SP 14 G	
		Ground Wire		Grnd Bus			Grnd Bus	

R-20

Signal #

**MODEL 2070 SIGNAL OPERATION
PROGRAMMABLE FEATURES
SIGNAL OPERATION SPECIFICATION**

R-20

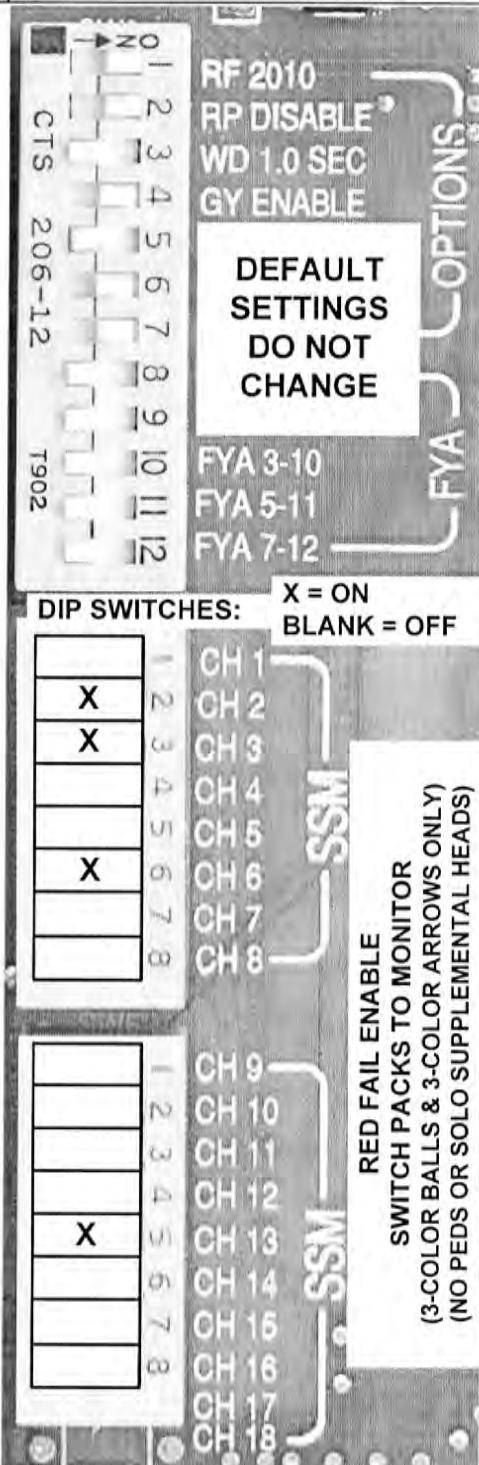
Signal: _____
 Contract: D262321
 PIN: 8811.96
 File: 39.15-303
 Date: 7/9/2015

County of ROCKLAND**TRAFFIC SIGNAL MONITOR PROGRAMMING**

CONFLICT MONITOR DIODES TO BE CUT (SWITCH PACKS TO RUN TOGETHER)			YELLOW DISABLE: WIRE JUMPERS TO BE INSTALLED FOR PEDS	210NYR MONITOR BOARD (SWITCH PACKS TO MONITOR)
SP1-SP6	SP10-SP13		1	
SP1-SP12			2	
	SP11-SP12		3	
SP2-SP6			4	
SP2-SP11			5	
SP2-SP12			6	
			7	
SP3-SP9			8	
SP3-SP10			9 X	
SP3-SP13			10 X	
			11 X	
SP6-SP11			12 X	
SP6-SP12			13	
			14	
SP9-SP10			15	
SP9-SP13			16	

CURRENT MONITOR BOARD (IF USED)	
CURRENT MONITOR DIODES TO BE CUT (SWITCH PACKS TO NOT MONITOR)	
1, 4, 5, 7-12, 14-16	

Notes:



R-20

Signal #

**MODEL 2070 SIGNAL OPERATION
PROGRAMMABLE FEATURES
SIGNAL OPERATION SPECIFICATION**

County of ROCKLAND

Signal:	R-20
Contract:	D262321
PIN:	8811.96
File:	39.15-303
Date:	7/9/2015

TABLE OF INPUT WIRING

TERM. NUMBER	FUNCTION	DET. NO.	DET. TYPE	DET. AN OVER	REMARKS
1A, 1B	Ø1	1	QUAD		PRESENCE LOOP
2A, 2B	Ø2	2	QUAD		PRESENCE LOOP
3A, 3B	Ø3	3	QUAD		PRESENCE LOOP
4A, 4B	Ø3	4	QUAD		PRESENCE LOOP
5A, 5B	Ø2	5	QUAD		PRESENCE LOOP
6A, 6B	Ø6	6	QUAD		PRESENCE LOOP
7A, 7B	Ø1	7	NORMAL		PRESENCE LOOP
8A, 8B					
9A, 9B					
10A, 10B					
11A, 11B					
12A, 12B	Ø2	12	NORMAL		PRESENCE LOOP
13A, 13B	Ø3	13	NORMAL		PRESENCE LOOP
14A, 14B	Ø3	14	NORMAL		PRESENCE LOOP
15A, 15B	Ø2	15	NORMAL		PRESENCE LOOP
16A, 16B	Ø6	16	NORMAL		PRESENCE LOOP
17A, 17B					
18A, 18B					
19A, 19B					
20A, 20B					
21A, 21B	PED 1, Ø3	21	BUTTON		PED DETECTOR
22A, 22B	PED 2, Ø3	22	BUTTON		PED DETECTOR
23A, 23B	PED 3, Ø2	23	BUTTON		PED DETECTOR
24A, 24B	PED 4, Ø6	24	BUTTON		PED DETECTOR
25A, 25B					
26A, 26B					
27A, 27B					
28A, 28B					

Appendix C

Capacity Analysis

2686-99-013T

Existing - AM

10: Route 303 & Mountainview Avenue

	→	→	←	←	←	↑	↑	↓	↓	↗	↙	
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	60	46	105	65	45	11	118	534	57	9	528	64
Future Volume (vph)	60	46	105	65	45	11	118	534	57	9	528	64
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	10	10	10	10	10	10
Grade (%)	-3%				-2%			0%			-1%	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Frt		0.933			0.988			0.988			0.984	
Flt Protected		0.986			0.974			0.992			0.999	
Satd. Flow (prot)	0	1724	0	0	1651	0	0	2968	0	0	2950	0
Flt Permitted		0.847			0.538			0.992			0.999	
Satd. Flow (perm)	0	1481	0	0	912	0	0	2968	0	0	2950	0
Right Turn on Red			Yes			Yes			No			Yes
Satd. Flow (RTOR)		42			4						12	
Link Speed (mph)		30			30			40			40	
Link Distance (ft)		498			709			1377			1086	
Travel Time (s)		11.3			16.1			23.5			18.5	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	2%	4%	3%	11%	0%	64%	7%	13%	4%	50%	13%	6%
Adj. Flow (vph)	65	49	113	70	48	12	127	574	61	10	568	69
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	227	0	0	130	0	0	762	0	0	647	0
Turn Type	Perm	NA		Perm	NA		Split	NA		Split	NA	
Protected Phases		4			8		6	6		2	2	
Permitted Phases		4			8							
Detector Phase	4	4		8	8		6	6		2	2	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		34.0	34.0		5.0	5.0	
Minimum Split (s)	10.0	10.0		10.0	10.0		40.0	40.0		11.0	11.0	
Total Split (s)	30.0	30.0		30.0	30.0		40.0	40.0		40.0	40.0	
Total Split (%)	27.3%	27.3%		27.3%	27.3%		36.4%	36.4%		36.4%	36.4%	
Yellow Time (s)	4.0	4.0		4.0	4.0		5.0	5.0		5.0	5.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)		0.0			0.0		0.0			0.0		
Total Lost Time (s)		5.0			5.0			6.0			6.0	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None		None	None		None	None		C-Max	C-Max	
Act Effct Green (s)		17.9			17.9			34.0			41.1	
Actuated g/C Ratio		0.16			0.16			0.31			0.37	
v/c Ratio		0.82			0.86			0.83			0.58	
Control Delay		58.6			85.7			49.2			30.8	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		58.6			85.7			49.2			30.8	
LOS		E			F			D			C	
Approach Delay		58.6			85.7			49.2			30.8	
Approach LOS		E			F			D			C	
Queue Length 50th (ft)		128			87			293			187	
Queue Length 95th (ft)		203			#152			#364			275	

2686-99-013T

Existing - AM
10: Route 303 & Mountainview Avenue



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Internal Link Dist (ft)	418			629			1297			1006		
Turn Bay Length (ft)												
Base Capacity (vph)	369				210			917			1109	
Starvation Cap Reductn	0				0			0			0	
Spillback Cap Reductn	0				0			0			0	
Storage Cap Reductn	0				0			0			0	
Reduced v/c Ratio	0.62				0.62			0.83			0.58	

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 5 (5%), Referenced to phase 2:SBTL, Start of Yellow

Natural Cycle: 80

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.86

Intersection Signal Delay: 46.4

Intersection LOS: D

Intersection Capacity Utilization 72.3%

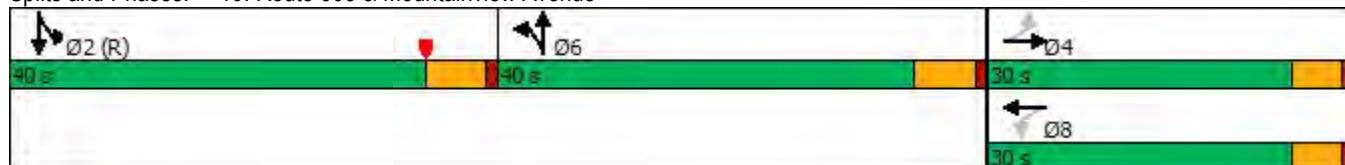
ICU Level of Service C

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 10: Route 303 & Mountainview Avenue



2686-99-013T

Existing - PM

10: Route 303 & Mountainview Avenue

	←	→	↙	↗	↖	↙	↖	↑	↗	↙	↓	↖
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	52	34	89	126	36	10	161	682	78	11	792	37
Future Volume (vph)	52	34	89	126	36	10	161	682	78	11	792	37
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	10	10	10	10	10	10
Grade (%)	-3%				-2%			0%			-1%	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Frt		0.931			0.992			0.987			0.993	
Flt Protected		0.985			0.965			0.991			0.999	
Satd. Flow (prot)	0	1733	0	0	1798	0	0	3065	0	0	3184	0
Flt Permitted		0.871			0.560			0.991			0.999	
Satd. Flow (perm)	0	1533	0	0	1043	0	0	3065	0	0	3184	0
Right Turn on Red			Yes			Yes			No			Yes
Satd. Flow (RTOR)		44			3						4	
Link Speed (mph)		30			30			40			40	
Link Distance (ft)		498			709			1377			1086	
Travel Time (s)		11.3			16.1			23.5			18.5	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	0%	0%	4%	2%	0%	12%	3%	9%	4%	50%	5%	3%
Adj. Flow (vph)	56	37	96	135	39	11	173	733	84	12	852	40
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	189	0	0	185	0	0	990	0	0	904	0
Turn Type	Perm	NA		Perm	NA		Split	NA		Split	NA	
Protected Phases		4			8		6	6		2	2	
Permitted Phases	4			8								
Detector Phase	4	4		8	8		6	6		2	2	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		34.0	34.0		5.0	5.0	
Minimum Split (s)	10.0	10.0		10.0	10.0		40.0	40.0		11.0	11.0	
Total Split (s)	30.0	30.0		30.0	30.0		40.0	40.0		40.0	40.0	
Total Split (%)	27.3%	27.3%		27.3%	27.3%		36.4%	36.4%		36.4%	36.4%	
Yellow Time (s)	4.0	4.0		4.0	4.0		5.0	5.0		5.0	5.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)		0.0		0.0			0.0			0.0		
Total Lost Time (s)		5.0			5.0			6.0			6.0	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None		None	None		None	None		C-Max	C-Max	
Act Effct Green (s)		21.4			21.4			34.0			37.6	
Actuated g/C Ratio		0.19			0.19			0.31			0.34	
v/c Ratio		0.57			0.90			1.05			0.83	
Control Delay		36.7			84.4			73.5			41.8	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		36.7			84.4			73.5			41.8	
LOS		D			F			E			D	
Approach Delay		36.7			84.4			73.5			41.8	
Approach LOS		D			F			E			D	
Queue Length 50th (ft)		90			123			~406			314	
Queue Length 95th (ft)		161			#239			#541			#445	

2686-99-013T

Existing - PM
10: Route 303 & Mountainview Avenue



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Internal Link Dist (ft)	418			629			1297			1006		
Turn Bay Length (ft)												
Base Capacity (vph)	382			239			947			1091		
Starvation Cap Reductn	0			0			0			0		
Spillback Cap Reductn	0			0			0			0		
Storage Cap Reductn	0			0			0			0		
Reduced v/c Ratio	0.49			0.77			1.05			0.83		

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 5 (5%), Referenced to phase 2:SBTL, Start of Yellow

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.05

Intersection Signal Delay: 58.7 Intersection LOS: E

Intersection Capacity Utilization 86.5% ICU Level of Service E

Analysis Period (min) 15

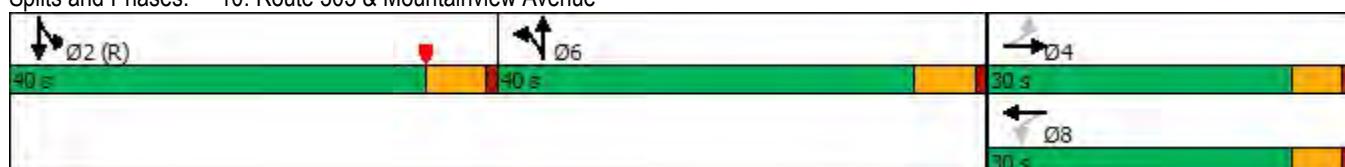
~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 10: Route 303 & Mountainview Avenue



2686-99-013T

No Build - AM

10: Route 303 & Mountainview Avenue

	→	→	←	←	←	↑	↑	↓	↓	←		
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	61	55	107	85	51	40	120	608	93	39	571	65
Future Volume (vph)	61	55	107	85	51	40	120	608	93	39	571	65
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	10	10	10	10	10	10
Grade (%)	-3%				-2%			0%			-1%	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Frt		0.935			0.969			0.983			0.986	
Flt Protected		0.986			0.976			0.993			0.997	
Satd. Flow (prot)	0	1727	0	0	1514	0	0	2960	0	0	2908	0
Flt Permitted		0.830			0.597			0.993			0.997	
Satd. Flow (perm)	0	1453	0	0	926	0	0	2960	0	0	2908	0
Right Turn on Red			Yes			Yes			No			Yes
Satd. Flow (RTOR)		39			12						11	
Link Speed (mph)		30			30			40			40	
Link Distance (ft)		498			709			1377			1086	
Travel Time (s)		11.3			16.1			23.5			18.5	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	2%	4%	3%	11%	0%	64%	7%	13%	4%	50%	13%	6%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	240	0	0	189	0	0	883	0	0	726	0
Turn Type	Perm	NA		Perm	NA		Split	NA		Split	NA	
Protected Phases		4			8		6	6		2	2	
Permitted Phases		4			8							
Detector Phase		4	4		8	8		6	6		2	2
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		34.0	34.0		5.0	5.0	
Minimum Split (s)	10.0	10.0		10.0	10.0		40.0	40.0		11.0	11.0	
Total Split (s)	30.0	30.0		30.0	30.0		40.0	40.0		40.0	40.0	
Total Split (%)	27.3%	27.3%		27.3%	27.3%		36.4%	36.4%		36.4%	36.4%	
Yellow Time (s)	4.0	4.0		4.0	4.0		5.0	5.0		5.0	5.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)		0.0			0.0			0.0			0.0	
Total Lost Time (s)		5.0			5.0			6.0			6.0	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None		None	None		None	None		C-Max	C-Max	
Act Effct Green (s)		22.8			22.8			34.0			36.2	
Actuated g/C Ratio		0.21			0.21			0.31			0.33	
v/c Ratio		0.73			0.94			0.97			0.75	
Control Delay		46.7			91.0			60.5			38.9	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		46.7			91.0			60.5			38.9	
LOS		D			F			E			D	
Approach Delay		46.7			91.0			60.5			38.9	
Approach LOS		D			F			E			D	
Queue Length 50th (ft)		130			122			345			243	
Queue Length 95th (ft)		220			#254			#468			319	
Internal Link Dist (ft)		418			629			1297			1006	

2686-99-013T

No Build - AM

10: Route 303 & Mountainview Avenue



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)												
Base Capacity (vph)	360			219			914			965		
Starvation Cap Reductn	0			0			0			0		
Spillback Cap Reductn	0			0			0			0		
Storage Cap Reductn	0			0			0			0		
Reduced v/c Ratio	0.67			0.86			0.97			0.75		

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 5 (5%), Referenced to phase 2:SBTL, Start of Yellow

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.97

Intersection Signal Delay: 54.0

Intersection LOS: D

Intersection Capacity Utilization 78.6%

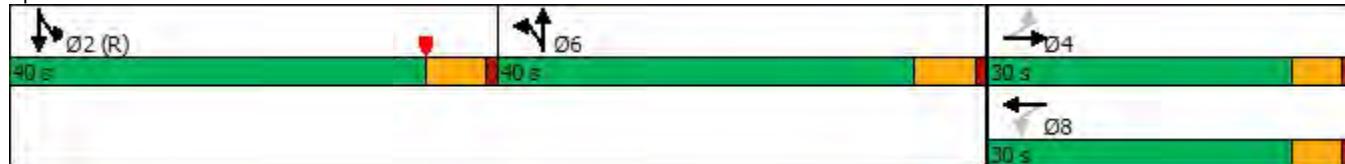
ICU Level of Service D

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 10: Route 303 & Mountainview Avenue



2686-99-013T

No Build - PM

10: Route 303 & Mountainview Avenue

	→	→	←	←	←	↑	↑	↓	↓	←		
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	53	38	91	175	47	50	164	722	94	33	858	38
Future Volume (vph)	53	38	91	175	47	50	164	722	94	33	858	38
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	10	10	10	10	10	10
Grade (%)	-3%				-2%			0%			-1%	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Frt		0.932			0.975			0.986			0.994	
Flt Protected		0.986			0.969			0.992			0.998	
Satd. Flow (prot)	0	1737	0	0	1752	0	0	3065	0	0	3154	0
Flt Permitted		0.849			0.604			0.992			0.998	
Satd. Flow (perm)	0	1496	0	0	1092	0	0	3065	0	0	3154	0
Right Turn on Red			Yes			Yes			No			Yes
Satd. Flow (RTOR)		42			10						4	
Link Speed (mph)		30			30			40			40	
Link Distance (ft)		498			709			1377			1086	
Travel Time (s)		11.3			16.1			23.5			18.5	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	0%	0%	4%	2%	0%	12%	3%	9%	4%	50%	5%	3%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	196	0	0	293	0	0	1053	0	0	999	0
Turn Type	Perm	NA		Perm	NA		Split	NA		Split	NA	
Protected Phases		4			8		6	6		2	2	
Permitted Phases	4			8								
Detector Phase	4	4		8	8		6	6		2	2	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		34.0	34.0		5.0	5.0	
Minimum Split (s)	10.0	10.0		10.0	10.0		40.0	40.0		11.0	11.0	
Total Split (s)	30.0	30.0		30.0	30.0		40.0	40.0		40.0	40.0	
Total Split (%)	27.3%	27.3%		27.3%	27.3%		36.4%	36.4%		36.4%	36.4%	
Yellow Time (s)	4.0	4.0		4.0	4.0		5.0	5.0		5.0	5.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)		0.0			0.0			0.0			0.0	
Total Lost Time (s)		5.0			5.0			6.0			6.0	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None		None	None		None	None		C-Max	C-Max	
Act Effct Green (s)		25.0			25.0			34.0			34.0	
Actuated g/C Ratio		0.23			0.23			0.31			0.31	
v/c Ratio		0.53			1.15			1.11			1.02	
Control Delay		34.9			140.9			94.4			72.6	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		34.9			140.9			94.4			72.6	
LOS	C			F			F			E		
Approach Delay		34.9			140.9			94.4			72.6	
Approach LOS		C			F			F			E	
Queue Length 50th (ft)		95			~238			~457			~394	
Queue Length 95th (ft)		171			#412			m#585			#526	
Internal Link Dist (ft)		418			629			1297			1006	



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)												
Base Capacity (vph)	372			255			947			977		
Starvation Cap Reductn	0			0			0			0		
Spillback Cap Reductn	0			0			0			0		
Storage Cap Reductn	0			0			0			0		
Reduced v/c Ratio	0.53			1.15			1.11			1.02		

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 5 (5%), Referenced to phase 2:SBTL, Start of Yellow

Natural Cycle: 130

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.15

Intersection Signal Delay: 86.6

Intersection LOS: F

Intersection Capacity Utilization 98.3%

ICU Level of Service F

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

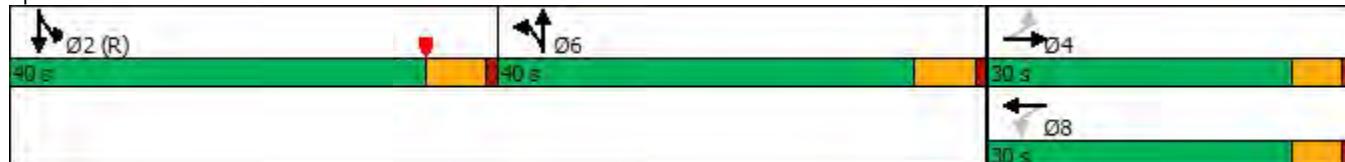
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

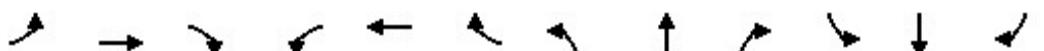
Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 10: Route 303 & Mountainview Avenue



	→	→	←	←	↑	↑	↓	↓	↗	↙	↖	↙
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	64	56	108	85	54	40	137	608	93	39	575	65
Future Volume (vph)	64	56	108	85	54	40	137	608	93	39	575	65
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	10	10	10	10	10	10
Grade (%)	-3%				-2%			0%			-1%	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Frt		0.936			0.970			0.983			0.986	
Flt Protected		0.986			0.977			0.992			0.997	
Satd. Flow (prot)	0	1729	0	0	1521	0	0	2959	0	0	2908	0
Flt Permitted		0.821			0.599			0.992			0.997	
Satd. Flow (perm)	0	1439	0	0	933	0	0	2959	0	0	2908	0
Right Turn on Red			Yes			Yes			No			Yes
Satd. Flow (RTOR)		38			12						11	
Link Speed (mph)		30			30			40			40	
Link Distance (ft)		498			709			1377			1086	
Travel Time (s)		11.3			16.1			23.5			18.5	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	2%	4%	3%	11%	0%	64%	7%	13%	4%	50%	13%	6%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	245	0	0	192	0	0	901	0	0	730	0
Turn Type	Perm	NA		Perm	NA		Split	NA		Split	NA	
Protected Phases		4			8		6	6		2	2	
Permitted Phases	4			8								
Detector Phase	4	4		8	8		6	6		2	2	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		34.0	34.0		5.0	5.0	
Minimum Split (s)	10.0	10.0		10.0	10.0		40.0	40.0		11.0	11.0	
Total Split (s)	30.0	30.0		30.0	30.0		40.0	40.0		40.0	40.0	
Total Split (%)	27.3%	27.3%		27.3%	27.3%		36.4%	36.4%		36.4%	36.4%	
Yellow Time (s)	4.0	4.0		4.0	4.0		5.0	5.0		5.0	5.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)		0.0			0.0			0.0			0.0	
Total Lost Time (s)		5.0			5.0			6.0			6.0	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None		None	None		None	None		C-Max	C-Max	
Act Effct Green (s)		22.9			22.9			34.0			36.1	
Actuated g/C Ratio		0.21			0.21			0.31			0.33	
v/c Ratio		0.74			0.94			0.99			0.76	
Control Delay		48.4			90.6			64.2			39.3	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		48.4			90.6			64.2			39.3	
LOS	D			F			E			D		
Approach Delay		48.4			90.6			64.2			39.3	
Approach LOS		D			F			E			D	
Queue Length 50th (ft)		135			124			353			245	
Queue Length 95th (ft)		227			#259			#480			321	
Internal Link Dist (ft)		418			629			1297			1006	



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)												
Base Capacity (vph)	356			221			914			961		
Starvation Cap Reductn	0			0			0			0		
Spillback Cap Reductn	0			0			0			0		
Storage Cap Reductn	0			0			0			0		
Reduced v/c Ratio	0.69			0.87			0.99			0.76		

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 5 (5%), Referenced to phase 2:SBTL, Start of Yellow

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.99

Intersection Signal Delay: 56.0

Intersection LOS: E

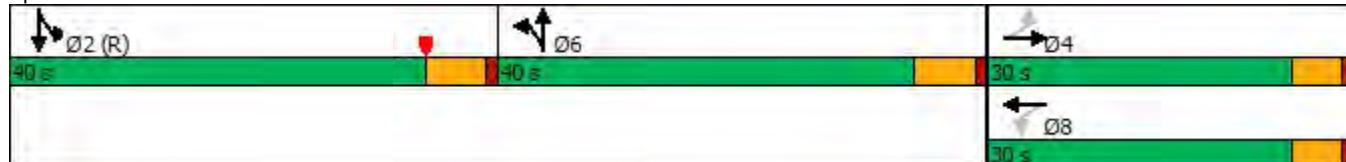
Intersection Capacity Utilization 78.5%

ICU Level of Service D

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 10: Route 303 & Mountainview Avenue

	→	→	←	←	←	↑	↑	↓	↓	↗	↙	
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	66	41	94	175	48	50	170	722	94	33	871	38
Future Volume (vph)	66	41	94	175	48	50	170	722	94	33	871	38
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	10	10	10	10	10	10
Grade (%)	-3%				-2%			0%			-1%	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Frt		0.937			0.975			0.986			0.994	
Flt Protected		0.984			0.969			0.991			0.998	
Satd. Flow (prot)	0	1729	0	0	1752	0	0	3063	0	0	3155	0
Flt Permitted		0.825			0.595			0.991			0.998	
Satd. Flow (perm)	0	1449	0	0	1076	0	0	3063	0	0	3155	0
Right Turn on Red			Yes			Yes			No			Yes
Satd. Flow (RTOR)		37			10						4	
Link Speed (mph)		30			30			40			40	
Link Distance (ft)		498			709			1377			1086	
Travel Time (s)		11.3			16.1			23.5			18.5	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	3%	0%	4%	2%	0%	12%	3%	9%	4%	50%	5%	3%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	216	0	0	294	0	0	1060	0	0	1013	0
Turn Type	Perm	NA		Perm	NA		Split	NA		Split	NA	
Protected Phases		4			8		6	6		2	2	
Permitted Phases	4			8								
Detector Phase	4	4		8	8		6	6		2	2	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		34.0	34.0		5.0	5.0	
Minimum Split (s)	10.0	10.0		10.0	10.0		40.0	40.0		11.0	11.0	
Total Split (s)	30.0	30.0		30.0	30.0		40.0	40.0		40.0	40.0	
Total Split (%)	27.3%	27.3%		27.3%	27.3%		36.4%	36.4%		36.4%	36.4%	
Yellow Time (s)	4.0	4.0		4.0	4.0		5.0	5.0		5.0	5.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)		0.0			0.0			0.0			0.0	
Total Lost Time (s)		5.0			5.0			6.0			6.0	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None		None	None		None	None		C-Max	C-Max	
Act Effct Green (s)		25.0			25.0			34.0			34.0	
Actuated g/C Ratio		0.23			0.23			0.31			0.31	
v/c Ratio		0.61			1.17			1.12			1.04	
Control Delay		39.4			147.1			97.6			76.4	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		39.4			147.1			97.6			76.4	
LOS	D			F			F			E		
Approach Delay		39.4			147.1			97.6			76.4	
Approach LOS		D		F			F			E		
Queue Length 50th (ft)		114			~242			~463			~405	
Queue Length 95th (ft)		196			#417			m#590			#538	
Internal Link Dist (ft)		418			629			1297			1006	



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)												
Base Capacity (vph)		357			252			946			977	
Starvation Cap Reductn		0			0			0			0	
Spillback Cap Reductn		0			0			0			0	
Storage Cap Reductn		0			0			0			0	
Reduced v/c Ratio		0.61			1.17			1.12			1.04	

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 5 (5%), Referenced to phase 2:SBTL, Start of Yellow

Natural Cycle: 140

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.17

Intersection Signal Delay: 90.1

Intersection LOS: F

Intersection Capacity Utilization 96.9%

ICU Level of Service F

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 10: Route 303 & Mountainview Avenue



2686-99-013T

Build w Mitigation - AM
10: Route 303 & Mountainview Avenue

	→	→	→	←	←	↑	↑	↓	↓	←	→	
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1	2	3	4	5	6	7	8	9	10	11	12
Traffic Volume (vph)	64	56	108	85	54	40	137	608	93	39	575	65
Future Volume (vph)	64	56	108	85	54	40	137	608	93	39	575	65
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	10	10	10	10	10	10
Grade (%)	-3%				-2%			0%			-1%	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Frt		0.901			0.970			0.983			0.986	
Flt Protected	0.950				0.977			0.992			0.997	
Satd. Flow (prot)	1796	1681	0	0	1521	0	0	2959	0	0	2908	0
Flt Permitted	0.574				0.612			0.992			0.997	
Satd. Flow (perm)	1085	1681	0	0	953	0	0	2959	0	0	2908	0
Right Turn on Red			Yes			Yes			No			Yes
Satd. Flow (RTOR)	82				12						11	
Link Speed (mph)	30				30			40			40	
Link Distance (ft)	498				709			1377			1086	
Travel Time (s)	11.3				16.1			23.5			18.5	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	2%	4%	3%	11%	0%	64%	7%	13%	4%	50%	13%	6%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	69	176	0	0	192	0	0	901	0	0	730	0
Turn Type	Perm	NA		Perm	NA		Split	NA		Split	NA	
Protected Phases		4			8		6	6		2	2	
Permitted Phases	4			8								
Detector Phase	4	4		8	8		6	6		2	2	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		34.0	34.0		5.0	5.0	
Minimum Split (s)	10.0	10.0		10.0	10.0		40.0	40.0		11.0	11.0	
Total Split (s)	30.0	30.0		30.0	30.0		40.0	40.0		40.0	40.0	
Total Split (%)	27.3%	27.3%		27.3%	27.3%		36.4%	36.4%		36.4%	36.4%	
Yellow Time (s)	4.0	4.0		4.0	4.0		5.0	5.0		5.0	5.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)	0.0	0.0			0.0			0.0			0.0	
Total Lost Time (s)	5.0	5.0			5.0			6.0			6.0	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None		None	None		None	None		C-Max	C-Max	
Act Effct Green (s)	22.6	22.6		22.6			34.0			36.4		
Actuated g/C Ratio	0.21	0.21		0.21			0.31			0.33		
v/c Ratio	0.31	0.43		0.94			0.99			0.75		
Control Delay	40.1	22.8		89.9			64.2			38.8		
Queue Delay	0.0	0.0		0.0			0.0			0.0		
Total Delay	40.1	22.8		89.9			64.2			38.8		
LOS	D	C		F			E			D		
Approach Delay		27.6			89.9		64.2			38.8		
Approach LOS		C		F			E			D		
Queue Length 50th (ft)	40	55		123			353			245		
Queue Length 95th (ft)	82	120		#256			#480			321		
Internal Link Dist (ft)		418		629			1297			1006		



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)												
Base Capacity (vph)	246	445			225			914			970	
Starvation Cap Reductn	0	0			0			0			0	
Spillback Cap Reductn	0	0			0			0			0	
Storage Cap Reductn	0	0			0			0			0	
Reduced v/c Ratio	0.28	0.40			0.85			0.99			0.75	

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 5 (5%), Referenced to phase 2:SBTL, Start of Yellow

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.99

Intersection Signal Delay: 53.3

Intersection LOS: D

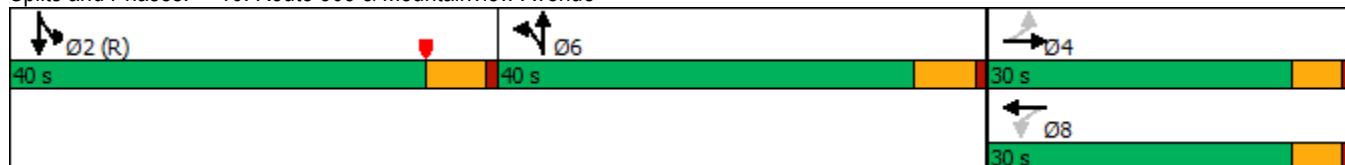
Intersection Capacity Utilization 85.3%

ICU Level of Service E

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 10: Route 303 & Mountainview Avenue

2686-99-013T

Build w Mitigation - PM
10: Route 303 & Mountainview Avenue

	↗	→	↘	↶	←	↷	↶	↑	↗	↘	↓	↶
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑			↔			↔		↔	↔	
Traffic Volume (vph)	66	41	94	175	48	50	170	722	94	33	871	38
Future Volume (vph)	66	41	94	175	48	50	170	722	94	33	871	38
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	10	10	10	10	10	10
Grade (%)	-3%				-2%			0%			-1%	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Frt		0.896			0.975			0.986			0.994	
Flt Protected		0.950			0.969			0.991			0.998	
Satd. Flow (prot)	1779	1681	0	0	1752	0	0	3063	0	0	3155	0
Flt Permitted		0.616			0.651			0.991			0.998	
Satd. Flow (perm)	1153	1681	0	0	1177	0	0	3063	0	0	3155	0
Right Turn on Red			Yes			Yes			No			Yes
Satd. Flow (RTOR)		97			10						4	
Link Speed (mph)		30			30			40			40	
Link Distance (ft)		498			709			1377			1086	
Travel Time (s)		11.3			16.1			23.5			18.5	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	3%	0%	4%	2%	0%	12%	3%	9%	4%	50%	5%	3%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	71	145	0	0	294	0	0	1060	0	0	1013	0
Turn Type	Perm	NA		Perm	NA		Split	NA		Split	NA	
Protected Phases		4			8		6	6		2	2	
Permitted Phases		4			8							
Detector Phase	4	4		8	8		6	6		2	2	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		34.0	34.0		5.0	5.0	
Minimum Split (s)	10.0	10.0		10.0	10.0		40.0	40.0		11.0	11.0	
Total Split (s)	30.0	30.0		30.0	30.0		40.0	40.0		40.0	40.0	
Total Split (%)	27.3%	27.3%		27.3%	27.3%		36.4%	36.4%		36.4%	36.4%	
Yellow Time (s)	4.0	4.0		4.0	4.0		5.0	5.0		5.0	5.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)	0.0	0.0			0.0			0.0			0.0	
Total Lost Time (s)	5.0	5.0			5.0			6.0			6.0	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None		None	None		None	None		C-Max	C-Max	
Act Effct Green (s)	25.0	25.0			25.0			34.0			34.0	
Actuated g/C Ratio	0.23	0.23			0.23			0.31			0.31	
v/c Ratio	0.27	0.32			1.07			1.12			1.04	
Control Delay	38.4	15.4			114.1			97.6			76.4	
Queue Delay	0.0	0.0			0.0			0.0			0.0	
Total Delay	38.4	15.4			114.1			97.6			76.4	
LOS	D	B			F			F			E	
Approach Delay		22.9			114.1			97.6			76.4	
Approach LOS		C			F			F			E	
Queue Length 50th (ft)	41	27			~225			~463			~405	
Queue Length 95th (ft)	84	81			#399			m#590			#538	
Internal Link Dist (ft)		418			629			1297			1006	



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)												
Base Capacity (vph)	262	457			275			946			977	
Starvation Cap Reductn	0	0			0			0			0	
Spillback Cap Reductn	0	0			0			0			0	
Storage Cap Reductn	0	0			0			0			0	
Reduced v/c Ratio	0.27	0.32			1.07			1.12			1.04	

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 5 (5%), Referenced to phase 2:SBTL, Start of Yellow

Natural Cycle: 120

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.12

Intersection Signal Delay: 84.9

Intersection LOS: F

Intersection Capacity Utilization 96.1%

ICU Level of Service F

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

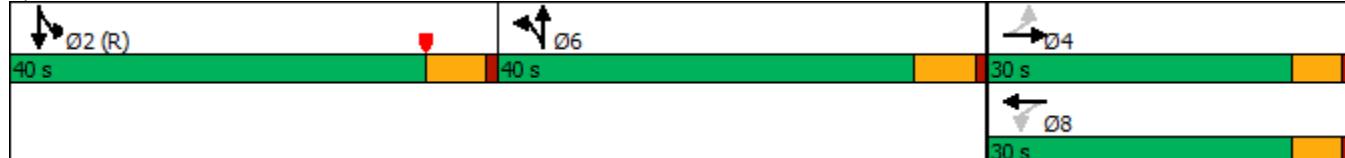
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 10: Route 303 & Mountainview Avenue



2686-99-013T

Design Horizon Year No Build - AM

10: Route 303 & Mountainview Avenue

	→	→	→	←	←	↑	↑	↓	↓	↗	↙	
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	74	67	131	104	63	49	146	742	113	48	697	79
Future Volume (vph)	74	67	131	104	63	49	146	742	113	48	697	79
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	10	10	10	10	10	10
Grade (%)	-3%				-2%			0%			-1%	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Frt		0.935			0.969			0.983			0.986	
Flt Protected		0.987			0.977			0.993			0.997	
Satd. Flow (prot)	0	1728	0	0	1516	0	0	2960	0	0	2907	0
Flt Permitted		0.811			0.566			0.993			0.997	
Satd. Flow (perm)	0	1420	0	0	878	0	0	2960	0	0	2907	0
Right Turn on Red			Yes			Yes			No			Yes
Satd. Flow (RTOR)		39			12						11	
Link Speed (mph)		30			30			40			40	
Link Distance (ft)		498			709			1377			1086	
Travel Time (s)		11.3			16.1			23.5			18.5	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	2%	4%	3%	11%	0%	64%	7%	13%	4%	50%	13%	6%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	293	0	0	233	0	0	1077	0	0	886	0
Turn Type	Perm	NA		Perm	NA		Split	NA		Split	NA	
Protected Phases		4			8		6	6		2	2	
Permitted Phases	4			8								
Detector Phase	4	4		8	8		6	6		2	2	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		34.0	34.0		5.0	5.0	
Minimum Split (s)	10.0	10.0		10.0	10.0		40.0	40.0		11.0	11.0	
Total Split (s)	30.0	30.0		30.0	30.0		40.0	40.0		40.0	40.0	
Total Split (%)	27.3%	27.3%		27.3%	27.3%		36.4%	36.4%		36.4%	36.4%	
Yellow Time (s)	4.0	4.0		4.0	4.0		5.0	5.0		5.0	5.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)		0.0			0.0			0.0			0.0	
Total Lost Time (s)		5.0			5.0			6.0			6.0	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None		None	None		None	None		C-Max	C-Max	
Act Effct Green (s)		25.0			25.0			34.0			34.0	
Actuated g/C Ratio		0.23			0.23			0.31			0.31	
v/c Ratio		0.83			1.12			1.18			0.98	
Control Delay		56.1			136.9			121.6			62.8	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		56.1			136.9			121.6			62.8	
LOS	E			F			F			E		
Approach Delay		56.1			136.9			121.6			62.8	
Approach LOS		E			F			F			E	
Queue Length 50th (ft)		174			~183			~489			322	
Queue Length 95th (ft)		#321			#342			m#590			#461	
Internal Link Dist (ft)		418			629			1297			1006	



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)												
Base Capacity (vph)	352			208			914			906		
Starvation Cap Reductn	0			0			0			0		
Spillback Cap Reductn	0			0			0			0		
Storage Cap Reductn	0			0			0			0		
Reduced v/c Ratio	0.83			1.12			1.18			0.98		

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 5 (5%), Referenced to phase 2:SBTL, Start of Yellow

Natural Cycle: 130

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.18

Intersection Signal Delay: 94.4

Intersection LOS: F

Intersection Capacity Utilization 86.7%

ICU Level of Service E

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

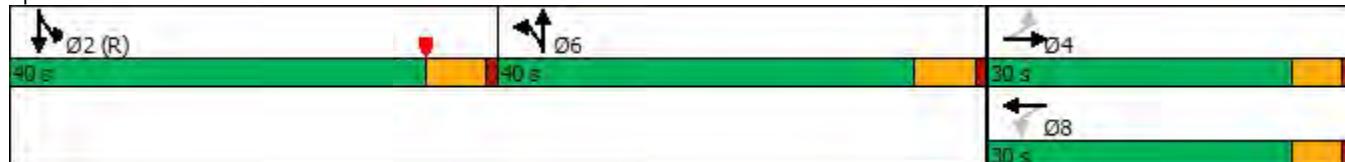
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 10: Route 303 & Mountainview Avenue



2686-99-013T

Design Horizon Year No Build - PM

10: Route 303 & Mountainview Avenue

	→	→	→	←	←	↑	↑	↓	↓	↗	↙	
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	65	46	111	215	58	61	200	882	115	40	1049	46
Future Volume (vph)	65	46	111	215	58	61	200	882	115	40	1049	46
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	10	10	10	10	10	10
Grade (%)	-3%				-2%			0%			-1%	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Frt		0.932			0.975			0.986			0.994	
Flt Protected		0.986			0.969			0.992			0.998	
Satd. Flow (prot)	0	1737	0	0	1752	0	0	3065	0	0	3154	0
Flt Permitted		0.844			0.559			0.992			0.998	
Satd. Flow (perm)	0	1487	0	0	1011	0	0	3065	0	0	3154	0
Right Turn on Red			Yes			Yes			No			Yes
Satd. Flow (RTOR)		42			10						4	
Link Speed (mph)		30			30			40			40	
Link Distance (ft)		498			709			1377			1086	
Travel Time (s)		11.3			16.1			23.5			18.5	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	0%	0%	4%	2%	0%	12%	3%	9%	4%	50%	5%	3%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	238	0	0	359	0	0	1287	0	0	1220	0
Turn Type	Perm	NA		Perm	NA		Split	NA		Split	NA	
Protected Phases		4			8		6	6		2	2	
Permitted Phases	4			8								
Detector Phase	4	4		8	8		6	6		2	2	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		34.0	34.0		5.0	5.0	
Minimum Split (s)	10.0	10.0		10.0	10.0		40.0	40.0		11.0	11.0	
Total Split (s)	30.0	30.0		30.0	30.0		40.0	40.0		40.0	40.0	
Total Split (%)	27.3%	27.3%		27.3%	27.3%		36.4%	36.4%		36.4%	36.4%	
Yellow Time (s)	4.0	4.0		4.0	4.0		5.0	5.0		5.0	5.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)		0.0			0.0			0.0			0.0	
Total Lost Time (s)		5.0			5.0			6.0			6.0	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None		None	None		None	None		C-Max	C-Max	
Act Effct Green (s)		25.0			25.0			34.0			34.0	
Actuated g/C Ratio		0.23			0.23			0.31			0.31	
v/c Ratio		0.64			1.51			1.36			1.25	
Control Delay		40.5			283.4			192.9			154.3	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		40.5			283.4			192.9			154.3	
LOS	D			F			F			F		
Approach Delay		40.5			283.4			192.9			154.3	
Approach LOS		D		F			F			F		
Queue Length 50th (ft)		126			~351			~645			~569	
Queue Length 95th (ft)		214			#537			m#607			#706	
Internal Link Dist (ft)		418			629			1297			1006	



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)												
Base Capacity (vph)	370			237			947			977		
Starvation Cap Reductn	0			0			0			0		
Spillback Cap Reductn	0			0			0			0		
Storage Cap Reductn	0			0			0			0		
Reduced v/c Ratio	0.64			1.51			1.36			1.25		

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 5 (5%), Referenced to phase 2:SBTL, Start of Yellow

Natural Cycle: 150

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.51

Intersection Signal Delay: 176.5

Intersection LOS: F

Intersection Capacity Utilization 115.3%

ICU Level of Service H

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

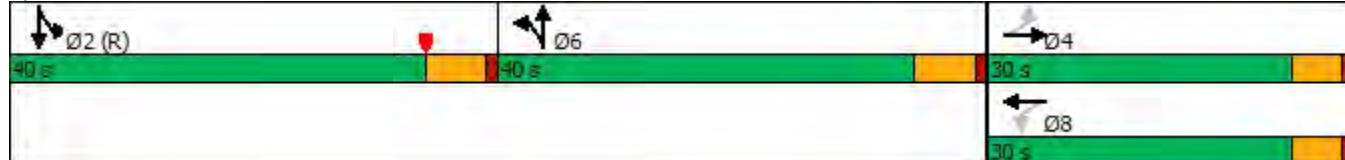
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 10: Route 303 & Mountainview Avenue



2686-99-013T

Design Horizon Year Build - AM

10: Route 303 & Mountainview Avenue

	→	→	←	←	←	↑	↑	↓	↓	↗	↙	
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	77	68	132	104	66	49	163	742	113	48	701	79
Future Volume (vph)	77	68	132	104	66	49	163	742	113	48	701	79
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	10	10	10	10	10	10
Grade (%)	-3%				-2%			0%			-1%	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Frt		0.936			0.970			0.983			0.986	
Flt Protected		0.986			0.977			0.992			0.997	
Satd. Flow (prot)	0	1729	0	0	1521	0	0	2959	0	0	2907	0
Flt Permitted		0.803			0.568			0.992			0.997	
Satd. Flow (perm)	0	1408	0	0	884	0	0	2959	0	0	2907	0
Right Turn on Red			Yes			Yes			No			Yes
Satd. Flow (RTOR)		39			12						10	
Link Speed (mph)		30			30			40			40	
Link Distance (ft)		498			709			1377			1086	
Travel Time (s)		11.3			16.1			23.5			18.5	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	2%	4%	3%	11%	0%	64%	7%	13%	4%	50%	13%	6%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	298	0	0	236	0	0	1095	0	0	891	0
Turn Type	Perm	NA		Perm	NA		Split	NA		Split	NA	
Protected Phases		4			8		6	6		2	2	
Permitted Phases	4			8								
Detector Phase	4	4		8	8		6	6		2	2	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		34.0	34.0		5.0	5.0	
Minimum Split (s)	10.0	10.0		10.0	10.0		40.0	40.0		11.0	11.0	
Total Split (s)	30.0	30.0		30.0	30.0		40.0	40.0		40.0	40.0	
Total Split (%)	27.3%	27.3%		27.3%	27.3%		36.4%	36.4%		36.4%	36.4%	
Yellow Time (s)	4.0	4.0		4.0	4.0		5.0	5.0		5.0	5.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)		0.0			0.0			0.0			0.0	
Total Lost Time (s)		5.0			5.0			6.0			6.0	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None		None	None		None	None		C-Max	C-Max	
Act Effct Green (s)		25.0			25.0			34.0			34.0	
Actuated g/C Ratio		0.23			0.23			0.31			0.31	
v/c Ratio		0.85			1.12			1.20			0.98	
Control Delay		58.5			137.8			129.4			64.4	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		58.5			137.8			129.4			64.4	
LOS	E			F			F			E		
Approach Delay		58.5			137.8			129.4			64.4	
Approach LOS		E			F			F			E	
Queue Length 50th (ft)		178			~186			~504			325	
Queue Length 95th (ft)		#331			#347			m#595			#466	
Internal Link Dist (ft)		418			629			1297			1006	



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)												
Base Capacity (vph)	350			210			914			905		
Starvation Cap Reductn	0			0			0			0		
Spillback Cap Reductn	0			0			0			0		
Storage Cap Reductn	0			0			0			0		
Reduced v/c Ratio	0.85			1.12			1.20			0.98		

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 5 (5%), Referenced to phase 2:SBTL, Start of Yellow

Natural Cycle: 140

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.20

Intersection Signal Delay: 98.8

Intersection LOS: F

Intersection Capacity Utilization 87.1%

ICU Level of Service E

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

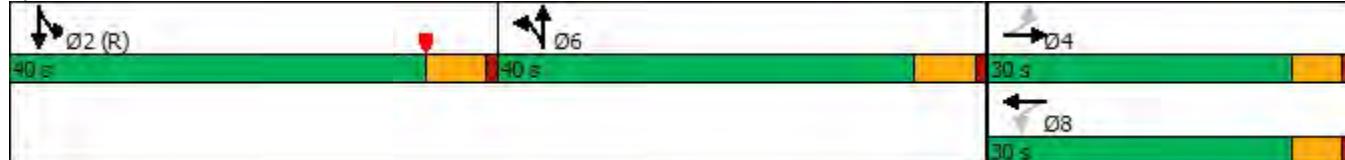
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 10: Route 303 & Mountainview Avenue



	→	→	→	←	←	↑	↑	↓	↓	↗	↙	
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	78	49	114	215	59	61	206	882	115	40	1062	46
Future Volume (vph)	78	49	114	215	59	61	206	882	115	40	1062	46
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	10	10	10	10	10	10
Grade (%)	-3%				-2%			0%			-1%	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Frt		0.936			0.975			0.986			0.994	
Flt Protected		0.984			0.969			0.991			0.998	
Satd. Flow (prot)	0	1727	0	0	1752	0	0	3063	0	0	3154	0
Flt Permitted		0.825			0.551			0.991			0.998	
Satd. Flow (perm)	0	1448	0	0	996	0	0	3063	0	0	3154	0
Right Turn on Red			Yes			Yes			No			Yes
Satd. Flow (RTOR)		38			10						4	
Link Speed (mph)		30			30			40			40	
Link Distance (ft)		498			709			1377			1086	
Travel Time (s)		11.3			16.1			23.5			18.5	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	3%	0%	4%	2%	0%	12%	3%	9%	4%	50%	5%	3%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	260	0	0	360	0	0	1294	0	0	1234	0
Turn Type	Perm	NA		Perm	NA		Split	NA		Split	NA	
Protected Phases		4			8		6	6		2	2	
Permitted Phases	4			8								
Detector Phase	4	4		8	8		6	6		2	2	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		34.0	34.0		5.0	5.0	
Minimum Split (s)	10.0	10.0		10.0	10.0		40.0	40.0		11.0	11.0	
Total Split (s)	30.0	30.0		30.0	30.0		40.0	40.0		40.0	40.0	
Total Split (%)	27.3%	27.3%		27.3%	27.3%		36.4%	36.4%		36.4%	36.4%	
Yellow Time (s)	4.0	4.0		4.0	4.0		5.0	5.0		5.0	5.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)		0.0			0.0			0.0			0.0	
Total Lost Time (s)		5.0			5.0			6.0			6.0	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None		None	None		None	None		C-Max	C-Max	
Act Effct Green (s)		25.0			25.0			34.0			34.0	
Actuated g/C Ratio		0.23			0.23			0.31			0.31	
v/c Ratio		0.73			1.54			1.37			1.26	
Control Delay		46.4			293.4			196.7			160.2	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		46.4			293.4			196.7			160.2	
LOS	D			F			F			F		
Approach Delay		46.4			293.4			196.7			160.2	
Approach LOS		D		F			F			F		
Queue Length 50th (ft)		147			~355			~652			~580	
Queue Length 95th (ft)		#260			#543			m#609			#718	
Internal Link Dist (ft)		418			629			1297			1006	



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)												
Base Capacity (vph)	358			234			946			977		
Starvation Cap Reductn	0			0			0			0		
Spillback Cap Reductn	0			0			0			0		
Storage Cap Reductn	0			0			0			0		
Reduced v/c Ratio	0.73			1.54			1.37			1.26		

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 5 (5%), Referenced to phase 2:SBTL, Start of Yellow

Natural Cycle: 150

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.54

Intersection Signal Delay: 181.1

Intersection LOS: F

Intersection Capacity Utilization 115.2%

ICU Level of Service H

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

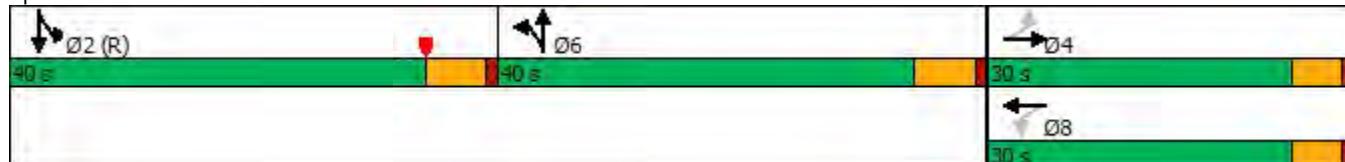
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 10: Route 303 & Mountainview Avenue



2686-99-013T

Design Horizon Year Build w Mitigation - AM

10: Route 303 & Mountainview Avenue

	↗	→	↘	↙	←	↖	↑	↗	↘	↓	↙	
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑			↔			↔		↔	↔	
Traffic Volume (vph)	77	68	132	104	66	49	163	742	113	48	701	79
Future Volume (vph)	77	68	132	104	66	49	163	742	113	48	701	79
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	10	10	10	10	10	10
Grade (%)	-3%				-2%			0%			-1%	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Frt		0.901			0.970			0.983			0.986	
Flt Protected	0.950				0.977			0.992			0.997	
Satd. Flow (prot)	1796	1681	0	0	1521	0	0	2959	0	0	2907	0
Flt Permitted	0.551				0.550			0.992			0.997	
Satd. Flow (perm)	1042	1681	0	0	856	0	0	2959	0	0	2907	0
Right Turn on Red			Yes			Yes			No			Yes
Satd. Flow (RTOR)	82				12						10	
Link Speed (mph)	30				30			40			40	
Link Distance (ft)	498				709			1377			1086	
Travel Time (s)	11.3				16.1			23.5			18.5	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	2%	4%	3%	11%	0%	64%	7%	13%	4%	50%	13%	6%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	83	215	0	0	236	0	0	1095	0	0	891	0
Turn Type	Perm	NA		Perm	NA		Split	NA		Split	NA	
Protected Phases		4			8		6	6		2	2	
Permitted Phases	4			8								
Detector Phase	4	4		8	8		6	6		2	2	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		34.0	34.0		5.0	5.0	
Minimum Split (s)	10.0	10.0		10.0	10.0		40.0	40.0		11.0	11.0	
Total Split (s)	30.0	30.0		30.0	30.0		40.0	40.0		40.0	40.0	
Total Split (%)	27.3%	27.3%		27.3%	27.3%		36.4%	36.4%		36.4%	36.4%	
Yellow Time (s)	4.0	4.0		4.0	4.0		5.0	5.0		5.0	5.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)	0.0	0.0			0.0			0.0			0.0	
Total Lost Time (s)	5.0	5.0			5.0			6.0			6.0	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None		None	None		None	None		C-Max	C-Max	
Act Effct Green (s)	25.0	25.0			25.0			34.0			34.0	
Actuated g/C Ratio	0.23	0.23			0.23			0.31			0.31	
v/c Ratio	0.35	0.48			1.16			1.20			0.98	
Control Delay	40.8	26.5			151.3			129.4			64.4	
Queue Delay	0.0	0.0			0.0			0.0			0.0	
Total Delay	40.8	26.5			151.3			129.4			64.4	
LOS	D	C			F			F			E	
Approach Delay		30.5			151.3			129.4			64.4	
Approach LOS		C			F			F			E	
Queue Length 50th (ft)	50	80			~191			~504			325	
Queue Length 95th (ft)	98	156			#352			m#595			#466	
Internal Link Dist (ft)		418			629			1297			1006	



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)												
Base Capacity (vph)	236	445			203			914			905	
Starvation Cap Reductn	0	0			0			0			0	
Spillback Cap Reductn	0	0			0			0			0	
Storage Cap Reductn	0	0			0			0			0	
Reduced v/c Ratio	0.35	0.48			1.16			1.20			0.98	

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 5 (5%), Referenced to phase 2:SBTL, Start of Yellow

Natural Cycle: 120

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.20

Intersection Signal Delay: 96.8

Intersection LOS: F

Intersection Capacity Utilization 94.4%

ICU Level of Service F

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

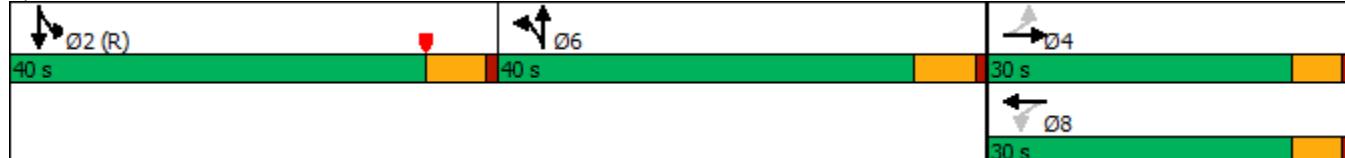
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 10: Route 303 & Mountainview Avenue

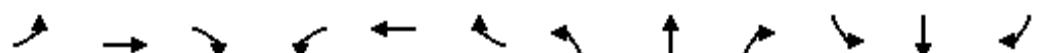


2686-99-013T

Design Horizon Year Build w Mitigation - PM

10: Route 303 & Mountainview Avenue

	→	→	→	←	←	↑	↑	↓	↓	↙	↙	
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1	2	3	4	5	6	7	8	9	10	11	12
Traffic Volume (vph)	78	49	114	215	59	61	206	882	115	40	1062	46
Future Volume (vph)	78	49	114	215	59	61	206	882	115	40	1062	46
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	10	10	10	10	10	10
Grade (%)	-3%				-2%			0%			-1%	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Frt		0.895			0.975			0.986			0.994	
Flt Protected		0.950			0.969			0.991			0.998	
Satd. Flow (prot)	1779	1679	0	0	1752	0	0	3063	0	0	3154	0
Flt Permitted		0.616			0.585			0.991			0.998	
Satd. Flow (perm)	1153	1679	0	0	1058	0	0	3063	0	0	3154	0
Right Turn on Red			Yes			Yes			No			Yes
Satd. Flow (RTOR)		98			10						4	
Link Speed (mph)		30			30			40			40	
Link Distance (ft)		498			709			1377			1086	
Travel Time (s)		11.3			16.1			23.5			18.5	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	3%	0%	4%	2%	0%	12%	3%	9%	4%	50%	5%	3%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	84	176	0	0	360	0	0	1294	0	0	1234	0
Turn Type	Perm	NA		Perm	NA		Split	NA		Split	NA	
Protected Phases		4			8		6	6		2	2	
Permitted Phases		4			8							
Detector Phase	4	4		8	8		6	6		2	2	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		34.0	34.0		5.0	5.0	
Minimum Split (s)	10.0	10.0		10.0	10.0		40.0	40.0		11.0	11.0	
Total Split (s)	30.0	30.0		30.0	30.0		40.0	40.0		40.0	40.0	
Total Split (%)	27.3%	27.3%		27.3%	27.3%		36.4%	36.4%		36.4%	36.4%	
Yellow Time (s)	4.0	4.0		4.0	4.0		5.0	5.0		5.0	5.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)	0.0	0.0			0.0			0.0			0.0	
Total Lost Time (s)	5.0	5.0			5.0			6.0			6.0	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None		None	None		None	None		C-Max	C-Max	
Act Effct Green (s)	25.0	25.0		25.0			34.0			34.0		
Actuated g/C Ratio	0.23	0.23		0.23			0.31			0.31		
v/c Ratio	0.32	0.39		1.45			1.37			1.26		
Control Delay	39.5	19.0		256.6			196.7			160.2		
Queue Delay	0.0	0.0		0.0			0.0			0.0		
Total Delay	39.5	19.0		256.6			196.7			160.2		
LOS	D	B		F			F			F		
Approach Delay		25.7		256.6			196.7			160.2		
Approach LOS		C		F			F			F		
Queue Length 50th (ft)	50	45		~345			~652			~580		
Queue Length 95th (ft)	97	108		#532			m#609			#718		
Internal Link Dist (ft)		418		629			1297			1006		



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)												
Base Capacity (vph)	262	457			248			946			977	
Starvation Cap Reductn	0	0			0			0			0	
Spillback Cap Reductn	0	0			0			0			0	
Storage Cap Reductn	0	0			0			0			0	
Reduced v/c Ratio	0.32	0.39			1.45			1.37			1.26	

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 5 (5%), Referenced to phase 2:SBTL, Start of Yellow

Natural Cycle: 140

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.45

Intersection Signal Delay: 175.1

Intersection LOS: F

Intersection Capacity Utilization 112.7%

ICU Level of Service H

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

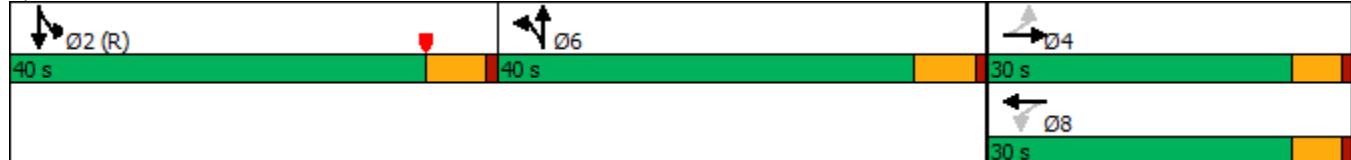
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 10: Route 303 & Mountainview Avenue



2686-99-013T

Existing - AM

20: Route 303 & Orangeburg Road (CR 20)/Driveway

	→	→	←	←	↑	↑	↑	↑	↓	↓	←	
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	193	13	246	13	9	8	236	508	2	10	490	198
Future Volume (vph)	193	13	246	13	9	8	236	508	2	10	490	198
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	12	12	12	11	11	11	11	11	11
Grade (%)	-3%				0%			-4%			-1%	
Storage Length (ft)	685			0	0		0	0		0	0	0
Storage Lanes	0			1	0		1	0		0	0	0
Taper Length (ft)	110				25			25			25	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Ped Bike Factor				0.99		1.00			1.00			1.00
Fr _t				0.850			0.850					0.957
Flt Protected				0.955			0.972			0.984		0.999
Satd. Flow (prot)	0	1686	1481	0	1847	1615	0	3182	0	0	2995	0
Flt Permitted				0.955			0.701			0.575		0.941
Satd. Flow (perm)	0	1686	1462	0	1331	1615	0	1859	0	0	2821	0
Right Turn on Red				Yes			Yes			Yes		Yes
Satd. Flow (RTOR)				262			119					65
Link Speed (mph)				30			25			40		40
Link Distance (ft)				1228			609			621		1377
Travel Time (s)				27.9			16.6			10.6		23.5
Confl. Peds. (#/hr)				1		1				4		4
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Heavy Vehicles (%)	6%	0%	7%	0%	0%	0%	6%	12%	0%	38%	11%	13%
Adj. Flow (vph)	205	14	262	14	10	9	251	540	2	11	521	211
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	219	262	0	24	9	0	793	0	0	743	0
Turn Type	Split	NA	pm+ov	Perm	NA	Perm	custom	NA		Perm	NA	
Protected Phases	4	4	6		8			6	16			5
Permitted Phases				4	8		8	1			5	
Detector Phase	4	4	4	8	8	8	6	16		5	5	
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0			45.0	45.0	
Minimum Split (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0			50.0	50.0	
Total Split (s)	30.0	30.0	15.0	15.0	15.0	15.0	15.0			50.0	50.0	
Total Split (%)	27.3%	27.3%	13.6%	13.6%	13.6%	13.6%	13.6%			45.5%	45.5%	
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0			4.0	4.0	
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0			1.0	1.0	
Lost Time Adjust (s)				0.0	0.0	0.0	0.0				0.0	
Total Lost Time (s)				5.0	5.0	5.0	5.0				5.0	
Lead/Lag				Lag			Lag			Lead	Lead	
Lead-Lag Optimize?				Yes			Yes			Yes	Yes	
Recall Mode	None			C-Max	C-Max							
Act Effct Green (s)		18.6	28.6		7.0	7.0		73.5			58.5	
Actuated g/C Ratio		0.17	0.26		0.06	0.06		0.67			0.53	
v/c Ratio		0.77	0.46		0.29	0.04		0.58			0.49	
Control Delay		60.7	5.8		56.9	0.4		15.5			21.9	
Queue Delay		0.0	0.0		0.0	0.0		0.0			0.0	
Total Delay		60.7	5.8		56.9	0.4		15.5			21.9	

Lane Group	Ø1
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Width (ft)	
Grade (%)	
Storage Length (ft)	
Storage Lanes	
Taper Length (ft)	
Lane Util. Factor	
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Confl. Peds. (#/hr)	
Peak Hour Factor	
Heavy Vehicles (%)	
Adj. Flow (vph)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Turn Type	
Protected Phases	1
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	45.0
Minimum Split (s)	50.0
Total Split (s)	65.0
Total Split (%)	59%
Yellow Time (s)	4.0
All-Red Time (s)	1.0
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Recall Mode	Max
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	

2686-99-013T

Existing - AM

20: Route 303 & Orangeburg Road (CR 20)/Driveway



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
LOS	E	A		E	A		B			C		
Approach Delay		30.8			41.5			15.5			21.9	
Approach LOS		C			D			B			C	
Queue Length 50th (ft)	149	0		17	0		89			104		
Queue Length 95th (ft)	218	53		43	0		137			219		
Internal Link Dist (ft)	1148			529			541			1297		
Turn Bay Length (ft)												
Base Capacity (vph)	383	645		121	255		1362			1530		
Starvation Cap Reductn	0	0		0	0		0			0		
Spillback Cap Reductn	0	0		0	0		0			0		
Storage Cap Reductn	0	0		0	0		0			0		
Reduced v/c Ratio	0.57	0.41		0.20	0.04		0.58			0.49		

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 0 (0%), Referenced to phase 5:SBTL, Start of Yellow

Natural Cycle: 80

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.77

Intersection Signal Delay: 21.8

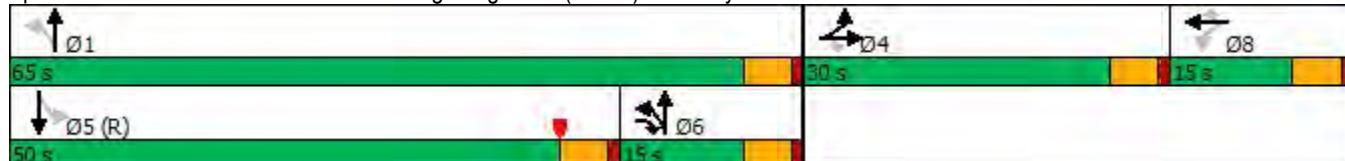
Intersection LOS: C

Intersection Capacity Utilization 105.5%

ICU Level of Service G

Analysis Period (min) 15

Splits and Phases: 20: Route 303 & Orangeburg Road (CR 20)/Driveway



Lane Group	Ø1
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (ft)	
Queue Length 95th (ft)	
Internal Link Dist (ft)	
Turn Bay Length (ft)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

2686-99-013T

Existing - PM

20: Route 303 & Orangeburg Road (CR 20)/Driveway

	→	→	←	←	↑	↑	↑	↑	↓	↓	←	
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	259	16	205	23	40	12	250	650	8	4	706	297
Future Volume (vph)	259	16	205	23	40	12	250	650	8	4	706	297
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	12	12	12	11	11	11	11	11	11
Grade (%)	-3%				0%			-4%			-1%	
Storage Length (ft)	685			0	0		0	0		0	0	0
Storage Lanes	0			1	0		1	0		0	0	0
Taper Length (ft)	110				25			25			25	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Ped Bike Factor	1.00					0.99						
Fr _t			0.850			0.850		0.999			0.956	
Flt Protected		0.955				0.982		0.986				
Satd. Flow (prot)	0	1613	1538	0	1866	1615	0	3344	0	0	3230	0
Flt Permitted		0.955			0.761			0.507			0.952	
Satd. Flow (perm)	0	1610	1538	0	1446	1592	0	1719	0	0	3075	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		216				119		1			71	
Link Speed (mph)		30			25			40			40	
Link Distance (ft)		1228			609			621			1377	
Travel Time (s)		27.9			16.6			10.6			23.5	
Confl. Peds. (#/hr)	2				2							
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	11%	0%	3%	0%	0%	0%	2%	6%	0%	0%	5%	1%
Adj. Flow (vph)	273	17	216	24	42	13	263	684	8	4	743	313
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	290	216	0	66	13	0	955	0	0	1060	0
Turn Type	Split	NA	pm+ov	Perm	NA	Perm	custom	NA		Perm	NA	
Protected Phases	4	4	6		8			6	16		5	
Permitted Phases		4	8			8	1			5		
Detector Phase	4	4	4	8	8	8	6	16		5	5	
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0			45.0	45.0	
Minimum Split (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0			50.0	50.0	
Total Split (s)	30.0	30.0	15.0	15.0	15.0	15.0	15.0			50.0	50.0	
Total Split (%)	27.3%	27.3%	13.6%	13.6%	13.6%	13.6%	13.6%			45.5%	45.5%	
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0			4.0	4.0	
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0			1.0	1.0	
Lost Time Adjust (s)		0.0	0.0		0.0	0.0					0.0	
Total Lost Time (s)		5.0	5.0		5.0	5.0					5.0	
Lead/Lag			Lag				Lag			Lead	Lead	
Lead-Lag Optimize?		Yes				Yes				Yes	Yes	
Recall Mode	None			C-Max	C-Max							
Act Effct Green (s)	22.6	37.6		8.6	8.6			65.9			50.9	
Actuated g/C Ratio	0.21	0.34		0.08	0.08			0.60			0.46	
v/c Ratio	0.88	0.32		0.59	0.06			0.81			0.73	
Control Delay	68.5	4.6		69.7	0.4			27.7			36.2	
Queue Delay	0.0	0.0		0.0	0.0			0.0			0.0	
Total Delay	68.5	4.6		69.7	0.4			27.7			36.2	

Lane Group	Ø1
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Width (ft)	
Grade (%)	
Storage Length (ft)	
Storage Lanes	
Taper Length (ft)	
Lane Util. Factor	
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Confl. Peds. (#/hr)	
Peak Hour Factor	
Heavy Vehicles (%)	
Adj. Flow (vph)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Turn Type	
Protected Phases	1
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	45.0
Minimum Split (s)	50.0
Total Split (s)	65.0
Total Split (%)	59%
Yellow Time (s)	4.0
All-Red Time (s)	1.0
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Recall Mode	Max
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	

2686-99-013T

Existing - PM

20: Route 303 & Orangeburg Road (CR 20)/Driveway



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
LOS	E	A		E	A		C			D		
Approach Delay	41.3			58.3			27.7			36.2		
Approach LOS	D			E			C			D		
Queue Length 50th (ft)	194	0		45	0		151			256		
Queue Length 95th (ft)	#327	49		92	0		m#205			323		
Internal Link Dist (ft)	1148			529			541			1297		
Turn Bay Length (ft)												
Base Capacity (vph)	366	696		131	252		1177			1459		
Starvation Cap Reductn	0	0		0	0		0			0		
Spillback Cap Reductn	0	0		0	0		0			0		
Storage Cap Reductn	0	0		0	0		0			0		
Reduced v/c Ratio	0.79	0.31		0.50	0.05		0.81			0.73		

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 0 (0%), Referenced to phase 5:SBTL, Start of Yellow

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.88

Intersection Signal Delay: 34.7

Intersection LOS: C

Intersection Capacity Utilization 109.4%

ICU Level of Service H

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 20: Route 303 & Orangeburg Road (CR 20)/Driveway



Lane Group	Ø1
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (ft)	
Queue Length 95th (ft)	
Internal Link Dist (ft)	
Turn Bay Length (ft)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

2686-99-013T

No Build - AM

20: Route 303 & Orangeburg Road (CR 20)/Driveway

	→	→	←	←	↑	↑	↑	↑	↓	↓	←	
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	211	13	253	13	9	8	243	602	2	10	547	206
Future Volume (vph)	211	13	253	13	9	8	243	602	2	10	547	206
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	12	12	12	11	11	11	11	11	11
Grade (%)	-3%				0%			-4%			-1%	
Storage Length (ft)	685		0	0		0	0		0	0		0
Storage Lanes	0		1	0		1	0		0	0		0
Taper Length (ft)	110			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Ped Bike Factor			0.99			1.00			1.00			1.00
Fr _t			0.850			0.850						0.960
Flt Protected		0.955				0.972			0.986			0.999
Satd. Flow (prot)	0	1685	1481	0	1847	1615	0	3183	0	0	3006	0
Flt Permitted		0.955				0.691			0.564			0.939
Satd. Flow (perm)	0	1685	1462	0	1312	1615	0	1821	0	0	2825	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		269				119						58
Link Speed (mph)		30			25			40			40	
Link Distance (ft)		1228			609			621			1377	
Travel Time (s)		27.9			16.6			10.6			23.5	
Confl. Peds. (#/hr)			1	1					4	4		
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Heavy Vehicles (%)	6%	0%	7%	0%	0%	0%	6%	12%	0%	38%	11%	13%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	238	269	0	24	9	0	901	0	0	812	0
Turn Type	Split	NA	pm+ov	Perm	NA	Perm	custom	NA		Perm	NA	
Protected Phases	4	4	6		8		6	16			5	
Permitted Phases			4	8		8	1				5	
Detector Phase	4	4	4	8	8	8	6	16		5	5	
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0			45.0	45.0	
Minimum Split (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0			50.0	50.0	
Total Split (s)	30.0	30.0	15.0	15.0	15.0	15.0	15.0			50.0	50.0	
Total Split (%)	27.3%	27.3%	13.6%	13.6%	13.6%	13.6%	13.6%	13.6%		45.5%	45.5%	
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0			4.0	4.0	
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0			1.0	1.0	
Lost Time Adjust (s)		0.0	0.0		0.0	0.0					0.0	
Total Lost Time (s)		5.0	5.0		5.0	5.0					5.0	
Lead/Lag			Lag			Lag			Lead	Lead		
Lead-Lag Optimize?			Yes			Yes			Yes	Yes		
Recall Mode	None		C-Max	C-Max								
Act Effct Green (s)		19.5	29.5		7.0	7.0		72.6			57.6	
Actuated g/C Ratio		0.18	0.27		0.06	0.06		0.66			0.52	
v/c Ratio		0.80	0.46		0.29	0.04		0.68			0.54	
Control Delay		62.2	5.6		57.1	0.4		17.7			29.2	
Queue Delay		0.0	0.0		0.0	0.0		0.0			0.0	
Total Delay		62.2	5.6		57.1	0.4		17.7			29.2	
LOS		E	A		E	A		B			C	

Lane Group	Ø1
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Width (ft)	
Grade (%)	
Storage Length (ft)	
Storage Lanes	
Taper Length (ft)	
Lane Util. Factor	
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Confl. Peds. (#/hr)	
Peak Hour Factor	
Heavy Vehicles (%)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Turn Type	
Protected Phases	1
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	45.0
Minimum Split (s)	50.0
Total Split (s)	65.0
Total Split (%)	59%
Yellow Time (s)	4.0
All-Red Time (s)	1.0
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Recall Mode	Max
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	

2686-99-013T

No Build - AM

20: Route 303 & Orangeburg Road (CR 20)/Driveway



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Approach Delay		32.2			41.7			17.7			29.2	
Approach LOS		C			D			B			C	
Queue Length 50th (ft)	162	0		17	0		88			191		
Queue Length 95th (ft)	238	54		43	0		162			m240		
Internal Link Dist (ft)	1148			529			541			1297		
Turn Bay Length (ft)												
Base Capacity (vph)	382	650		119	255		1325			1506		
Starvation Cap Reductn	0	0		0	0		0			0		
Spillback Cap Reductn	0	0		0	0		0			0		
Storage Cap Reductn	0	0		0	0		0			0		
Reduced v/c Ratio	0.62	0.41		0.20	0.04		0.68			0.54		

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 0 (0%), Referenced to phase 5:SBTL, Start of Yellow

Natural Cycle: 80

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.80

Intersection Signal Delay: 25.5

Intersection LOS: C

Intersection Capacity Utilization 106.5%

ICU Level of Service G

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 20: Route 303 & Orangeburg Road (CR 20)/Driveway



Lane Group	Ø1
Approach Delay	
Approach LOS	
Queue Length 50th (ft)	
Queue Length 95th (ft)	
Internal Link Dist (ft)	
Turn Bay Length (ft)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

2686-99-013T

No Build - PM

20: Route 303 & Orangeburg Road (CR 20)/Driveway

	→	→	←	←	↑	↑	↑	↑	↓	↓	←	
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	268	16	213	23	41	12	260	700	8	4	806	314
Future Volume (vph)	268	16	213	23	41	12	260	700	8	4	806	314
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	12	12	12	11	11	11	11	11	11
Grade (%)	-3%				0%			-4%			-1%	
Storage Length (ft)	685		0	0		0	0		0	0		0
Storage Lanes	0		1	0		1	0		0	0		0
Taper Length (ft)	110			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Ped Bike Factor	1.00					0.99						
Fr _t		0.850				0.850		0.999			0.958	
Flt Protected		0.955				0.982		0.987				
Satd. Flow (prot)	0	1613	1538	0	1866	1615	0	3346	0	0	3235	0
Flt Permitted		0.955			0.760		0.505				0.952	
Satd. Flow (perm)	0	1610	1538	0	1444	1592	0	1712	0	0	3080	0
Right Turn on Red		Yes				Yes			Yes			Yes
Satd. Flow (RTOR)		198				119		1			63	
Link Speed (mph)		30			25			40			40	
Link Distance (ft)		1228			609			621			1377	
Travel Time (s)		27.9			16.6			10.6			23.5	
Confl. Peds. (#/hr)	2				2							
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	11%	0%	3%	0%	0%	0%	2%	6%	0%	0%	5%	1%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	299	224	0	67	13	0	1019	0	0	1183	0
Turn Type	Split	NA	pm+ov	Perm	NA	Perm	custom	NA		Perm	NA	
Protected Phases	4	4	6		8		6	16			5	
Permitted Phases			4	8		8	1				5	
Detector Phase	4	4	4	8	8	8	6	16		5	5	
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0			45.0	45.0	
Minimum Split (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0			50.0	50.0	
Total Split (s)	30.0	30.0	15.0	15.0	15.0	15.0	15.0			50.0	50.0	
Total Split (%)	27.3%	27.3%	13.6%	13.6%	13.6%	13.6%	13.6%	13.6%		45.5%	45.5%	
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0			4.0	4.0	
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0			1.0	1.0	
Lost Time Adjust (s)		0.0	0.0		0.0	0.0					0.0	
Total Lost Time (s)		5.0	5.0		5.0	5.0					5.0	
Lead/Lag			Lag				Lag			Lead	Lead	
Lead-Lag Optimize?			Yes				Yes			Yes	Yes	
Recall Mode	None			C-Max	C-Max							
Act Effct Green (s)	22.9	37.9		8.6	8.6			65.5			50.5	
Actuated g/C Ratio	0.21	0.34		0.08	0.08			0.60			0.46	
v/c Ratio	0.89	0.34		0.60	0.06			1.01dl			0.82	
Control Delay	70.6	6.4		70.1	0.4			30.4			40.4	
Queue Delay		0.0	0.0		0.0	0.0		0.0			0.0	
Total Delay	70.6	6.4		70.1	0.4			30.4			40.4	
LOS	E	A		E	A			C			D	

Lane Group	Ø1
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Width (ft)	
Grade (%)	
Storage Length (ft)	
Storage Lanes	
Taper Length (ft)	
Lane Util. Factor	
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Confl. Peds. (#/hr)	
Peak Hour Factor	
Heavy Vehicles (%)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Turn Type	
Protected Phases	1
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	45.0
Minimum Split (s)	50.0
Total Split (s)	65.0
Total Split (%)	59%
Yellow Time (s)	4.0
All-Red Time (s)	1.0
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Recall Mode	Max
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	

2686-99-013T

No Build - PM

20: Route 303 & Orangeburg Road (CR 20)/Driveway



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Approach Delay		43.1			58.8			30.4			40.4	
Approach LOS		D			E			C			D	
Queue Length 50th (ft)	202		12		46	0		171			302	
Queue Length 95th (ft)	#343		64		93	0		m#231			m293	
Internal Link Dist (ft)	1148				529			541			1297	
Turn Bay Length (ft)												
Base Capacity (vph)	366	685		131	252		1168			1449		
Starvation Cap Reductn	0	0		0	0		0			0		
Spillback Cap Reductn	0	0		0	0		0			0		
Storage Cap Reductn	0	0		0	0		0			0		
Reduced v/c Ratio	0.82	0.33		0.51	0.05		0.87			0.82		

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 0 (0%), Referenced to phase 5:SBTL, Start of Yellow

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.89

Intersection Signal Delay: 37.8

Intersection LOS: D

Intersection Capacity Utilization 109.9%

ICU Level of Service H

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

dl Defacto Left Lane. Recode with 1 though lane as a left lane.

Splits and Phases: 20: Route 303 & Orangeburg Road (CR 20)/Driveway



Lane Group	Ø1
Approach Delay	
Approach LOS	
Queue Length 50th (ft)	
Queue Length 95th (ft)	
Internal Link Dist (ft)	
Turn Bay Length (ft)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

	→	→	←	←	↑	↑	↑	↑	↓	↓	←	
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	218	13	253	13	9	8	243	612	2	10	550	208
Future Volume (vph)	218	13	253	13	9	8	243	612	2	10	550	208
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	12	12	12	11	11	11	11	11	11
Grade (%)	-3%				0%			-4%			-1%	
Storage Length (ft)	685		0	0		0	0		0	0		0
Storage Lanes	0		1	0		1	0		0	0		0
Taper Length (ft)	110			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Ped Bike Factor			0.99			1.00			1.00			1.00
Fr _t			0.850			0.850						0.959
Flt Protected			0.955			0.972			0.986			0.999
Satd. Flow (prot)	0	1685	1481	0	1847	1615	0	3183	0	0	3003	0
Flt Permitted			0.955			0.687			0.563			0.939
Satd. Flow (perm)	0	1685	1462	0	1305	1615	0	1817	0	0	2822	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			269			119						59
Link Speed (mph)			30			25			40			40
Link Distance (ft)			1228			609			621			1377
Travel Time (s)			27.9			16.6			10.6			23.5
Confl. Peds. (#/hr)			1		1				4		4	
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Heavy Vehicles (%)	6%	0%	7%	0%	0%	0%	6%	12%	0%	38%	11%	13%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	246	269	0	24	9	0	912	0	0	817	0
Turn Type	Split	NA	pm+ov	Perm	NA	Perm	custom	NA		Perm	NA	
Protected Phases	4	4	6		8		6	16			5	
Permitted Phases			4	8		8	1			5		
Detector Phase	4	4	4	8	8	8	6	16		5	5	
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0			45.0	45.0	
Minimum Split (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0			50.0	50.0	
Total Split (s)	30.0	30.0	15.0	15.0	15.0	15.0	15.0			50.0	50.0	
Total Split (%)	27.3%	27.3%	13.6%	13.6%	13.6%	13.6%	13.6%	13.6%		45.5%	45.5%	
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0			4.0	4.0	
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0			1.0	1.0	
Lost Time Adjust (s)			0.0	0.0		0.0	0.0				0.0	
Total Lost Time (s)			5.0	5.0		5.0	5.0				5.0	
Lead/Lag				Lag			Lag			Lead	Lead	
Lead-Lag Optimize?				Yes			Yes			Yes	Yes	
Recall Mode	None			C-Max	C-Max							
Act Effct Green (s)		19.9	29.9		7.0	7.0		72.2			57.2	
Actuated g/C Ratio		0.18	0.27		0.06	0.06		0.66			0.52	
v/c Ratio		0.81	0.45		0.29	0.04		0.69			0.55	
Control Delay		62.8	5.6		57.3	0.4		18.4			29.7	
Queue Delay		0.0	0.0		0.0	0.0		0.0			0.0	
Total Delay		62.8	5.6		57.3	0.4		18.4			29.7	
LOS		E	A		E	A		B			C	

Lane Group	Ø1
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Width (ft)	
Grade (%)	
Storage Length (ft)	
Storage Lanes	
Taper Length (ft)	
Lane Util. Factor	
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Confl. Peds. (#/hr)	
Peak Hour Factor	
Heavy Vehicles (%)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Turn Type	
Protected Phases	1
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	45.0
Minimum Split (s)	50.0
Total Split (s)	65.0
Total Split (%)	59%
Yellow Time (s)	4.0
All-Red Time (s)	1.0
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Recall Mode	Max
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Approach Delay		32.9			41.7			18.4			29.7	
Approach LOS		C			D			B			C	
Queue Length 50th (ft)	167	0		17	0		91			192		
Queue Length 95th (ft)	247	54		43	0		164			m241		
Internal Link Dist (ft)	1148			529			541			1297		
Turn Bay Length (ft)												
Base Capacity (vph)	382	650		118	255		1316			1495		
Starvation Cap Reductn	0	0		0	0		0			0		
Spillback Cap Reductn	0	0		0	0		0			0		
Storage Cap Reductn	0	0		0	0		0			0		
Reduced v/c Ratio	0.64	0.41		0.20	0.04		0.69			0.55		

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 0 (0%), Referenced to phase 5:SBTL, Start of Yellow

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.81

Intersection Signal Delay: 26.0

Intersection LOS: C

Intersection Capacity Utilization 106.9%

ICU Level of Service G

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 20: Route 303 & Orangeburg Road (CR 20)/Driveway



Lane Group	Ø1
Approach Delay	
Approach LOS	
Queue Length 50th (ft)	
Queue Length 95th (ft)	
Internal Link Dist (ft)	
Turn Bay Length (ft)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

	←	→	↖	↙	↔	↙	↖	↑	↗	↘	↓	↗
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	270	16	213	23	41	12	260	704	8	4	815	321
Future Volume (vph)	270	16	213	23	41	12	260	704	8	4	815	321
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	12	12	12	11	11	11	11	11	11
Grade (%)	-3%				0%			-4%			-1%	
Storage Length (ft)	685		0	0		0	0		0	0		0
Storage Lanes	0		1	0		1	0		0	0		0
Taper Length (ft)	110			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Ped Bike Factor		1.00				0.99						
Fr _t			0.850			0.850		0.999			0.958	
Flt Protected		0.955			0.982			0.987				
Satd. Flow (prot)	0	1613	1538	0	1866	1615	0	3346	0	0	3235	0
Flt Permitted		0.955			0.760			0.506			0.952	
Satd. Flow (perm)	0	1610	1538	0	1444	1592	0	1716	0	0	3080	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		196			119			1			64	
Link Speed (mph)		30			25			40			40	
Link Distance (ft)		1228			609			621			1377	
Travel Time (s)		27.9			16.6			10.6			23.5	
Confl. Peds. (#/hr)	2				2							
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	11%	0%	3%	0%	0%	0%	2%	6%	0%	0%	5%	1%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	301	224	0	67	13	0	1023	0	0	1200	0
Turn Type	Split	NA	pm+ov	Perm	NA	Perm	custom	NA		Perm	NA	
Protected Phases	4	4	6		8		6	16			5	
Permitted Phases			4	8		8	1			5		
Detector Phase	4	4	6	8	8	8	6	16		5	5	
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0			45.0	45.0	
Minimum Split (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0			50.0	50.0	
Total Split (s)	30.0	30.0	15.0	15.0	15.0	15.0	15.0			50.0	50.0	
Total Split (%)	27.3%	27.3%	13.6%	13.6%	13.6%	13.6%	13.6%	13.6%		45.5%	45.5%	
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0			4.0	4.0	
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0			1.0	1.0	
Lost Time Adjust (s)		0.0	0.0		0.0	0.0					0.0	
Total Lost Time (s)		5.0	5.0		5.0	5.0					5.0	
Lead/Lag			Lag			Lag			Lead	Lead		
Lead-Lag Optimize?			Yes			Yes			Yes	Yes		
Recall Mode	None		C-Max	C-Max								
Act Effct Green (s)	23.0	38.0		8.6	8.6		65.5			50.5		
Actuated g/C Ratio	0.21	0.35		0.08	0.08		0.60			0.46		
v/c Ratio	0.89	0.34		0.60	0.06		1.03dl			0.83		
Control Delay	70.9	6.6		70.1	0.4		30.6			40.6		
Queue Delay		0.0	0.0		0.0	0.0		0.0			0.0	
Total Delay	70.9	6.6		70.1	0.4		30.6			40.6		
LOS	E	A		E	A		C			D		

Lane Group	Ø1
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Width (ft)	
Grade (%)	
Storage Length (ft)	
Storage Lanes	
Taper Length (ft)	
Lane Util. Factor	
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Confl. Peds. (#/hr)	
Peak Hour Factor	
Heavy Vehicles (%)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Turn Type	
Protected Phases	1
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	45.0
Minimum Split (s)	50.0
Total Split (s)	65.0
Total Split (%)	59%
Yellow Time (s)	4.0
All-Red Time (s)	1.0
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Recall Mode	Max
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Approach Delay		43.5			58.8			30.6			40.6	
Approach LOS		D			E			C			D	
Queue Length 50th (ft)	203		13		46	0		172			306	
Queue Length 95th (ft)	#346		65		93	0		m#234			m294	
Internal Link Dist (ft)	1148				529			541			1297	
Turn Bay Length (ft)												
Base Capacity (vph)	366	659		131	252		1169			1447		
Starvation Cap Reductn	0	0		0	0		0			0		
Spillback Cap Reductn	0	0		0	0		0			0		
Storage Cap Reductn	0	0		0	0		0			0		
Reduced v/c Ratio	0.82	0.34		0.51	0.05		0.88			0.83		

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 0 (0%), Referenced to phase 5:SBTL, Start of Yellow

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.89

Intersection Signal Delay: 38.0

Intersection LOS: D

Intersection Capacity Utilization 110.0%

ICU Level of Service H

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

dl Defacto Left Lane. Recode with 1 though lane as a left lane.

Splits and Phases: 20: Route 303 & Orangeburg Road (CR 20)/Driveway



Lane Group	Ø1
Approach Delay	
Approach LOS	
Queue Length 50th (ft)	
Queue Length 95th (ft)	
Internal Link Dist (ft)	
Turn Bay Length (ft)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

2686-99-013T

Design Horizon Year No Build - AM

20: Route 303 & Orangeburg Road (CR 20)/Driveway

	→	→	←	←	↑	↑	↑	↑	↓	↓	←	
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	257	13	310	13	9	8	297	736	2	11	669	252
Future Volume (vph)	257	13	310	13	9	8	297	736	2	11	669	252
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	12	12	12	11	11	11	11	11	11
Grade (%)	-3%				0%			-4%			-1%	
Storage Length (ft)	685		0	0		0	0		0	0		0
Storage Lanes	0		1	0		1	0		0	0		0
Taper Length (ft)	110			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Ped Bike Factor			0.99			1.00			1.00			1.00
Fr _t			0.850			0.850						0.959
Flt Protected		0.955				0.972			0.986			0.999
Satd. Flow (prot)	0	1684	1481	0	1847	1615	0	3183	0	0	3004	0
Flt Permitted		0.955			0.667			0.522			0.936	
Satd. Flow (perm)	0	1684	1462	0	1267	1615	0	1685	0	0	2814	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		252			119						58	
Link Speed (mph)		30			25			40			40	
Link Distance (ft)		1228			609			621			1377	
Travel Time (s)		27.9			16.6			10.6			23.5	
Confl. Peds. (#/hr)			1	1					4	4		
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Heavy Vehicles (%)	6%	0%	7%	0%	0%	0%	6%	12%	0%	38%	11%	13%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	287	330	0	24	9	0	1101	0	0	992	0
Turn Type	Split	NA	pm+ov	Perm	NA	Perm	custom	NA		Perm	NA	
Protected Phases	4	4	6		8		6	16			5	
Permitted Phases			4	8		8	1			5		
Detector Phase	4	4	4	8	8	8	6	16		5	5	
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0			45.0	45.0	
Minimum Split (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0			50.0	50.0	
Total Split (s)	30.0	30.0	15.0	15.0	15.0	15.0	15.0			50.0	50.0	
Total Split (%)	27.3%	27.3%	13.6%	13.6%	13.6%	13.6%	13.6%	13.6%		45.5%	45.5%	
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0			4.0	4.0	
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0			1.0	1.0	
Lost Time Adjust (s)		0.0	0.0		0.0	0.0				0.0		
Total Lost Time (s)		5.0	5.0		5.0	5.0				5.0		
Lead/Lag			Lag			Lag			Lead	Lead		
Lead-Lag Optimize?			Yes			Yes			Yes	Yes		
Recall Mode	None		C-Max	C-Max								
Act Effct Green (s)	21.9	31.9		7.0	7.0			70.1			55.1	
Actuated g/C Ratio	0.20	0.29		0.06	0.06		0.64			0.50		
v/c Ratio	0.86	0.55		0.30	0.04		0.91			0.69		
Control Delay	65.8	10.7		57.8	0.4		29.8			35.4		
Queue Delay		0.0	0.0		0.0	0.0		0.0			0.0	
Total Delay	65.8	10.7		57.8	0.4		29.8			35.4		
LOS	E	B		E	A		C			D		

Lane Group	Ø1
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Width (ft)	
Grade (%)	
Storage Length (ft)	
Storage Lanes	
Taper Length (ft)	
Lane Util. Factor	
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Confl. Peds. (#/hr)	
Peak Hour Factor	
Heavy Vehicles (%)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Turn Type	
Protected Phases	1
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	45.0
Minimum Split (s)	50.0
Total Split (s)	65.0
Total Split (%)	59%
Yellow Time (s)	4.0
All-Red Time (s)	1.0
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Recall Mode	Max
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Approach Delay		36.3			42.1			29.8			35.4	
Approach LOS		D			D			C			D	
Queue Length 50th (ft)	193	37		17	0		192			245		
Queue Length 95th (ft)	#309	112		43	0		#381			m253		
Internal Link Dist (ft)	1148			529			541			1297		
Turn Bay Length (ft)												
Base Capacity (vph)	382	638		115	255		1210			1439		
Starvation Cap Reductn	0	0		0	0		0			0		
Spillback Cap Reductn	0	0		0	0		0			0		
Storage Cap Reductn	0	0		0	0		0			0		
Reduced v/c Ratio	0.75	0.52		0.21	0.04		0.91			0.69		

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 0 (0%), Referenced to phase 5:SBTL, Start of Yellow

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.91

Intersection Signal Delay: 33.4

Intersection LOS: C

Intersection Capacity Utilization 109.1%

ICU Level of Service H

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 20: Route 303 & Orangeburg Road (CR 20)/Driveway



Lane Group	Ø1
Approach Delay	
Approach LOS	
Queue Length 50th (ft)	
Queue Length 95th (ft)	
Internal Link Dist (ft)	
Turn Bay Length (ft)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

2686-99-013T

Design Horizon Year No Build - PM

20: Route 303 & Orangeburg Road (CR 20)/Driveway

	→	→	←	←	↑	↑	↑	↑	↓	↓	←	
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	328	16	262	24	41	13	318	856	8	5	986	384
Future Volume (vph)	328	16	262	24	41	13	318	856	8	5	986	384
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	12	12	12	11	11	11	11	11	11
Grade (%)	-3%				0%			-4%			-1%	
Storage Length (ft)	685		0	0		0	0		0	0		0
Storage Lanes	0		1	0		1	0		0	0		0
Taper Length (ft)	110			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Ped Bike Factor	1.00					0.99						
Fr _t		0.850				0.850		0.999			0.958	
Flt Protected		0.955				0.982		0.987				
Satd. Flow (prot)	0	1611	1538	0	1866	1615	0	3346	0	0	3235	0
Flt Permitted		0.955			0.731		0.523				0.950	
Satd. Flow (perm)	0	1608	1538	0	1389	1592	0	1773	0	0	3073	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		169				119		1			63	
Link Speed (mph)		30			25			40			40	
Link Distance (ft)		1228			609			621			1377	
Travel Time (s)		27.9			16.6			10.6			23.5	
Confl. Peds. (#/hr)	2				2							
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	11%	0%	3%	0%	0%	0%	2%	6%	0%	0%	5%	1%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	362	276	0	68	14	0	1244	0	0	1447	0
Turn Type	Split	NA	pm+ov	Perm	NA	Perm	custom	NA		Perm	NA	
Protected Phases	4	4	6		8		6	16			5	
Permitted Phases			4	8		8	1			5		
Detector Phase	4	4	6	8	8	8	6	16		5	5	
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0			45.0	45.0	
Minimum Split (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0			50.0	50.0	
Total Split (s)	30.0	30.0	15.0	15.0	15.0	15.0	15.0			50.0	50.0	
Total Split (%)	27.3%	27.3%	13.6%	13.6%	13.6%	13.6%	13.6%			45.5%	45.5%	
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0			4.0	4.0	
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0			1.0	1.0	
Lost Time Adjust (s)		0.0	0.0		0.0	0.0				0.0		
Total Lost Time (s)		5.0	5.0		5.0	5.0				5.0		
Lead/Lag			Lag			Lag			Lead	Lead		
Lead-Lag Optimize?			Yes			Yes			Yes	Yes		
Recall Mode	None		C-Max	C-Max								
Act Effct Green (s)		25.0	40.0		8.7	8.7		63.4			48.4	
Actuated g/C Ratio		0.23	0.36		0.08	0.08		0.58			0.44	
v/c Ratio		0.99	0.41		0.62	0.06		1.48dl			1.04	
Control Delay		87.8	12.0		72.7	0.5		65.2			64.3	
Queue Delay		0.0	0.0		0.0	0.0		0.0			0.0	
Total Delay		87.8	12.0		72.7	0.5		65.2			64.3	
LOS		F	B		E	A		E			E	

Lane Group	Ø1
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Width (ft)	
Grade (%)	
Storage Length (ft)	
Storage Lanes	
Taper Length (ft)	
Lane Util. Factor	
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Confl. Peds. (#/hr)	
Peak Hour Factor	
Heavy Vehicles (%)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Turn Type	
Protected Phases	1
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	45.0
Minimum Split (s)	50.0
Total Split (s)	65.0
Total Split (%)	59%
Yellow Time (s)	4.0
All-Red Time (s)	1.0
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Recall Mode	Max
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Approach Delay		55.0			60.3			65.2			64.3	
Approach LOS		E			E			E			E	
Queue Length 50th (ft)	256	51		47	0		~341			~434		
Queue Length 95th (ft)	#448	122		#100	0		m#364			m296		
Internal Link Dist (ft)	1148			529			541			1297		
Turn Bay Length (ft)												
Base Capacity (vph)	366	666		126	252		1165			1386		
Starvation Cap Reductn	0	0		0	0		0			0		
Spillback Cap Reductn	0	0		0	0		0			0		
Storage Cap Reductn	0	0		0	0		0			0		
Reduced v/c Ratio	0.99	0.41		0.54	0.06		1.07			1.04		

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 0 (0%), Referenced to phase 5:SBTL, Start of Yellow

Natural Cycle: 130

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.07

Intersection Signal Delay: 62.8

Intersection LOS: E

Intersection Capacity Utilization 115.4%

ICU Level of Service H

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

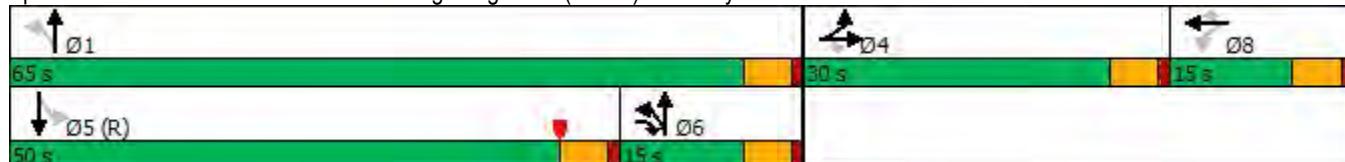
95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

dl Defacto Left Lane. Recode with 1 though lane as a left lane.

Splits and Phases: 20: Route 303 & Orangeburg Road (CR 20)/Driveway



Lane Group	Ø1
Approach Delay	
Approach LOS	
Queue Length 50th (ft)	
Queue Length 95th (ft)	
Internal Link Dist (ft)	
Turn Bay Length (ft)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

2686-99-013T

Design Horizon Year Build - AM
20: Route 303 & Orangeburg Road (CR 20)/Driveway

	→	→	←	←	↑	↑	↑	↑	↓	↓	←	
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	264	13	310	13	9	8	297	746	2	11	672	254
Future Volume (vph)	264	13	310	13	9	8	297	746	2	11	672	254
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	12	12	12	11	11	11	11	11	11
Grade (%)	-3%				0%			-4%			-1%	
Storage Length (ft)	685		0	0		0	0		0	0		0
Storage Lanes	0		1	0		1	0		0	0		0
Taper Length (ft)	110			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Ped Bike Factor			0.99			1.00			1.00			1.00
Fr _t			0.850			0.850						0.959
Flt Protected			0.955			0.972			0.986			0.999
Satd. Flow (prot)	0	1684	1481	0	1847	1615	0	3183	0	0	3004	0
Flt Permitted			0.955			0.663			0.521			0.936
Satd. Flow (perm)	0	1684	1462	0	1259	1615	0	1682	0	0	2814	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			251			119						59
Link Speed (mph)			30			25			40			40
Link Distance (ft)			1228			609			621			1377
Travel Time (s)			27.9			16.6			10.6			23.5
Confl. Peds. (#/hr)			1		1				4		4	
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Heavy Vehicles (%)	6%	0%	7%	0%	0%	0%	6%	12%	0%	38%	11%	13%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	295	330	0	24	9	0	1112	0	0	997	0
Turn Type	Split	NA	pm+ov	Perm	NA	Perm	custom	NA		Perm	NA	
Protected Phases	4	4	6		8		6	16			5	
Permitted Phases			4	8		8	1				5	
Detector Phase	4	4	4	8	8	8	6	16		5	5	
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0			45.0	45.0	
Minimum Split (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0			50.0	50.0	
Total Split (s)	30.0	30.0	15.0	15.0	15.0	15.0	15.0			50.0	50.0	
Total Split (%)	27.3%	27.3%	13.6%	13.6%	13.6%	13.6%	13.6%	13.6%		45.5%	45.5%	
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0			4.0	4.0	
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0			1.0	1.0	
Lost Time Adjust (s)			0.0	0.0		0.0	0.0				0.0	
Total Lost Time (s)			5.0	5.0		5.0	5.0				5.0	
Lead/Lag				Lag			Lag			Lead	Lead	
Lead-Lag Optimize?				Yes			Yes			Yes	Yes	
Recall Mode	None			C-Max	C-Max							
Act Effct Green (s)	22.3	32.3		7.0	7.0			69.8			54.8	
Actuated g/C Ratio	0.20	0.29		0.06	0.06		0.63				0.50	
v/c Ratio	0.87	0.54		0.30	0.04		0.92				0.70	
Control Delay	66.8	10.7		58.0	0.4		31.5				35.6	
Queue Delay		0.0	0.0		0.0	0.0		0.0			0.0	
Total Delay	66.8	10.7		58.0	0.4		31.5				35.6	
LOS	E	B		E	A		C			D		

Lane Group	Ø1
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Width (ft)	
Grade (%)	
Storage Length (ft)	
Storage Lanes	
Taper Length (ft)	
Lane Util. Factor	
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Confl. Peds. (#/hr)	
Peak Hour Factor	
Heavy Vehicles (%)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Turn Type	
Protected Phases	1
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	45.0
Minimum Split (s)	50.0
Total Split (s)	65.0
Total Split (%)	59%
Yellow Time (s)	4.0
All-Red Time (s)	1.0
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Recall Mode	Max
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Approach Delay		37.2			42.3			31.5			35.6	
Approach LOS		D			D			C			D	
Queue Length 50th (ft)	198	38		17	0		200			247		
Queue Length 95th (ft)	#324	113		43	0		#420			m253		
Internal Link Dist (ft)	1148			529			541			1297		
Turn Bay Length (ft)												
Base Capacity (vph)	382	638		114	255		1203			1431		
Starvation Cap Reductn	0	0		0	0		0			0		
Spillback Cap Reductn	0	0		0	0		0			0		
Storage Cap Reductn	0	0		0	0		0			0		
Reduced v/c Ratio	0.77	0.52		0.21	0.04		0.92			0.70		

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 0 (0%), Referenced to phase 5:SBTL, Start of Yellow

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.92

Intersection Signal Delay: 34.4

Intersection LOS: C

Intersection Capacity Utilization 109.5%

ICU Level of Service H

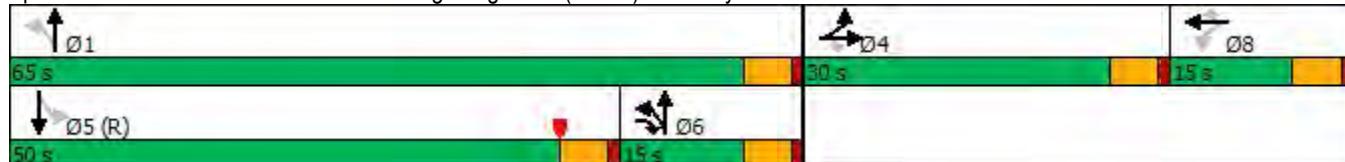
Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 20: Route 303 & Orangeburg Road (CR 20)/Driveway



Lane Group	Ø1
Approach Delay	
Approach LOS	
Queue Length 50th (ft)	
Queue Length 95th (ft)	
Internal Link Dist (ft)	
Turn Bay Length (ft)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

2686-99-013T

Design Horizon Year Build - PM
20: Route 303 & Orangeburg Road (CR 20)/Driveway

	←	→	↖	↗	↙	↖	↗	↑	↗	↖	↓	↗
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	330	16	262	24	41	13	318	860	8	5	995	391
Future Volume (vph)	330	16	262	24	41	13	318	860	8	5	995	391
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	12	12	12	11	11	11	11	11	11
Grade (%)	-3%				0%			-4%			-1%	
Storage Length (ft)	685		0	0		0	0		0	0		0
Storage Lanes	0		1	0		1	0		0	0		0
Taper Length (ft)	110			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Ped Bike Factor	1.00					0.99						
Fr _t		0.850				0.850		0.999			0.958	
Flt Protected		0.955				0.982		0.987				
Satd. Flow (prot)	0	1611	1538	0	1866	1615	0	3346	0	0	3235	0
Flt Permitted		0.955			0.730			0.524			0.950	
Satd. Flow (perm)	0	1608	1538	0	1387	1592	0	1776	0	0	3073	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		168				119		1			64	
Link Speed (mph)		30			25			40			40	
Link Distance (ft)		1228			609			621			1377	
Travel Time (s)		27.9			16.6			10.6			23.5	
Confl. Peds. (#/hr)	2				2							
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	11%	0%	3%	0%	0%	0%	2%	6%	0%	0%	5%	1%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	364	276	0	68	14	0	1248	0	0	1464	0
Turn Type	Split	NA	pm+ov	Perm	NA	Perm	custom	NA		Perm	NA	
Protected Phases	4	4	6		8		6	16			5	
Permitted Phases			4	8		8	1			5		
Detector Phase	4	4	4	8	8	8	6	16		5	5	
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0			45.0	45.0	
Minimum Split (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0			50.0	50.0	
Total Split (s)	30.0	30.0	15.0	15.0	15.0	15.0	15.0			50.0	50.0	
Total Split (%)	27.3%	27.3%	13.6%	13.6%	13.6%	13.6%	13.6%			45.5%	45.5%	
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0			4.0	4.0	
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0			1.0	1.0	
Lost Time Adjust (s)		0.0	0.0		0.0	0.0					0.0	
Total Lost Time (s)		5.0	5.0		5.0	5.0					5.0	
Lead/Lag			Lag				Lag			Lead	Lead	
Lead-Lag Optimize?			Yes				Yes			Yes	Yes	
Recall Mode	None			C-Max	C-Max							
Act Effct Green (s)		25.0	40.0		8.7	8.7		63.4			48.4	
Actuated g/C Ratio		0.23	0.36		0.08	0.08		0.58			0.44	
v/c Ratio		0.99	0.41		0.62	0.06		1.48dl			1.06	
Control Delay		89.2	12.1		72.7	0.5		66.0			68.8	
Queue Delay		0.0	0.0		0.0	0.0		0.0			0.0	
Total Delay		89.2	12.1		72.7	0.5		66.0			68.8	
LOS		F	B		E	A		E			E	

Lane Group	Ø1
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Width (ft)	
Grade (%)	
Storage Length (ft)	
Storage Lanes	
Taper Length (ft)	
Lane Util. Factor	
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Confl. Peds. (#/hr)	
Peak Hour Factor	
Heavy Vehicles (%)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Turn Type	
Protected Phases	1
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	45.0
Minimum Split (s)	50.0
Total Split (s)	65.0
Total Split (%)	59%
Yellow Time (s)	4.0
All-Red Time (s)	1.0
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Recall Mode	Max
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Approach Delay		55.9			60.3			66.0			68.8	
Approach LOS		E			E			E			E	
Queue Length 50th (ft)	258		52		47	0		~346			~444	
Queue Length 95th (ft)	#452		122		#100	0		m#369			m296	
Internal Link Dist (ft)	1148				529			541			1297	
Turn Bay Length (ft)												
Base Capacity (vph)	366	666		126	252		1166			1387		
Starvation Cap Reductn	0	0		0	0		0			0		
Spillback Cap Reductn	0	0		0	0		0			0		
Storage Cap Reductn	0	0		0	0		0			0		
Reduced v/c Ratio	0.99	0.41		0.54	0.06		1.07			1.06		

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 0 (0%), Referenced to phase 5:SBTL, Start of Yellow

Natural Cycle: 130

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.07

Intersection Signal Delay: 65.2

Intersection LOS: E

Intersection Capacity Utilization 115.9%

ICU Level of Service H

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

dl Defacto Left Lane. Recode with 1 though lane as a left lane.

Splits and Phases: 20: Route 303 & Orangeburg Road (CR 20)/Driveway



Lane Group	Ø1
Approach Delay	
Approach LOS	
Queue Length 50th (ft)	
Queue Length 95th (ft)	
Internal Link Dist (ft)	
Turn Bay Length (ft)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

2686-99-013T

Existing - AM

30: Route 303 & South Greenbush Road/Route 340

	↗	→	↘	↙	←	↖	↑	↗	↘	↓	↙	
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	25	28	7	58	30	207	6	514	63	233	496	20
Future Volume (vph)	25	28	7	58	30	207	6	514	63	233	496	20
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	16	16	16	12	12	12	11	11	11	11	11	11
Grade (%)	-2%				-3%			-3%			-1%	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Ped Bike Factor									1.00		1.00	
Frt		0.985				0.905			0.984			0.996
Flt Protected		0.979				0.990						0.985
Satd. Flow (prot)	0	1871	0	0	1617	0	0	3141	0	0	3139	0
Flt Permitted		0.620				0.927			0.948			0.602
Satd. Flow (perm)	0	1185	0	0	1514	0	0	2978	0	0	1918	0
Right Turn on Red			Yes				Yes			Yes		No
Satd. Flow (RTOR)		6				113			14			
Link Speed (mph)		30				40			40			40
Link Distance (ft)		280				324			945			621
Travel Time (s)		6.4				5.5			16.1			10.6
Confl. Peds. (#/hr)								1				1
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	21%	7%	0%	12%	3%	6%	17%	11%	10%	8%	10%	17%
Adj. Flow (vph)	26	29	7	61	32	218	6	541	66	245	522	21
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	62	0	0	311	0	0	613	0	0	788	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		custom	NA	
Protected Phases		4				8			2		1	1 6
Permitted Phases	4				8			2			6	
Detector Phase	4	4		8	8			2	2		1	1 6
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		45.0	45.0			5.0	
Minimum Split (s)	10.0	10.0		10.0	10.0		50.0	50.0			10.0	
Total Split (s)	40.0	40.0		40.0	40.0		50.0	50.0			20.0	
Total Split (%)	36.4%	36.4%		36.4%	36.4%		45.5%	45.5%			18.2%	
Yellow Time (s)	4.0	4.0		4.0	4.0		4.0	4.0			4.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0			1.0	
Lost Time Adjust (s)		0.0			0.0			0.0				
Total Lost Time (s)		5.0			5.0			5.0				
Lead/Lag							Lag	Lag		Lead		
Lead-Lag Optimize?							Yes	Yes		Yes		
Recall Mode	None	None		None	None		C-Min	C-Min		None		
Act Effect Green (s)	20.1			20.1				61.7			79.9	
Actuated g/C Ratio	0.18			0.18				0.56			0.73	
v/c Ratio	0.28			0.84				0.37			0.51	
Control Delay	35.4			46.7				15.5			9.1	
Queue Delay	0.0			0.0				0.0			0.0	
Total Delay	35.4			46.7				15.5			9.1	
LOS	D			D				B			A	
Approach Delay	35.4			46.7				15.5			9.1	
Approach LOS	D			D				B			A	

Lane Group	Ø6
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Width (ft)	
Grade (%)	
Lane Util. Factor	
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Confl. Peds. (#/hr)	
Peak Hour Factor	
Heavy Vehicles (%)	
Adj. Flow (vph)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Turn Type	
Protected Phases	6
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	45.0
Minimum Split (s)	50.0
Total Split (s)	70.0
Total Split (%)	64%
Yellow Time (s)	4.0
All-Red Time (s)	1.0
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Recall Mode	C-Min
Act Effect Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	

2686-99-013T

Existing - AM

30: Route 303 & South Greenbush Road/Route 340



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Queue Length 50th (ft)	34			139			121			105		
Queue Length 95th (ft)	66			220			197			107		
Internal Link Dist (ft)	200			244			865			541		
Turn Bay Length (ft)												
Base Capacity (vph)	381			558			1675			1559		
Starvation Cap Reductn	0			0			0			0		
Spillback Cap Reductn	0			0			0			0		
Storage Cap Reductn	0			0			0			0		
Reduced v/c Ratio	0.16			0.56			0.37			0.51		

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBTL, Start of Yellow

Natural Cycle: 75

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.84

Intersection Signal Delay: 18.8

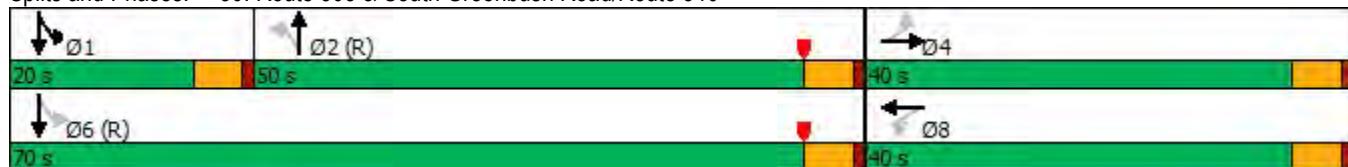
Intersection LOS: B

Intersection Capacity Utilization 90.1%

ICU Level of Service E

Analysis Period (min) 15

Splits and Phases: 30: Route 303 & South Greenbush Road/Route 340



Lane Group	Ø6
Queue Length 50th (ft)	
Queue Length 95th (ft)	
Internal Link Dist (ft)	
Turn Bay Length (ft)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

2686-99-013T

Existing - PM

30: Route 303 & South Greenbush Road/Route 340

	→	→	←	←	↑	↑	↓	↓	↗	↙	↖	↙
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	25	26	11	78	52	330	13	553	49	211	701	22
Future Volume (vph)	25	26	11	78	52	330	13	553	49	211	701	22
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	16	16	16	12	12	12	11	11	11	11	11	11
Grade (%)	-2%			-3%			-3%			-1%		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Frt		0.976			0.903			0.988			0.996	
Flt Protected		0.981			0.992			0.999			0.989	
Satd. Flow (prot)	0	2050	0	0	1654	0	0	3369	0	0	3310	0
Flt Permitted		0.681			0.928			0.923			0.589	
Satd. Flow (perm)	0	1423	0	0	1547	0	0	3113	0	0	1971	0
Right Turn on Red			Yes			Yes			Yes			No
Satd. Flow (RTOR)		10			122			10				
Link Speed (mph)		30			40			40			40	
Link Distance (ft)		280			324			945			621	
Travel Time (s)		6.4			5.5			16.1			10.6	
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Heavy Vehicles (%)	4%	0%	0%	1%	6%	5%	0%	4%	2%	4%	4%	20%
Adj. Flow (vph)	27	29	12	86	57	363	14	608	54	232	770	24
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	68	0	0	506	0	0	676	0	0	1026	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		custom	NA	
Protected Phases		4			8			2		1	16	
Permitted Phases	4			8			2			6		
Detector Phase	4	4		8	8		2	2		1	16	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		45.0	45.0		5.0		
Minimum Split (s)	10.0	10.0		10.0	10.0		50.0	50.0		10.0		
Total Split (s)	40.0	40.0		40.0	40.0		50.0	50.0		20.0		
Total Split (%)	36.4%	36.4%		36.4%	36.4%		45.5%	45.5%		18.2%		
Yellow Time (s)	4.0	4.0		4.0	4.0		4.0	4.0		4.0		
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0		
Lost Time Adjust (s)		0.0			0.0			0.0				
Total Lost Time (s)		5.0			5.0			5.0				
Lead/Lag							Lag	Lag		Lead		
Lead-Lag Optimize?							Yes	Yes		Yes		
Recall Mode	None	None		None	None		C-Min	C-Min		None		
Act Effct Green (s)		32.0			32.0			48.0			68.0	
Actuated g/C Ratio		0.29			0.29			0.44			0.62	
v/c Ratio		0.16			0.94			0.50			0.73	
Control Delay		24.5			56.0			24.2			18.3	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		24.5			56.0			24.2			18.3	
LOS		C			E			C			B	
Approach Delay		24.5			56.0			24.2			18.3	
Approach LOS		C			E			C			B	
Queue Length 50th (ft)		29			266			183			123	
Queue Length 95th (ft)		63			#466			240			201	

Lane Group	Ø6
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Width (ft)	
Grade (%)	
Lane Util. Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Peak Hour Factor	
Heavy Vehicles (%)	
Adj. Flow (vph)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Turn Type	
Protected Phases	6
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	45.0
Minimum Split (s)	50.0
Total Split (s)	70.0
Total Split (%)	64%
Yellow Time (s)	4.0
All-Red Time (s)	1.0
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Recall Mode	C-Min
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (ft)	
Queue Length 95th (ft)	

2686-99-013T

Existing - PM

30: Route 303 & South Greenbush Road/Route 340



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Internal Link Dist (ft)	200			244			865			541		
Turn Bay Length (ft)												
Base Capacity (vph)	459				575			1363			1400	
Starvation Cap Reductn	0				0			0			0	
Spillback Cap Reductn	0				0			0			0	
Storage Cap Reductn	0				0			0			0	
Reduced v/c Ratio	0.15				0.88			0.50			0.73	

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBTL, Start of Yellow

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.94

Intersection Signal Delay: 28.6

Intersection LOS: C

Intersection Capacity Utilization 105.5%

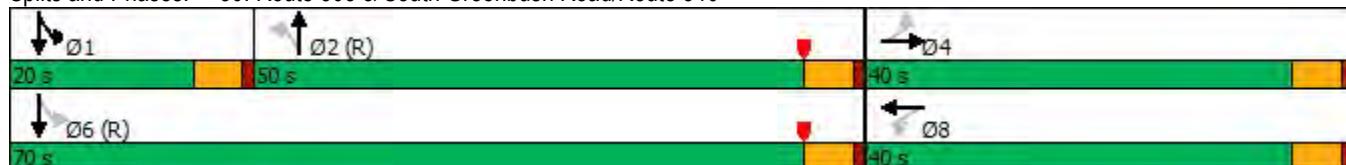
ICU Level of Service G

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 30: Route 303 & South Greenbush Road/Route 340



Lane Group	Ø6
Internal Link Dist (ft)	
Turn Bay Length (ft)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

2686-99-013T

No Build - AM

30: Route 303 & South Greenbush Road/Route 340

	↗	→	↘	↙	←	↖	↑	↗	↘	↓	↙	
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	26	30	7	59	31	227	7	594	64	248	545	20
Future Volume (vph)	26	30	7	59	31	227	7	594	64	248	545	20
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	16	16	16	12	12	12	11	11	11	11	11	11
Grade (%)	-2%				-3%			-3%			-1%	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Ped Bike Factor											1.00	
Frt		0.986				0.903			0.986			0.996
Flt Protected		0.980				0.991			0.999			0.985
Satd. Flow (prot)	0	1877	0	0	1616	0	0	3144	0	0	3139	0
Flt Permitted		0.621				0.929			0.947			0.578
Satd. Flow (perm)	0	1189	0	0	1515	0	0	2981	0	0	1842	0
Right Turn on Red			Yes				Yes			Yes		No
Satd. Flow (RTOR)		6				121			12			
Link Speed (mph)		30				40			40			40
Link Distance (ft)		280				324			945			621
Travel Time (s)		6.4				5.5			16.1			10.6
Confl. Peds. (#/hr)								1				1
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	21%	7%	0%	12%	3%	6%	17%	11%	10%	8%	10%	17%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	66	0	0	334	0	0	699	0	0	856	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		custom	NA	
Protected Phases		4				8			2		1	1 6
Permitted Phases	4				8			2			6	
Detector Phase	4	4		8	8		2	2			1	1 6
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		45.0	45.0			5.0	
Minimum Split (s)	10.0	10.0		10.0	10.0		50.0	50.0			10.0	
Total Split (s)	40.0	40.0		40.0	40.0		50.0	50.0			20.0	
Total Split (%)	36.4%	36.4%		36.4%	36.4%		45.5%	45.5%			18.2%	
Yellow Time (s)	4.0	4.0		4.0	4.0		4.0	4.0			4.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0			1.0	
Lost Time Adjust (s)		0.0			0.0			0.0				
Total Lost Time (s)		5.0			5.0			5.0				
Lead/Lag							Lag	Lag		Lead		
Lead-Lag Optimize?							Yes	Yes		Yes		
Recall Mode	None	None		None	None		C-Min	C-Min		None		
Act Effct Green (s)		21.4				21.4			59.8			78.6
Actuated g/C Ratio		0.19				0.19			0.54			0.71
v/c Ratio		0.28				0.85			0.43			0.58
Control Delay		34.3				46.3			17.4			10.2
Queue Delay		0.0				0.0			0.0			0.0
Total Delay		34.3				46.3			17.4			10.2
LOS		C				D			B			B
Approach Delay		34.3				46.3			17.4			10.2
Approach LOS		C				D			B			B
Queue Length 50th (ft)		36				150			148			69

Lane Group	Ø6
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Width (ft)	
Grade (%)	
Lane Util. Factor	
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Confl. Peds. (#/hr)	
Peak Hour Factor	
Heavy Vehicles (%)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Turn Type	
Protected Phases	6
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	45.0
Minimum Split (s)	50.0
Total Split (s)	70.0
Total Split (%)	64%
Yellow Time (s)	4.0
All-Red Time (s)	1.0
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Recall Mode	C-Min
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (ft)	

2686-99-013T

No Build - AM

30: Route 303 & South Greenbush Road/Route 340



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Queue Length 95th (ft)	68			232			240			143		
Internal Link Dist (ft)	200			244			865			541		
Turn Bay Length (ft)												
Base Capacity (vph)	382				564			1624			1493	
Starvation Cap Reductn	0				0			0			0	
Spillback Cap Reductn	0				0			0			0	
Storage Cap Reductn	0				0			0			0	
Reduced v/c Ratio	0.17				0.59			0.43			0.57	

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBTL, Start of Yellow

Natural Cycle: 75

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.85

Intersection Signal Delay: 19.8

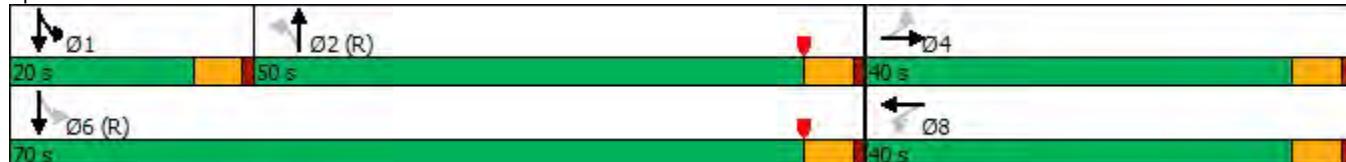
Intersection LOS: B

Intersection Capacity Utilization 93.2%

ICU Level of Service F

Analysis Period (min) 15

Splits and Phases: 30: Route 303 & South Greenbush Road/Route 340



Lane Group	Ø6
Queue Length 95th (ft)	
Internal Link Dist (ft)	
Turn Bay Length (ft)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

2686-99-013T

No Build - PM

30: Route 303 & South Greenbush Road/Route 340

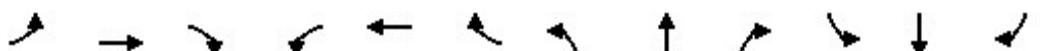
	→	→	←	←	↑	↑	↓	↓	↗	↙	↖	↙
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	26	30	11	94	53	343	16	599	50	244	780	22
Future Volume (vph)	26	30	11	94	53	343	16	599	50	244	780	22
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	16	16	16	12	12	12	11	11	11	11	11	11
Grade (%)	-2%			-3%			-3%			-1%		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Frt		0.978			0.905			0.989			0.997	
Flt Protected		0.981			0.991			0.999			0.988	
Satd. Flow (prot)	0	2054	0	0	1658	0	0	3373	0	0	3311	0
Flt Permitted		0.688			0.920			0.906			0.565	
Satd. Flow (perm)	0	1441	0	0	1539	0	0	3059	0	0	1894	0
Right Turn on Red			Yes			Yes			Yes			No
Satd. Flow (RTOR)		9			112			9				
Link Speed (mph)		30			40			40			40	
Link Distance (ft)		280			324			945			621	
Travel Time (s)		6.4			5.5			16.1			10.6	
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Heavy Vehicles (%)	4%	0%	0%	1%	6%	5%	0%	4%	2%	4%	4%	20%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	74	0	0	538	0	0	731	0	0	1149	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		custom	NA	
Protected Phases		4			8			2		1	16	
Permitted Phases	4			8			2			6		
Detector Phase	4	4		8	8		2	2		1	16	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		45.0	45.0		5.0		
Minimum Split (s)	10.0	10.0		10.0	10.0		50.0	50.0		10.0		
Total Split (s)	40.0	40.0		40.0	40.0		50.0	50.0		20.0		
Total Split (%)	36.4%	36.4%		36.4%	36.4%		45.5%	45.5%		18.2%		
Yellow Time (s)	4.0	4.0		4.0	4.0		4.0	4.0		4.0		
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0		
Lost Time Adjust (s)		0.0			0.0			0.0				
Total Lost Time (s)		5.0			5.0			5.0				
Lead/Lag							Lag	Lag		Lead		
Lead-Lag Optimize?							Yes	Yes		Yes		
Recall Mode	None	None		None	None		C-Min	C-Min		None		
Act Effct Green (s)		34.1			34.1			45.9			65.9	
Actuated g/C Ratio		0.31			0.31			0.42			0.60	
v/c Ratio		0.16			0.97			0.57			0.87	
Control Delay		24.8			62.2			26.6			25.0	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		24.8			62.2			26.6			25.0	
LOS	C		E			C			C			
Approach Delay		24.8			62.2			26.6			25.0	
Approach LOS		C		E			C			C		
Queue Length 50th (ft)		33			307			204			191	
Queue Length 95th (ft)		69			#530			266			#274	
Internal Link Dist (ft)		200			244			865			541	

Lane Group	Ø6
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Width (ft)	
Grade (%)	
Lane Util. Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Peak Hour Factor	
Heavy Vehicles (%)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Turn Type	
Protected Phases	6
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	45.0
Minimum Split (s)	50.0
Total Split (s)	70.0
Total Split (%)	64%
Yellow Time (s)	4.0
All-Red Time (s)	1.0
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Recall Mode	C-Min
Act Effect Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (ft)	
Queue Length 95th (ft)	
Internal Link Dist (ft)	

2686-99-013T

No Build - PM

30: Route 303 & South Greenbush Road/Route 340



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)												
Base Capacity (vph)	464				566			1282			1328	
Starvation Cap Reductn	0				0			0			0	
Spillback Cap Reductn	0				0			0			0	
Storage Cap Reductn	0				0			0			0	
Reduced v/c Ratio	0.16				0.95			0.57			0.87	

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBTL, Start of Yellow

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.97

Intersection Signal Delay: 33.5

Intersection LOS: C

Intersection Capacity Utilization 111.2%

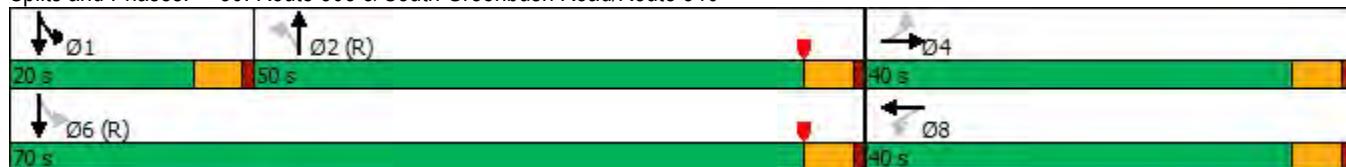
ICU Level of Service H

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 30: Route 303 & South Greenbush Road/Route 340



Lane Group	Ø6
Turn Bay Length (ft)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	26	30	7	59	31	234	7	597	64	250	546	20
Future Volume (vph)	26	30	7	59	31	234	7	597	64	250	546	20
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	16	16	16	12	12	12	11	11	11	11	11	11
Grade (%)	-2%				-3%			-3%			-1%	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Ped Bike Factor									1.00		1.00	
Frt		0.986				0.903			0.986			0.996
Flt Protected		0.980				0.991						0.985
Satd. Flow (prot)	0	1877	0	0	1616	0	0	3147	0	0	3139	0
Flt Permitted		0.619				0.930			0.947			0.576
Satd. Flow (perm)	0	1185	0	0	1516	0	0	2981	0	0	1836	0
Right Turn on Red			Yes				Yes			Yes		No
Satd. Flow (RTOR)		6			124				12			
Link Speed (mph)		30			40				40			40
Link Distance (ft)		280			324			945			621	
Travel Time (s)		6.4			5.5			16.1			10.6	
Confl. Peds. (#/hr)								1				1
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	21%	7%	0%	12%	3%	6%	17%	11%	10%	8%	10%	17%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	66	0	0	341	0	0	702	0	0	859	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		custom	NA	
Protected Phases		4			8			2		1	1	6
Permitted Phases	4			8			2			6		
Detector Phase	4	4		8	8		2	2		1	1	6
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		45.0	45.0			5.0	
Minimum Split (s)	10.0	10.0		10.0	10.0		50.0	50.0			10.0	
Total Split (s)	40.0	40.0		40.0	40.0		50.0	50.0			20.0	
Total Split (%)	36.4%	36.4%		36.4%	36.4%		45.5%	45.5%			18.2%	
Yellow Time (s)	4.0	4.0		4.0	4.0		4.0	4.0			4.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0			1.0	
Lost Time Adjust (s)		0.0			0.0			0.0				
Total Lost Time (s)		5.0			5.0			5.0				
Lead/Lag							Lag	Lag		Lead		
Lead-Lag Optimize?							Yes	Yes		Yes		
Recall Mode	None	None		None	None		C-Min	C-Min		None		
Act Effct Green (s)		21.7			21.7			59.4			78.3	
Actuated g/C Ratio		0.20			0.20			0.54			0.71	
v/c Ratio		0.28			0.86			0.43			0.58	
Control Delay		33.9			46.1			17.7			10.4	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		33.9			46.1			17.7			10.4	
LOS		C			D			B			B	
Approach Delay		33.9			46.1			17.7			10.4	
Approach LOS		C			D			B			B	
Queue Length 50th (ft)		36			153			150			70	

Lane Group	Ø6
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Width (ft)	
Grade (%)	
Lane Util. Factor	
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Confl. Peds. (#/hr)	
Peak Hour Factor	
Heavy Vehicles (%)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Turn Type	
Protected Phases	6
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	45.0
Minimum Split (s)	50.0
Total Split (s)	70.0
Total Split (%)	64%
Yellow Time (s)	4.0
All-Red Time (s)	1.0
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Recall Mode	C-Min
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (ft)	



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Queue Length 95th (ft)		68			237			243			145	
Internal Link Dist (ft)		200			244			865			541	
Turn Bay Length (ft)												
Base Capacity (vph)		381			566			1614			1484	
Starvation Cap Reductn		0			0			0			0	
Spillback Cap Reductn		0			0			0			0	
Storage Cap Reductn		0			0			0			0	
Reduced v/c Ratio		0.17			0.60			0.43			0.58	

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBTL, Start of Yellow

Natural Cycle: 75

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.86

Intersection Signal Delay: 20.0

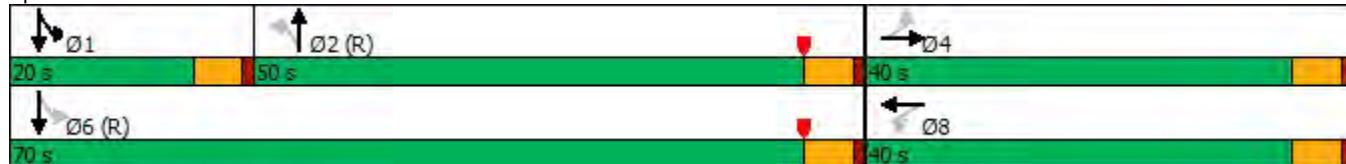
Intersection LOS: B

Intersection Capacity Utilization 93.7%

ICU Level of Service F

Analysis Period (min) 15

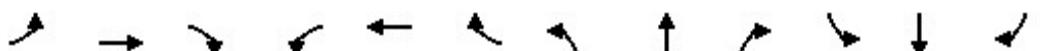
Splits and Phases: 30: Route 303 & South Greenbush Road/Route 340



Lane Group	Ø6
Queue Length 95th (ft)	
Internal Link Dist (ft)	
Turn Bay Length (ft)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

	→	→	←	←	↑	↑	↓	↓	↗	↙	↖	↙
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	26	30	11	94	53	345	16	601	50	250	783	22
Future Volume (vph)	26	30	11	94	53	345	16	601	50	250	783	22
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	16	16	16	12	12	12	11	11	11	11	11	11
Grade (%)	-2%			-3%			-3%			-1%		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Frt		0.978			0.905			0.989			0.997	
Flt Protected		0.981			0.991			0.999			0.988	
Satd. Flow (prot)	0	2054	0	0	1658	0	0	3373	0	0	3311	0
Flt Permitted		0.687			0.920			0.906			0.563	
Satd. Flow (perm)	0	1439	0	0	1539	0	0	3059	0	0	1887	0
Right Turn on Red			Yes			Yes			Yes			No
Satd. Flow (RTOR)		9			113			9				
Link Speed (mph)		30			40			40			40	
Link Distance (ft)		280			324			945			621	
Travel Time (s)		6.4			5.5			16.1			10.6	
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Heavy Vehicles (%)	4%	0%	0%	1%	6%	5%	0%	4%	2%	4%	4%	20%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	74	0	0	540	0	0	733	0	0	1159	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		custom	NA	
Protected Phases		4			8			2		1	16	
Permitted Phases	4			8			2			6		
Detector Phase	4	4		8	8		2	2		1	16	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		45.0	45.0		5.0		
Minimum Split (s)	10.0	10.0		10.0	10.0		50.0	50.0		10.0		
Total Split (s)	40.0	40.0		40.0	40.0		50.0	50.0		20.0		
Total Split (%)	36.4%	36.4%		36.4%	36.4%		45.5%	45.5%		18.2%		
Yellow Time (s)	4.0	4.0		4.0	4.0		4.0	4.0		4.0		
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0		
Lost Time Adjust (s)		0.0			0.0			0.0				
Total Lost Time (s)		5.0			5.0			5.0				
Lead/Lag							Lag	Lag		Lead		
Lead-Lag Optimize?							Yes	Yes		Yes		
Recall Mode	None	None		None	None		C-Min	C-Min		None		
Act Effct Green (s)		34.1			34.1			45.9			65.9	
Actuated g/C Ratio		0.31			0.31			0.42			0.60	
v/c Ratio		0.16			0.97			0.57			0.88	
Control Delay		24.8			62.7			26.6			25.8	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		24.8			62.7			26.6			25.8	
LOS	C		E				C			C		
Approach Delay		24.8			62.7			26.6			25.8	
Approach LOS		C		E			C			C		
Queue Length 50th (ft)		33			308			205			196	
Queue Length 95th (ft)		69			#533			267			#286	
Internal Link Dist (ft)		200			244			865			541	

Lane Group	Ø6
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Width (ft)	
Grade (%)	
Lane Util. Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Peak Hour Factor	
Heavy Vehicles (%)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Turn Type	
Protected Phases	6
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	45.0
Minimum Split (s)	50.0
Total Split (s)	70.0
Total Split (%)	64%
Yellow Time (s)	4.0
All-Red Time (s)	1.0
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Recall Mode	C-Min
Act Effect Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (ft)	
Queue Length 95th (ft)	
Internal Link Dist (ft)	



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)												
Base Capacity (vph)	464			566			1281			1324		
Starvation Cap Reductn	0			0			0			0		
Spillback Cap Reductn	0			0			0			0		
Storage Cap Reductn	0			0			0			0		
Reduced v/c Ratio	0.16			0.95			0.57			0.88		

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 0 (0%), Referenced to phase 2:NBTL and 6:SBTL, Start of Yellow

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.97

Intersection Signal Delay: 34.0

Intersection LOS: C

Intersection Capacity Utilization 111.6%

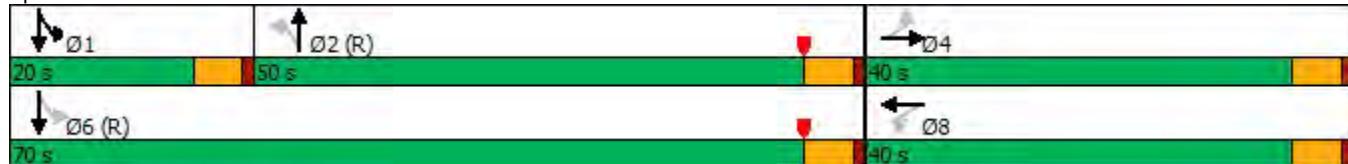
ICU Level of Service H

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 30: Route 303 & South Greenbush Road/Route 340



Lane Group	Ø6
Turn Bay Length (ft)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

2686-99-013T

Design Horizon Year No Build - AM

30: Route 303 & South Greenbush Road/Route 340

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	32	37	9	72	38	277	9	726	78	303	665	24
Future Volume (vph)	32	37	9	72	38	277	9	726	78	303	665	24
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	16	16	16	12	12	12	11	11	11	11	11	11
Grade (%)	-2%				-3%			-3%			-1%	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Ped Bike Factor											1.00	
Frt		0.985				0.903			0.986			0.996
Flt Protected		0.980				0.991			0.999			0.985
Satd. Flow (prot)	0	1874	0	0	1616	0	0	3144	0	0	3139	0
Flt Permitted		0.624				0.925			0.942			0.532
Satd. Flow (perm)	0	1193	0	0	1508	0	0	2965	0	0	1695	0
Right Turn on Red			Yes				Yes			Yes		No
Satd. Flow (RTOR)		6			121				12			
Link Speed (mph)		30			40			40			40	
Link Distance (ft)		280			324			945			621	
Travel Time (s)		6.4			5.5			16.1			10.6	
Confl. Peds. (#/hr)								1				1
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	21%	7%	0%	12%	3%	6%	17%	11%	10%	8%	10%	17%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	82	0	0	408	0	0	855	0	0	1044	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		custom	NA	
Protected Phases		4			8			2		1	16	
Permitted Phases	4			8			2			6		
Detector Phase	4	4		8	8		2	2		1	16	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		45.0	45.0			5.0	
Minimum Split (s)	10.0	10.0		10.0	10.0		50.0	50.0			10.0	
Total Split (s)	40.0	40.0		40.0	40.0		50.0	50.0			20.0	
Total Split (%)	36.4%	36.4%		36.4%	36.4%		45.5%	45.5%			18.2%	
Yellow Time (s)	4.0	4.0		4.0	4.0		4.0	4.0			4.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0			1.0	
Lost Time Adjust (s)		0.0			0.0			0.0				
Total Lost Time (s)		5.0			5.0			5.0				
Lead/Lag							Lag	Lag		Lead		
Lead-Lag Optimize?							Yes	Yes		Yes		
Recall Mode	None	None		None	None		C-Min	C-Min		None		
Act Effct Green (s)		26.6			26.6			53.4			73.4	
Actuated g/C Ratio		0.24			0.24			0.49			0.67	
v/c Ratio		0.28			0.89			0.59			0.79	
Control Delay		31.4			50.1			23.7			19.0	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		31.4			50.1			23.7			19.0	
LOS		C			D			C			B	
Approach Delay		31.4			50.1			23.7			19.0	
Approach LOS		C			D			C			B	
Queue Length 50th (ft)		43			202			222			140	

Lane Group	Ø6
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Width (ft)	
Grade (%)	
Lane Util. Factor	
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Confl. Peds. (#/hr)	
Peak Hour Factor	
Heavy Vehicles (%)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Turn Type	
Protected Phases	6
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	45.0
Minimum Split (s)	50.0
Total Split (s)	70.0
Total Split (%)	64%
Yellow Time (s)	4.0
All-Red Time (s)	1.0
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Recall Mode	C-Min
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (ft)	



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Queue Length 95th (ft)		79			303			329			#264	
Internal Link Dist (ft)		200			244			865			541	
Turn Bay Length (ft)												
Base Capacity (vph)		383			562			1445			1328	
Starvation Cap Reductn		0			0			0			0	
Spillback Cap Reductn		0			0			0			0	
Storage Cap Reductn		0			0			0			0	
Reduced v/c Ratio		0.21			0.73			0.59			0.79	

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBTL, Start of Yellow

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.89

Intersection Signal Delay: 26.4

Intersection LOS: C

Intersection Capacity Utilization 102.8%

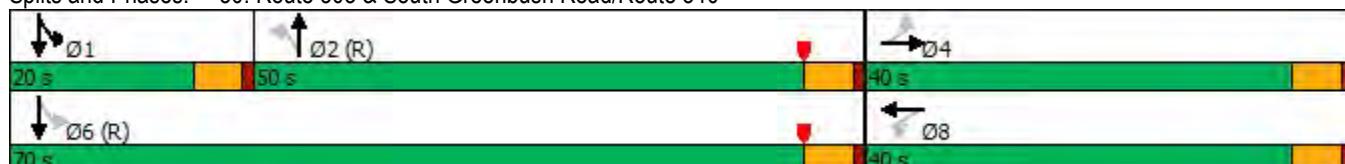
ICU Level of Service G

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 30: Route 303 & South Greenbush Road/Route 340



Lane Group	Ø6
Queue Length 95th (ft)	
Internal Link Dist (ft)	
Turn Bay Length (ft)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

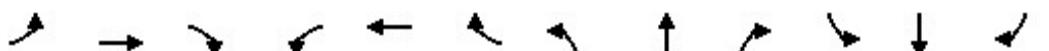
2686-99-013T

Design Horizon Year No Build - PM

30: Route 303 & South Greenbush Road/Route 340

	→	→	→	←	←	↑	↑	↓	↓	←	→	
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	32	37	13	115	65	419	20	731	61	298	952	27
Future Volume (vph)	32	37	13	115	65	419	20	731	61	298	952	27
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	16	16	16	12	12	12	11	11	11	11	11	11
Grade (%)	-2%			-3%			-3%			-1%		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Frt		0.979			0.905			0.989			0.997	
Flt Protected		0.981			0.991			0.999			0.988	
Satd. Flow (prot)	0	2057	0	0	1658	0	0	3373	0	0	3311	0
Flt Permitted		0.617			0.915			0.883			0.522	
Satd. Flow (perm)	0	1294	0	0	1531	0	0	2981	0	0	1749	0
Right Turn on Red			Yes			Yes			Yes			No
Satd. Flow (RTOR)		9			112			9				
Link Speed (mph)		30			40			40			40	
Link Distance (ft)		280			324			945			621	
Travel Time (s)		6.4			5.5			16.1			10.6	
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Heavy Vehicles (%)	4%	0%	0%	1%	6%	5%	0%	4%	2%	4%	4%	20%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	90	0	0	657	0	0	892	0	0	1403	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		custom	NA	
Protected Phases		4			8			2		1	16	
Permitted Phases	4			8			2			6		
Detector Phase	4	4		8	8		2	2		1	16	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		45.0	45.0		5.0		
Minimum Split (s)	10.0	10.0		10.0	10.0		50.0	50.0		10.0		
Total Split (s)	40.0	40.0		40.0	40.0		50.0	50.0		20.0		
Total Split (%)	36.4%	36.4%		36.4%	36.4%		45.5%	45.5%		18.2%		
Yellow Time (s)	4.0	4.0		4.0	4.0		4.0	4.0		4.0		
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0		
Lost Time Adjust (s)		0.0			0.0			0.0				
Total Lost Time (s)		5.0			5.0			5.0				
Lead/Lag							Lag	Lag		Lead		
Lead-Lag Optimize?							Yes	Yes		Yes		
Recall Mode	None	None		None	None		C-Min	C-Min		None		
Act Effct Green (s)		35.0			35.0			45.0			65.0	
Actuated g/C Ratio		0.32			0.32			0.41			0.59	
v/c Ratio		0.22			1.17			0.73			1.13	
Control Delay		26.3			122.8			31.3			87.7	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		26.3			122.8			31.3			87.7	
LOS	C			F			C			F		
Approach Delay		26.3			122.8			31.3			87.7	
Approach LOS		C			F			C			F	
Queue Length 50th (ft)		41			~497			271			~263	
Queue Length 95th (ft)		83			#724			349		m#276		
Internal Link Dist (ft)		200			244			865			541	

Lane Group	Ø6
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Width (ft)	
Grade (%)	
Lane Util. Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Peak Hour Factor	
Heavy Vehicles (%)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Turn Type	
Protected Phases	6
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	45.0
Minimum Split (s)	50.0
Total Split (s)	70.0
Total Split (%)	64%
Yellow Time (s)	4.0
All-Red Time (s)	1.0
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Recall Mode	C-Min
Act Effect Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (ft)	
Queue Length 95th (ft)	
Internal Link Dist (ft)	



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)												
Base Capacity (vph)		417			563			1224			1246	
Starvation Cap Reductn		0			0			0			0	
Spillback Cap Reductn		0			0			0			0	
Storage Cap Reductn		0			0			0			0	
Reduced v/c Ratio		0.22			1.17			0.73			1.13	

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 0 (0%), Referenced to phase 2:NBTL and 6:SBTL, Start of Yellow

Natural Cycle: 100

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.17

Intersection Signal Delay: 76.9

Intersection LOS: E

Intersection Capacity Utilization 124.8%

ICU Level of Service H

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 30: Route 303 & South Greenbush Road/Route 340



Lane Group	Ø6
Turn Bay Length (ft)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
<hr/> Intersection Summary	

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	32	37	9	72	38	284	9	729	78	305	666	24
Future Volume (vph)	32	37	9	72	38	284	9	729	78	305	666	24
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	16	16	16	12	12	12	11	11	11	11	11	11
Grade (%)	-2%				-3%			-3%			-1%	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Ped Bike Factor											1.00	
Frt		0.985				0.903			0.986			0.996
Flt Protected		0.980				0.991			0.999			0.985
Satd. Flow (prot)	0	1874	0	0	1616	0	0	3144	0	0	3139	0
Flt Permitted		0.621				0.926			0.942			0.531
Satd. Flow (perm)	0	1187	0	0	1510	0	0	2965	0	0	1692	0
Right Turn on Red			Yes				Yes			Yes		No
Satd. Flow (RTOR)		6			124				12			
Link Speed (mph)		30			40			40			40	
Link Distance (ft)		280			324			945			621	
Travel Time (s)		6.4			5.5			16.1			10.6	
Confl. Peds. (#/hr)								1				1
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	21%	7%	0%	12%	3%	6%	17%	11%	10%	8%	10%	17%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	82	0	0	415	0	0	858	0	0	1047	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		custom	NA	
Protected Phases		4			8			2		1	16	
Permitted Phases	4			8			2			6		
Detector Phase	4	4		8	8		2	2		1	16	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		45.0	45.0			5.0	
Minimum Split (s)	10.0	10.0		10.0	10.0		50.0	50.0			10.0	
Total Split (s)	40.0	40.0		40.0	40.0		50.0	50.0			20.0	
Total Split (%)	36.4%	36.4%		36.4%	36.4%		45.5%	45.5%			18.2%	
Yellow Time (s)	4.0	4.0		4.0	4.0		4.0	4.0			4.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0			1.0	
Lost Time Adjust (s)		0.0			0.0			0.0				
Total Lost Time (s)		5.0			5.0			5.0				
Lead/Lag							Lag	Lag		Lead		
Lead-Lag Optimize?							Yes	Yes		Yes		
Recall Mode	None	None		None	None		C-Min	C-Min		None		
Act Effct Green (s)		26.8			26.8			53.2			73.2	
Actuated g/C Ratio		0.24			0.24			0.48			0.67	
v/c Ratio		0.28			0.90			0.60			0.79	
Control Delay		31.3			50.2			23.9			19.4	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		31.3			50.2			23.9			19.4	
LOS		C			D			C			B	
Approach Delay		31.3			50.2			23.9			19.4	
Approach LOS		C			D			C			B	
Queue Length 50th (ft)		43			205			225			146	

Lane Group	Ø6
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Width (ft)	
Grade (%)	
Lane Util. Factor	
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Confl. Peds. (#/hr)	
Peak Hour Factor	
Heavy Vehicles (%)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Turn Type	
Protected Phases	6
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	45.0
Minimum Split (s)	50.0
Total Split (s)	70.0
Total Split (%)	64%
Yellow Time (s)	4.0
All-Red Time (s)	1.0
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Recall Mode	C-Min
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (ft)	



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Queue Length 95th (ft)		79			309			331			#269	
Internal Link Dist (ft)		200			244			865			541	
Turn Bay Length (ft)												
Base Capacity (vph)		381			565			1439			1322	
Starvation Cap Reductn		0			0			0			0	
Spillback Cap Reductn		0			0			0			0	
Storage Cap Reductn		0			0			0			0	
Reduced v/c Ratio		0.22			0.73			0.60			0.79	

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBTL, Start of Yellow

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.90

Intersection Signal Delay: 26.7

Intersection LOS: C

Intersection Capacity Utilization 103.3%

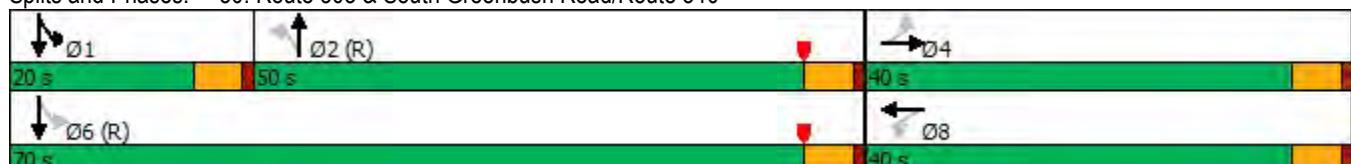
ICU Level of Service G

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 30: Route 303 & South Greenbush Road/Route 340



Lane Group	Ø6
Queue Length 95th (ft)	
Internal Link Dist (ft)	
Turn Bay Length (ft)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

	→	→	←	←	↑	↑	↓	↓	↗	↙	↖	↙
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	32	37	13	115	65	421	20	733	61	304	955	27
Future Volume (vph)	32	37	13	115	65	421	20	733	61	304	955	27
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	16	16	16	12	12	12	11	11	11	11	11	11
Grade (%)	-2%			-3%			-3%			-1%		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Frt		0.979			0.905			0.989			0.997	
Flt Protected		0.981			0.991			0.999			0.988	
Satd. Flow (prot)	0	2057	0	0	1658	0	0	3373	0	0	3311	0
Flt Permitted		0.616			0.916			0.882			0.521	
Satd. Flow (perm)	0	1291	0	0	1532	0	0	2978	0	0	1746	0
Right Turn on Red			Yes			Yes			Yes			No
Satd. Flow (RTOR)		9			113			9				
Link Speed (mph)		30			40			40			40	
Link Distance (ft)		280			324			945			621	
Travel Time (s)		6.4			5.5			16.1			10.6	
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Heavy Vehicles (%)	4%	0%	0%	1%	6%	5%	0%	4%	2%	4%	4%	20%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	90	0	0	660	0	0	894	0	0	1413	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		custom	NA	
Protected Phases		4			8			2		1	16	
Permitted Phases	4			8			2			6		
Detector Phase	4	4		8	8		2	2		1	16	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		45.0	45.0		5.0		
Minimum Split (s)	10.0	10.0		10.0	10.0		50.0	50.0		10.0		
Total Split (s)	40.0	40.0		40.0	40.0		50.0	50.0		20.0		
Total Split (%)	36.4%	36.4%		36.4%	36.4%		45.5%	45.5%		18.2%		
Yellow Time (s)	4.0	4.0		4.0	4.0		4.0	4.0		4.0		
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0		
Lost Time Adjust (s)		0.0			0.0			0.0				
Total Lost Time (s)		5.0			5.0			5.0				
Lead/Lag							Lag	Lag		Lead		
Lead-Lag Optimize?							Yes	Yes		Yes		
Recall Mode	None	None		None	None		C-Min	C-Min		None		
Act Effct Green (s)		35.0			35.0			45.0			65.0	
Actuated g/C Ratio		0.32			0.32			0.41			0.59	
v/c Ratio		0.22			1.17			0.73			1.13	
Control Delay		26.4			124.0			31.4			91.3	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		26.4			124.0			31.4			91.3	
LOS	C		F			C			F			
Approach Delay		26.4			124.0			31.4			91.3	
Approach LOS		C		F			C			F		
Queue Length 50th (ft)		41			~500			272			~270	
Queue Length 95th (ft)		83			#728			350		m#275		
Internal Link Dist (ft)		200			244			865			541	

Lane Group	Ø6
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Width (ft)	
Grade (%)	
Lane Util. Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Peak Hour Factor	
Heavy Vehicles (%)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Turn Type	
Protected Phases	6
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	45.0
Minimum Split (s)	50.0
Total Split (s)	70.0
Total Split (%)	64%
Yellow Time (s)	4.0
All-Red Time (s)	1.0
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Recall Mode	C-Min
Act Effect Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (ft)	
Queue Length 95th (ft)	
Internal Link Dist (ft)	



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)												
Base Capacity (vph)		416			564			1223			1245	
Starvation Cap Reductn		0			0			0			0	
Spillback Cap Reductn		0			0			0			0	
Storage Cap Reductn		0			0			0			0	
Reduced v/c Ratio		0.22			1.17			0.73			1.13	

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 0 (0%), Referenced to phase 2:NBTL and 6:SBTL, Start of Yellow

Natural Cycle: 120

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.17

Intersection Signal Delay: 78.9

Intersection LOS: E

Intersection Capacity Utilization 125.1%

ICU Level of Service H

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

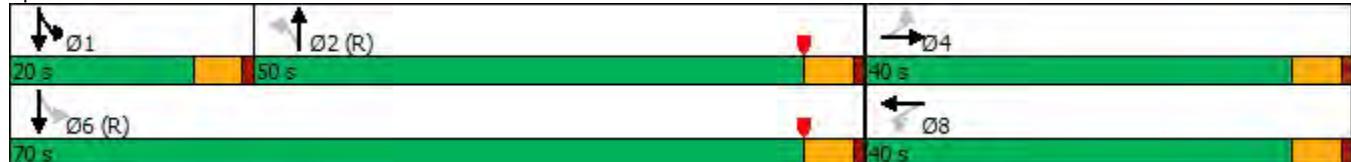
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 30: Route 303 & South Greenbush Road/Route 340



Lane Group	Ø6
Turn Bay Length (ft)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

Intersection

Int Delay, s/veh 1.1

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W		↑↑	↑↑		
Traffic Vol, veh/h	24	21	39	566	580	50
Future Vol, veh/h	24	21	39	566	580	50
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	-6	-	-	-1	-3	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	50	43	13	28	12	10
Mvmt Flow	25	22	41	596	611	53

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	1018	332	664	0	-	0
Stage 1	638	-	-	-	-	-
Stage 2	380	-	-	-	-	-
Critical Hdwy	6.6	7.16	4.36	-	-	-
Critical Hdwy Stg 1	5.6	-	-	-	-	-
Critical Hdwy Stg 2	5.6	-	-	-	-	-
Follow-up Hdwy	4	3.73	2.33	-	-	-
Pot Cap-1 Maneuver	232	589	851	-	-	-
Stage 1	466	-	-	-	-	-
Stage 2	611	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	215	589	851	-	-	-
Mov Cap-2 Maneuver	215	-	-	-	-	-
Stage 1	432	-	-	-	-	-
Stage 2	611	-	-	-	-	-

Approach EB NB SB

HCM Control Delay, s 18.9 0.9 0

HCM LOS C

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	851	-	306	-	-
HCM Lane V/C Ratio	0.048	-	0.155	-	-
HCM Control Delay (s)	9.4	0.3	18.9	-	-
HCM Lane LOS	A	A	C	-	-
HCM 95th %tile Q(veh)	0.2	-	0.5	-	-

Intersection

Int Delay, s/veh 1.5

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W			↑↑	↑↑	
Traffic Vol, veh/h	38	55	10	734	785	19
Future Vol, veh/h	38	55	10	734	785	19
Conflicting Peds, #/hr	0	0	1	0	0	1
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	-6	-	-	-1	-3	-
Peak Hour Factor	93	93	93	93	93	93
Heavy Vehicles, %	24	20	70	15	16	53
Mvmt Flow	41	59	11	789	844	20

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	1272	433	865	0	-	0
Stage 1	855	-	-	-	-	-
Stage 2	417	-	-	-	-	-
Critical Hdwy	6.08	6.7	5.5	-	-	-
Critical Hdwy Stg 1	5.08	-	-	-	-	-
Critical Hdwy Stg 2	5.08	-	-	-	-	-
Follow-up Hdwy	3.74	3.5	2.9	-	-	-
Pot Cap-1 Maneuver	202	563	460	-	-	-
Stage 1	435	-	-	-	-	-
Stage 2	659	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	193	563	460	-	-	-
Mov Cap-2 Maneuver	193	-	-	-	-	-
Stage 1	416	-	-	-	-	-
Stage 2	658	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	21.6	0.5	0
HCM LOS	C		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	460	-	316	-	-
HCM Lane V/C Ratio	0.023	-	0.316	-	-
HCM Control Delay (s)	13	0.3	21.6	-	-
HCM Lane LOS	B	A	C	-	-
HCM 95th %tile Q(veh)	0.1	-	1.3	-	-

Intersection

Int Delay, s/veh 2.2

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	33	34	90	619	641	83
Future Vol, veh/h	33	34	90	619	641	83
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	-6	-	-	-1	-3	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	50	43	13	28	12	10
Mvmt Flow	35	36	95	652	675	87

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	1235	381	762	0	-	0
Stage 1	719	-	-	-	-	-
Stage 2	516	-	-	-	-	-
Critical Hdwy	6.6	7.16	4.36	-	-	-
Critical Hdwy Stg 1	5.6	-	-	-	-	-
Critical Hdwy Stg 2	5.6	-	-	-	-	-
Follow-up Hdwy	4	3.73	2.33	-	-	-
Pot Cap-1 Maneuver	172	548	778	-	-	-
Stage 1	427	-	-	-	-	-
Stage 2	530	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	139	548	778	-	-	-
Mov Cap-2 Maneuver	139	-	-	-	-	-
Stage 1	345	-	-	-	-	-
Stage 2	530	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	28.3	2	0
HCM LOS	D		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	778	-	224	-	-
HCM Lane V/C Ratio	0.122	-	0.315	-	-
HCM Control Delay (s)	10.3	0.8	28.3	-	-
HCM Lane LOS	B	A	D	-	-
HCM 95th %tile Q(veh)	0.4	-	1.3	-	-

Intersection

Int Delay, s/veh 4

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W			↑↑	↑↑	
Traffic Vol, veh/h	65	94	23	802	835	27
Future Vol, veh/h	65	94	23	802	835	27
Conflicting Peds, #/hr	0	0	1	0	0	1
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	-6	-	-	-1	-3	-
Peak Hour Factor	93	93	93	93	93	93
Heavy Vehicles, %	24	20	70	15	16	53
Mvmt Flow	70	101	25	862	898	29

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	1395	465	928	0	-	0
Stage 1	914	-	-	-	-	-
Stage 2	481	-	-	-	-	-
Critical Hdwy	6.08	6.7	5.5	-	-	-
Critical Hdwy Stg 1	5.08	-	-	-	-	-
Critical Hdwy Stg 2	5.08	-	-	-	-	-
Follow-up Hdwy	3.74	3.5	2.9	-	-	-
Pot Cap-1 Maneuver	173	538	427	-	-	-
Stage 1	410	-	-	-	-	-
Stage 2	620	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	153	538	427	-	-	-
Mov Cap-2 Maneuver	153	-	-	-	-	-
Stage 1	364	-	-	-	-	-
Stage 2	619	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	40.3	1.1	0
HCM LOS	E		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	427	-	265	-	-
HCM Lane V/C Ratio	0.058	-	0.645	-	-
HCM Control Delay (s)	13.9	0.7	40.3	-	-
HCM Lane LOS	B	A	E	-	-
HCM 95th %tile Q(veh)	0.2	-	4.1	-	-

Intersection

Int Delay, s/veh 2.2

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W		↑↑	↑↑		
Traffic Vol, veh/h	33	34	90	622	653	83
Future Vol, veh/h	33	34	90	622	653	83
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	-6	-	-	-1	-3	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	50	43	13	28	12	10
Mvmt Flow	35	36	95	655	687	87

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	1249	387	774	0	-	0
Stage 1	731	-	-	-	-	-
Stage 2	518	-	-	-	-	-
Critical Hdwy	6.6	7.16	4.36	-	-	-
Critical Hdwy Stg 1	5.6	-	-	-	-	-
Critical Hdwy Stg 2	5.6	-	-	-	-	-
Follow-up Hdwy	4	3.73	2.33	-	-	-
Pot Cap-1 Maneuver	169	543	769	-	-	-
Stage 1	422	-	-	-	-	-
Stage 2	529	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	136	543	769	-	-	-
Mov Cap-2 Maneuver	136	-	-	-	-	-
Stage 1	340	-	-	-	-	-
Stage 2	529	-	-	-	-	-

Approach EB NB SB

HCM Control Delay, s 29.1 2 0

HCM LOS D

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	769	-	219	-	-
HCM Lane V/C Ratio	0.123	-	0.322	-	-
HCM Control Delay (s)	10.3	0.8	29.1	-	-
HCM Lane LOS	B	A	D	-	-
HCM 95th %tile Q(veh)	0.4	-	1.3	-	-

Intersection

Int Delay, s/veh 4.1

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	65	94	23	815	840	27
Future Vol, veh/h	65	94	23	815	840	27
Conflicting Peds, #/hr	0	0	1	0	0	1
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	-6	-	-	-1	-3	-
Peak Hour Factor	93	93	93	93	93	93
Heavy Vehicles, %	24	20	70	15	16	53
Mvmt Flow	70	101	25	876	903	29

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	1407	467	933	0	-	0
Stage 1	919	-	-	-	-	-
Stage 2	488	-	-	-	-	-
Critical Hdwy	6.08	6.7	5.5	-	-	-
Critical Hdwy Stg 1	5.08	-	-	-	-	-
Critical Hdwy Stg 2	5.08	-	-	-	-	-
Follow-up Hdwy	3.74	3.5	2.9	-	-	-
Pot Cap-1 Maneuver	170	537	425	-	-	-
Stage 1	408	-	-	-	-	-
Stage 2	616	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	150	537	425	-	-	-
Mov Cap-2 Maneuver	150	-	-	-	-	-
Stage 1	361	-	-	-	-	-
Stage 2	615	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	41.6	1.2	0
HCM LOS	E		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	425	-	261	-	-
HCM Lane V/C Ratio	0.058	-	0.655	-	-
HCM Control Delay (s)	14	0.8	41.6	-	-
HCM Lane LOS	B	A	E	-	-
HCM 95th %tile Q(veh)	0.2	-	4.2	-	-

Intersection

Int Delay, s/veh 4.1

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W			↑↑	↑↑	
Traffic Vol, veh/h	40	41	110	755	783	101
Future Vol, veh/h	40	41	110	755	783	101
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	-6	-	-	-1	-3	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	50	43	13	28	12	10
Mvmt Flow	42	43	116	795	824	106

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	1507	465	930	0	-	0
Stage 1	877	-	-	-	-	-
Stage 2	630	-	-	-	-	-
Critical Hdwy	6.6	7.16	4.36	-	-	-
Critical Hdwy Stg 1	5.6	-	-	-	-	-
Critical Hdwy Stg 2	5.6	-	-	-	-	-
Follow-up Hdwy	4	3.73	2.33	-	-	-
Pot Cap-1 Maneuver	117	482	667	-	-	-
Stage 1	360	-	-	-	-	-
Stage 2	470	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	81	482	667	-	-	-
Mov Cap-2 Maneuver	81	-	-	-	-	-
Stage 1	248	-	-	-	-	-
Stage 2	470	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	64.3	2.7	0
HCM LOS	F		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	667	-	140	-	-
HCM Lane V/C Ratio	0.174	-	0.609	-	-
HCM Control Delay (s)	11.5	1.4	64.3	-	-
HCM Lane LOS	B	A	F	-	-
HCM 95th %tile Q(veh)	0.6	-	3.2	-	-

Intersection

Int Delay, s/veh 16.9

Movement	EBL	EBR	NBL	NBT	SBT	SBR
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Lane Configurations						
Traffic Vol, veh/h	79	115	28	980	1020	33
Future Vol, veh/h	79	115	28	980	1020	33
Conflicting Peds, #/hr	0	0	1	0	0	1
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	-6	-	-	-1	-3	-
Peak Hour Factor	93	93	93	93	93	93
Heavy Vehicles, %	24	20	70	15	16	53
Mvmt Flow	85	124	30	1054	1097	35

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	1703	567	1133	0	-	0
Stage 1	1116	-	-	-	-	-
Stage 2	587	-	-	-	-	-
Critical Hdwy	6.08	6.7	5.5	-	-	-
Critical Hdwy Stg 1	5.08	-	-	-	-	-
Critical Hdwy Stg 2	5.08	-	-	-	-	-
Follow-up Hdwy	3.74	3.5	2.9	-	-	-
Pot Cap-1 Maneuver	116	466	335	-	-	-
Stage 1	337	-	-	-	-	-
Stage 2	562	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	91	466	335	-	-	-
Mov Cap-2 Maneuver	91	-	-	-	-	-
Stage 1	264	-	-	-	-	-
Stage 2	561	-	-	-	-	-

Approach	EB	NB	SB
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HCM Control Delay, s 185.2 2.1 0

HCM LOS F

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	335	-	174	-	-
HCM Lane V/C Ratio	0.09	-	1.199	-	-
HCM Control Delay (s)	16.8	1.7	185.2	-	-
HCM Lane LOS	C	A	F	-	-
HCM 95th %tile Q(veh)	0.3	-	11.3	-	-

Intersection

Int Delay, s/veh 4.2

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W			↑↑	↑↑	
Traffic Vol, veh/h	40	41	110	758	795	101
Future Vol, veh/h	40	41	110	758	795	101
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	-6	-	-	-1	-3	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	50	43	13	28	12	10
Mvmt Flow	42	43	116	798	837	106

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	1521	472	943	0	-	0
Stage 1	890	-	-	-	-	-
Stage 2	631	-	-	-	-	-
Critical Hdwy	6.6	7.16	4.36	-	-	-
Critical Hdwy Stg 1	5.6	-	-	-	-	-
Critical Hdwy Stg 2	5.6	-	-	-	-	-
Follow-up Hdwy	4	3.73	2.33	-	-	-
Pot Cap-1 Maneuver	115	477	659	-	-	-
Stage 1	355	-	-	-	-	-
Stage 2	469	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	79	477	659	-	-	-
Mov Cap-2 Maneuver	79	-	-	-	-	-
Stage 1	243	-	-	-	-	-
Stage 2	469	-	-	-	-	-

Approach EB NB SB

HCM Control Delay, s 67.1 2.7 0

HCM LOS F

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	659	-	137	-	-
HCM Lane V/C Ratio	0.176	-	0.622	-	-
HCM Control Delay (s)	11.6	1.4	67.1	-	-
HCM Lane LOS	B	A	F	-	-
HCM 95th %tile Q(veh)	0.6	-	3.3	-	-

Intersection

Int Delay, s/veh 17.5

Movement	EBL	EBR	NBL	NBT	SBT	SBR
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Lane Configurations						
Traffic Vol, veh/h	79	115	28	993	1025	33
Future Vol, veh/h	79	115	28	993	1025	33
Conflicting Peds, #/hr	0	0	1	0	0	1
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	-6	-	-	-1	-3	-
Peak Hour Factor	93	93	93	93	93	93
Heavy Vehicles, %	24	20	70	15	16	53
Mvmt Flow	85	124	30	1068	1102	35

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	1715	570	1138	0	-	0
Stage 1	1121	-	-	-	-	-
Stage 2	594	-	-	-	-	-
Critical Hdwy	6.08	6.7	5.5	-	-	-
Critical Hdwy Stg 1	5.08	-	-	-	-	-
Critical Hdwy Stg 2	5.08	-	-	-	-	-
Follow-up Hdwy	3.74	3.5	2.9	-	-	-
Pot Cap-1 Maneuver	114	464	333	-	-	-
Stage 1	335	-	-	-	-	-
Stage 2	558	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	89	464	333	-	-	-
Mov Cap-2 Maneuver	89	-	-	-	-	-
Stage 1	261	-	-	-	-	-
Stage 2	557	-	-	-	-	-

Approach	EB	NB	SB
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HCM Control Delay, s 193.9 2.2 0

HCM LOS F

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	333	-	171	-	-
HCM Lane V/C Ratio	0.09	-	1.22	-	-
HCM Control Delay (s)	16.9	1.8	193.9	-	-
HCM Lane LOS	C	A	F	-	-
HCM 95th %tile Q(veh)	0.3	-	11.5	-	-

Intersection

Int Delay, s/veh 0

Movement	EBL	EBR	NBL	NBT	SBT	SBR
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Lane Configurations



Traffic Vol, veh/h 0 4 0 712 675 12

Future Vol, veh/h 0 4 0 712 675 12

Conflicting Peds, #/hr 0 0 0 0 0 0

Sign Control Stop Stop Free Free Free Free

RT Channelized - None - None - None

Storage Length - 0 - - - -

Veh in Median Storage, # 0 - - 0 0 -

Grade, % 0 - - 4 2 -

Peak Hour Factor 94 94 94 94 94 94

Heavy Vehicles, % 0 0 0 17 14 17

Mvmt Flow 0 4 0 757 718 13

Major/Minor	Minor2	Major1	Major2
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Conflicting Flow All - 366 - 0 - 0

Stage 1 - - - - - -

Stage 2 - - - - - -

Critical Hdwy - 6.9 - - - -

Critical Hdwy Stg 1 - - - - - -

Critical Hdwy Stg 2 - - - - - -

Follow-up Hdwy - 3.3 - - - -

Pot Cap-1 Maneuver 0 637 0 - - -

Stage 1 0 - 0 - - -

Stage 2 0 - 0 - - -

Platoon blocked, % - - - - - -

Mov Cap-1 Maneuver - 637 - - - -

Mov Cap-2 Maneuver - - - - - -

Stage 1 - - - - - -

Stage 2 - - - - - -

Approach	EB	NB	SB
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HCM Control Delay, s 10.7 0 0

HCM LOS B

Minor Lane/Major Mvmt	NBT	EBLn1	SBT	SBR
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Capacity (veh/h) - 637 - -

HCM Lane V/C Ratio - 0.007 - -

HCM Control Delay (s) - 10.7 - -

HCM Lane LOS - B - -

HCM 95th %tile Q(veh) - 0 - -

Intersection

Int Delay, s/veh 0.1

Movement	EBL	EBR	NBL	NBT	SBT	SBR
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Lane Configurations						
Traffic Vol, veh/h	0	13	0	838	929	5
Future Vol, veh/h	0	13	0	838	929	5
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	4	2	-
Peak Hour Factor	96	96	96	96	96	96
Heavy Vehicles, %	0	0	0	10	6	20
Mvmt Flow	0	14	0	873	968	5

Major/Minor	Minor2	Major1	Major2
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Conflicting Flow All	-	487	-	0	-	0
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.9	-	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.3	-	-	-	-
Pot Cap-1 Maneuver	0	532	0	-	-	-
Stage 1	0	-	0	-	-	-
Stage 2	0	-	0	-	-	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	-	532	-	-	-	-
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-

Approach	EB	NB	SB
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HCM Control Delay, s	11.9	0	0
HCM LOS	B		

Minor Lane/Major Mvmt	NBT	EBLn1	SBT	SBR
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Capacity (veh/h)	-	532	-	-
HCM Lane V/C Ratio	-	0.025	-	-
HCM Control Delay (s)	-	11.9	-	-
HCM Lane LOS	-	B	-	-
HCM 95th %tile Q(veh)	-	0.1	-	-

Intersection

Int Delay, s/veh 0

Movement	EBL	EBR	NBL	NBT	SBT	SBR
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Lane Configurations						
Traffic Vol, veh/h	0	4	0	868	824	12
Future Vol, veh/h	0	4	0	868	824	12
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	4	2	-
Peak Hour Factor	94	94	94	94	94	94
Heavy Vehicles, %	0	0	0	17	14	17
Mvmt Flow	0	4	0	923	877	13

Major/Minor	Minor2	Major1	Major2
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Conflicting Flow All	-	445	-	0	-	0
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.9	-	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.3	-	-	-	-
Pot Cap-1 Maneuver	0	566	0	-	-	-
Stage 1	0	-	0	-	-	-
Stage 2	0	-	0	-	-	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	-	566	-	-	-	-
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-

Approach	EB	NB	SB
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HCM Control Delay, s	11.4	0	0
HCM LOS	B		

Minor Lane/Major Mvmt	NBT	EBLn1	SBT	SBR
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Capacity (veh/h)	-	566	-	-
HCM Lane V/C Ratio	-	0.008	-	-
HCM Control Delay (s)	-	11.4	-	-
HCM Lane LOS	-	B	-	-
HCM 95th %tile Q(veh)	-	0	-	-

Intersection

Int Delay, s/veh 0.1

Movement	EBL	EBC	NBL	NBT	SBT	SBR
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Lane Configurations



Traffic Vol, veh/h 0 13 0 1021 1135 5

Future Vol, veh/h 0 13 0 1021 1135 5

Conflicting Peds, #/hr 0 0 0 0 0 0

Sign Control Stop Stop Free Free Free Free

RT Channelized - None - None - None

Storage Length - 0 - - - -

Veh in Median Storage, # 0 - - 0 0 -

Grade, % 0 - - 4 2 -

Peak Hour Factor 96 96 96 96 96 96

Heavy Vehicles, % 0 0 0 10 6 20

Mvmt Flow 0 14 0 1064 1182 5

Major/Minor	Minor2	Major1	Major2
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Conflicting Flow All - 594 - 0 - 0

Stage 1 - - - - - -

Stage 2 - - - - - -

Critical Hdwy - 6.9 - - - -

Critical Hdwy Stg 1 - - - - - -

Critical Hdwy Stg 2 - - - - - -

Follow-up Hdwy - 3.3 - - - -

Pot Cap-1 Maneuver 0 453 0 - - -

Stage 1 0 - 0 - - -

Stage 2 0 - 0 - - -

Platoon blocked, % - - - - - -

Mov Cap-1 Maneuver - 453 - - - -

Mov Cap-2 Maneuver - - - - - -

Stage 1 - - - - - -

Stage 2 - - - - - -

Approach	EB	NB	SB
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HCM Control Delay, s 13.2 0 0

HCM LOS B

Minor Lane/Major Mvmt	NBT	EBLn1	SBT	SBR
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Capacity (veh/h) - 453 - -

HCM Lane V/C Ratio - 0.03 - -

HCM Control Delay (s) - 13.2 - -

HCM Lane LOS - B - -

HCM 95th %tile Q(veh) - 0.1 - -

Intersection

Int Delay, s/veh 0.2

Movement	EBL	EBT	WBT	WBR	SBL	SBR
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Lane Configurations						
Traffic Vol, veh/h	3	223	236	20	5	1
Future Vol, veh/h	3	223	236	20	5	1
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	-8	6	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	16	12	0	0	0
Mvmt Flow	3	242	257	22	5	1

Major/Minor	Major1	Major2	Minor2
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Conflicting Flow All	279	0	-	0	516	268
Stage 1	-	-	-	-	268	-
Stage 2	-	-	-	-	248	-
Critical Hdwy	4.1	-	-	-	6.4	6.2
Critical Hdwy Stg 1	-	-	-	-	5.4	-
Critical Hdwy Stg 2	-	-	-	-	5.4	-
Follow-up Hdwy	2.2	-	-	-	3.5	3.3
Pot Cap-1 Maneuver	1295	-	-	-	523	776
Stage 1	-	-	-	-	782	-
Stage 2	-	-	-	-	798	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	1295	-	-	-	521	776
Mov Cap-2 Maneuver	-	-	-	-	521	-
Stage 1	-	-	-	-	780	-
Stage 2	-	-	-	-	798	-

Approach	EB	WB	SB
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HCM Control Delay, s	0.1	0	11.6
HCM LOS		B	

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	1295	-	-	-	551
HCM Lane V/C Ratio	0.003	-	-	-	0.012
HCM Control Delay (s)	7.8	0	-	-	11.6
HCM Lane LOS	A	A	-	-	B
HCM 95th %tile Q(veh)	0	-	-	-	0

Intersection

Int Delay, s/veh 0.6

Movement	EBL	EBT	WBT	WBR	SBL	SBR
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Lane Configurations						
Traffic Vol, veh/h	1	182	249	7	19	3
Future Vol, veh/h	1	182	249	7	19	3
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	-8	6	-	0	-
Peak Hour Factor	86	86	86	86	86	86
Heavy Vehicles, %	0	11	3	0	11	0
Mvmt Flow	1	212	290	8	22	3

Major/Minor	Major1	Major2	Minor2
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Conflicting Flow All	298	0	-	0	508	294
Stage 1	-	-	-	-	294	-
Stage 2	-	-	-	-	214	-
Critical Hdwy	4.1	-	-	-	6.51	6.2
Critical Hdwy Stg 1	-	-	-	-	5.51	-
Critical Hdwy Stg 2	-	-	-	-	5.51	-
Follow-up Hdwy	2.2	-	-	-	3.599	3.3
Pot Cap-1 Maneuver	1275	-	-	-	509	750
Stage 1	-	-	-	-	736	-
Stage 2	-	-	-	-	801	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	1275	-	-	-	508	750
Mov Cap-2 Maneuver	-	-	-	-	508	-
Stage 1	-	-	-	-	735	-
Stage 2	-	-	-	-	801	-

Approach	EB	WB	SB
----------	----	----	----

HCM Control Delay, s	0	0	12.1
HCM LOS			B

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	1275	-	-	-	531
HCM Lane V/C Ratio	0.001	-	-	-	0.048
HCM Control Delay (s)	7.8	0	-	-	12.1
HCM Lane LOS	A	A	-	-	B
HCM 95th %tile Q(veh)	0	-	-	-	0.2

Intersection

Int Delay, s/veh 0.2

Movement	EBL	EBT	WBT	WBR	SBL	SBR
----------	-----	-----	-----	-----	-----	-----

Lane Configurations						
Traffic Vol, veh/h	3	272	288	20	5	1
Future Vol, veh/h	3	272	288	20	5	1
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	-8	6	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	16	12	0	0	0
Mvmt Flow	3	296	313	22	5	1

Major/Minor	Major1	Major2	Minor2
-------------	--------	--------	--------

Conflicting Flow All	335	0	-	0	626	324
Stage 1	-	-	-	-	324	-
Stage 2	-	-	-	-	302	-
Critical Hdwy	4.1	-	-	-	6.4	6.2
Critical Hdwy Stg 1	-	-	-	-	5.4	-
Critical Hdwy Stg 2	-	-	-	-	5.4	-
Follow-up Hdwy	2.2	-	-	-	3.5	3.3
Pot Cap-1 Maneuver	1236	-	-	-	451	722
Stage 1	-	-	-	-	738	-
Stage 2	-	-	-	-	755	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	1236	-	-	-	450	722
Mov Cap-2 Maneuver	-	-	-	-	450	-
Stage 1	-	-	-	-	736	-
Stage 2	-	-	-	-	755	-

Approach	EB	WB	SB
----------	----	----	----

HCM Control Delay, s	0.1	0	12.6
HCM LOS		B	

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	1236	-	-	-	480
HCM Lane V/C Ratio	0.003	-	-	-	0.014
HCM Control Delay (s)	7.9	0	-	-	12.6
HCM Lane LOS	A	A	-	-	B
HCM 95th %tile Q(veh)	0	-	-	-	0

Intersection

Int Delay, s/veh 0.5

Movement	EBL	EBT	WBT	WBR	SBL	SBR
----------	-----	-----	-----	-----	-----	-----

Lane Configurations						
Traffic Vol, veh/h	1	222	304	7	19	3
Future Vol, veh/h	1	222	304	7	19	3
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	-8	6	-	0	-
Peak Hour Factor	86	86	86	86	86	86
Heavy Vehicles, %	0	11	3	0	11	0
Mvmt Flow	1	258	353	8	22	3

Major/Minor	Major1	Major2	Minor2
-------------	--------	--------	--------

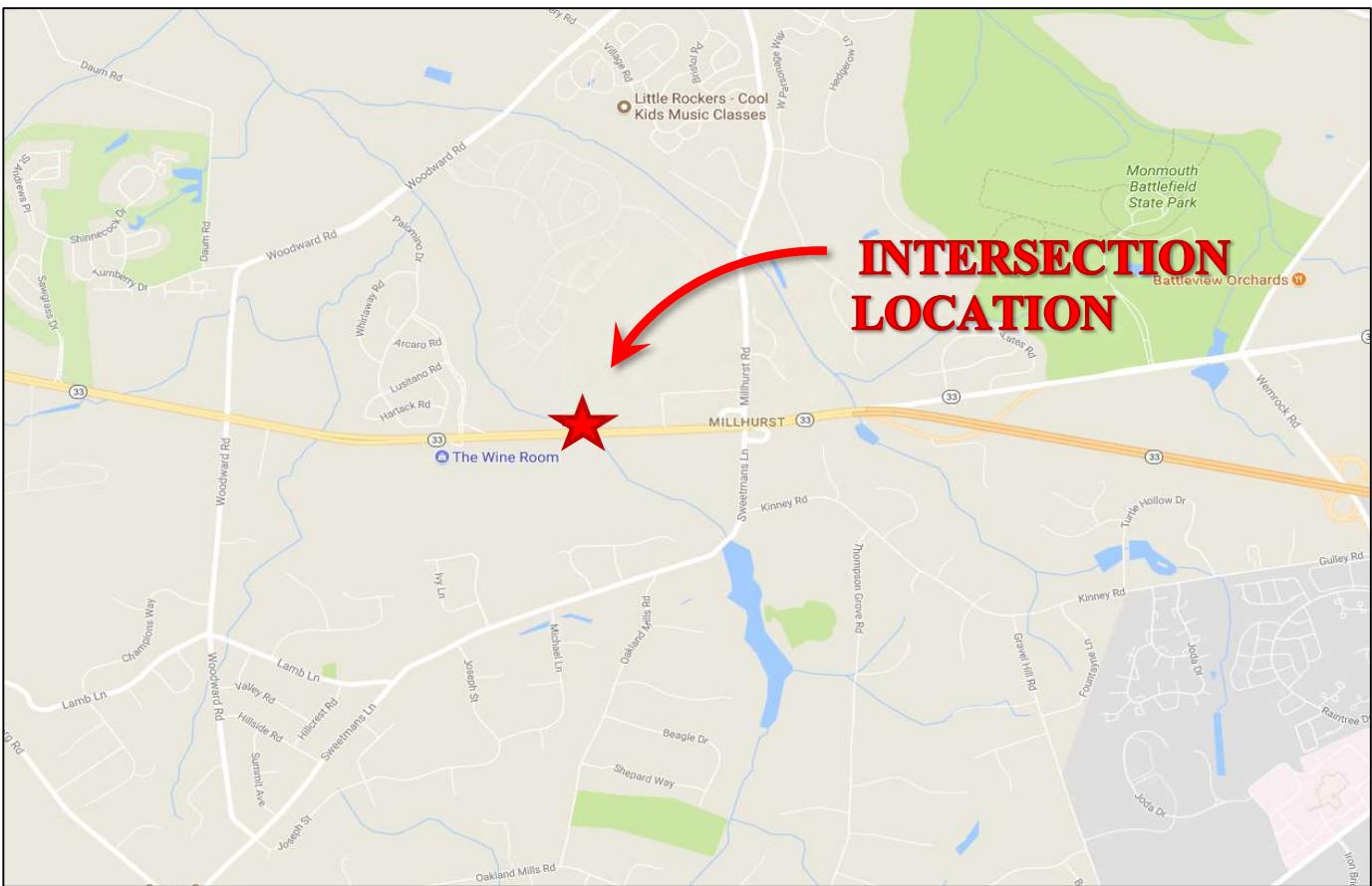
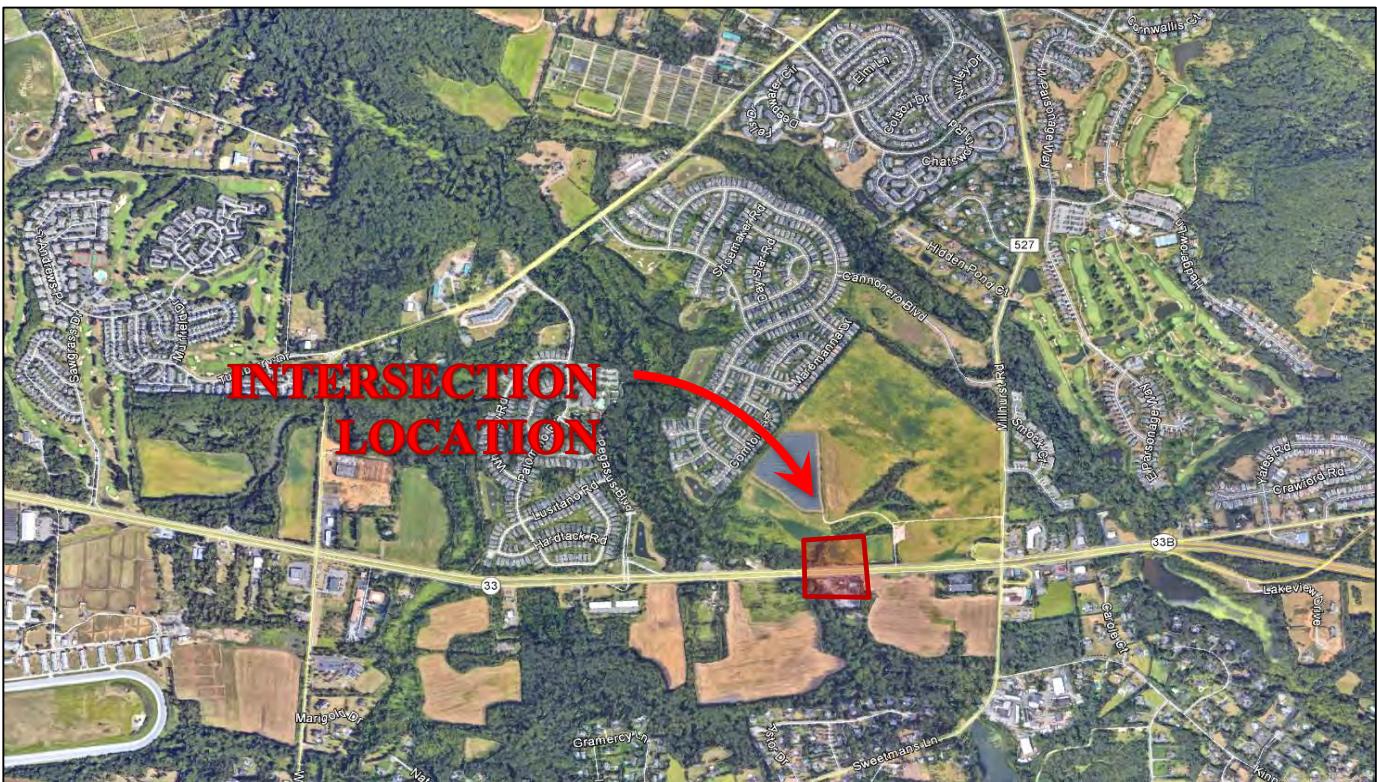
Conflicting Flow All	361	0	-	0	617	357
Stage 1	-	-	-	-	357	-
Stage 2	-	-	-	-	260	-
Critical Hdwy	4.1	-	-	-	6.51	6.2
Critical Hdwy Stg 1	-	-	-	-	5.51	-
Critical Hdwy Stg 2	-	-	-	-	5.51	-
Follow-up Hdwy	2.2	-	-	-	3.599	3.3
Pot Cap-1 Maneuver	1209	-	-	-	439	692
Stage 1	-	-	-	-	689	-
Stage 2	-	-	-	-	763	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	1209	-	-	-	439	692
Mov Cap-2 Maneuver	-	-	-	-	439	-
Stage 1	-	-	-	-	688	-
Stage 2	-	-	-	-	763	-

Approach	EB	WB	SB
----------	----	----	----

HCM Control Delay, s	0	0	13.2
HCM LOS			B

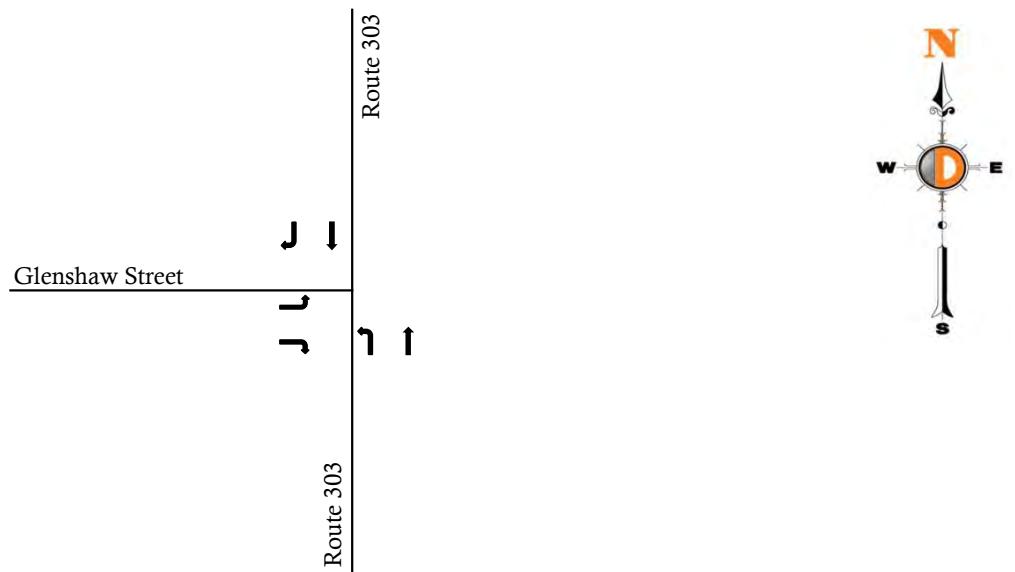
Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	1209	-	-	-	462
HCM Lane V/C Ratio	0.001	-	-	-	0.055
HCM Control Delay (s)	8	0	-	-	13.2
HCM Lane LOS	A	A	-	-	B
HCM 95th %tile Q(veh)	0	-	-	-	0.2

Appendix D
Traffic Signal Warrant Analysis



Route 303 & Glenshaw Street
Signal Warrant Analysis
2686-99-013T

Intersection Location Map



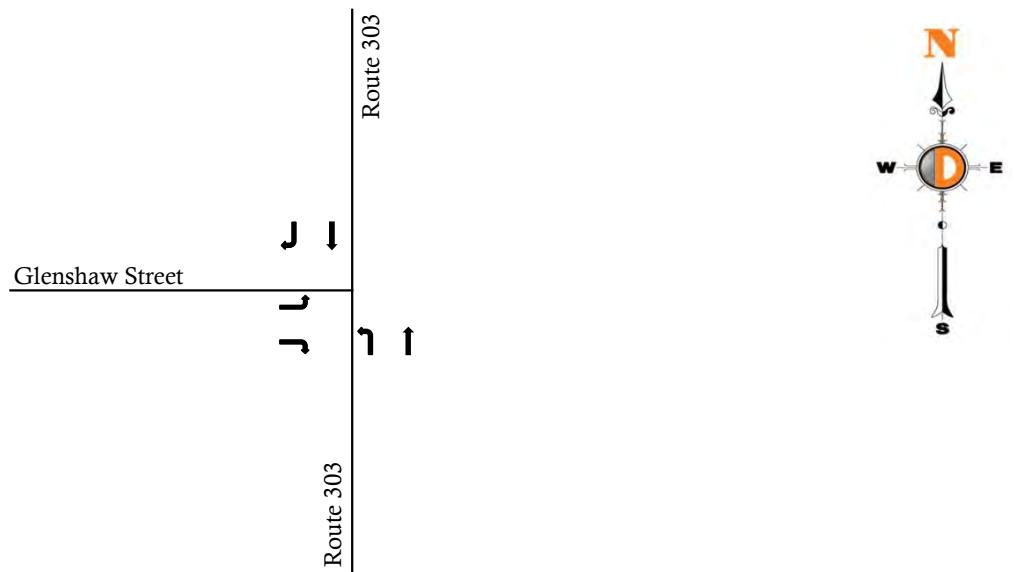
Weekday	Glenshaw Street			Route 303					
	Eastbound			Northbound			Southbound		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
	8	0	8	37	478	0	0	465	38
8:00 AM	8	0	8	37	478	0	0	465	38
9:00 AM	23	0	22	43	479	0	0	561	43
5:00 PM	30	0	32	9	696	0	0	652	18
6:00 PM	32	0	74	9	573	0	0	714	6



Route 303 & Glenshaw Street
Signal Warrant Analysis
2686-99-013T

Figure 1

Existing Traffic Volumes



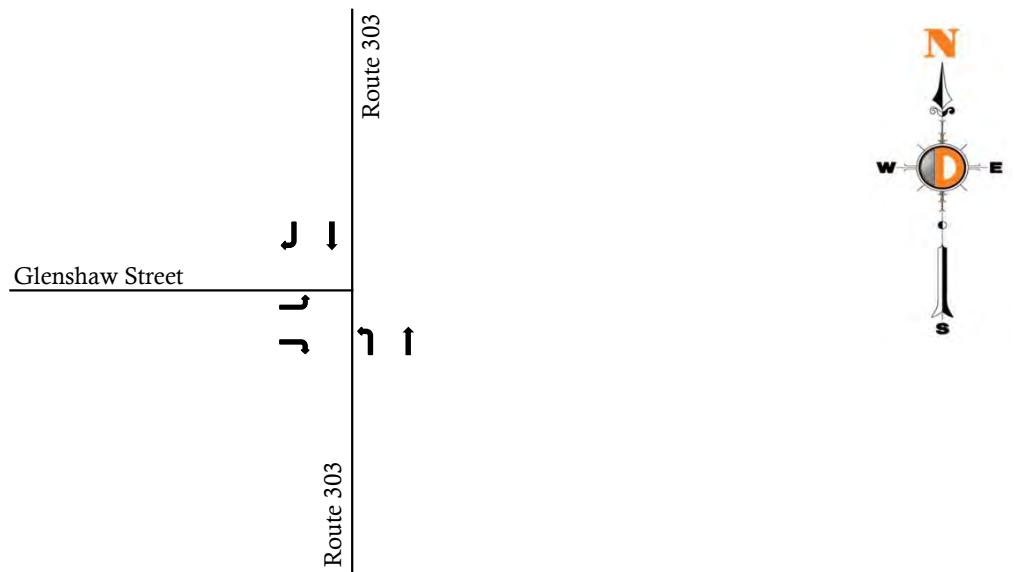
Weekday	Glenshaw Street			Route 303					
	Eastbound			Northbound			Southbound		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
	8	0	8	38	488	0	0	474	39
8:00 AM	8	0	8	38	488	0	0	474	39
9:00 AM	23	0	22	44	489	0	0	572	44
5:00 PM	31	0	33	9	710	0	0	665	18
6:00 PM	33	0	75	9	584	0	0	728	6



Route 303 & Glenshaw Street
Signal Warrant Analysis
2686-99-013T

Figure 3

No Build Traffic Volumes



Weekday	Glenshaw Street			Route 303					
	Eastbound			Northbound			Southbound		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
	10	0	10	46	595	0	0	578	48
8:00 AM	10	0	10	46	595	0	0	578	48
9:00 AM	28	0	27	54	597	0	0	698	54
5:00 PM	38	0	40	11	866	0	0	811	22
6:00 PM	40	0	92	11	713	0	0	888	7



Route 303 & Glenshaw Street
Signal Warrant Analysis
2686-99-013T

Figure 4

No Build Traffic Volumes

Trip Generation				
Land Use	Trip Type	Trips		
		In	Out	Total
175,760 SF Warehouse	Cars	116	115	231

Weekday	Temp. Dist	Total Trips	In-Out %		Car Trips	
			In	Out	In	Out
8:00 AM	3.9%	9	87.3%	12.7%	8	1
9:00 AM	7.2%	17	71.9%	28.1%	12	5
5:00 PM	5.0%	12	13.2%	86.8%	2	10
6:00 PM	5.7%	13	20.3%	79.7%	3	10



Trip Distribution

Glenshaw Street			Route 303						
Eastbound			Northbound			Southbound			
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Cars	IN	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	30.0%	0.0%
	OUT	0.0%	0.0%	0.0%	0.0%	30.0%	0.0%	0.0%	0.0%

Weekday	Glenshaw Street			Route 303					
	Eastbound			Northbound			Southbound		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
8:00 AM	0	0	0	0	0	0	0	2	0
9:00 AM	0	0	0	0	1	0	0	4	0
5:00 PM	0	0	0	0	3	0	0	1	0
6:00 PM	0	0	0	0	3	0	0	1	0



Trip Generation				
Land Use	Trip Type	Trips		
		In	Out	Total
175,760 SF Warehouse	Trucks	43	42	85

Weekday	Temp. Dist	Total Trips	In-Out %		Truck Trips	
			In	Out	In	Out
8:00 AM	5.5%	5	34.4%	65.6%	2	3
9:00 AM	9.9%	8	49.2%	50.8%	4	4
5:00 PM	3.9%	3	36.6%	63.4%	1	2
6:00 PM	0.0%	0	50.0%	50.0%	0	0



Trip Distribution

		Glenshaw Street			Route 303					
		Eastbound			Northbound			Southbound		
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Trucks	IN	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	100.0%	0.0%
	OUT	0.0%	0.0%	0.0%	0.0%	100.0%	0.0%	0.0%	0.0%	0.0%

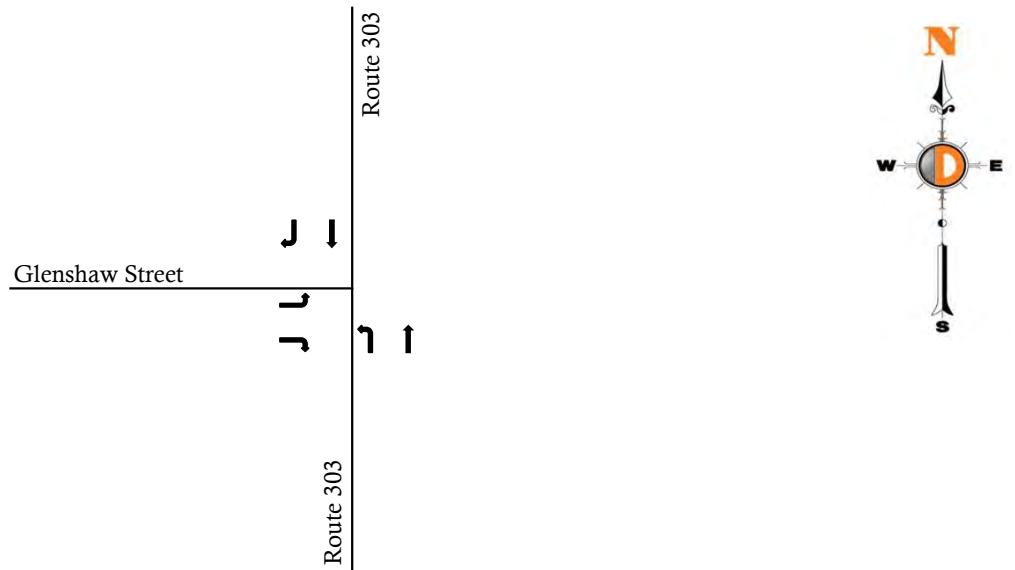
Weekday	Glenshaw Street			Route 303					
	Eastbound			Northbound			Southbound		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
8:00 AM	0	0	0	0	3	0	0	2	0
9:00 AM	0	0	0	0	4	0	0	4	0
5:00 PM	0	0	0	0	2	0	0	1	0
6:00 PM	0	0	0	0	0	0	0	0	0



Route 303 & Glenshaw Street
Signal Warrant Analysis
2686-99-013T

Figure 8

Truck Site Generated Volumes



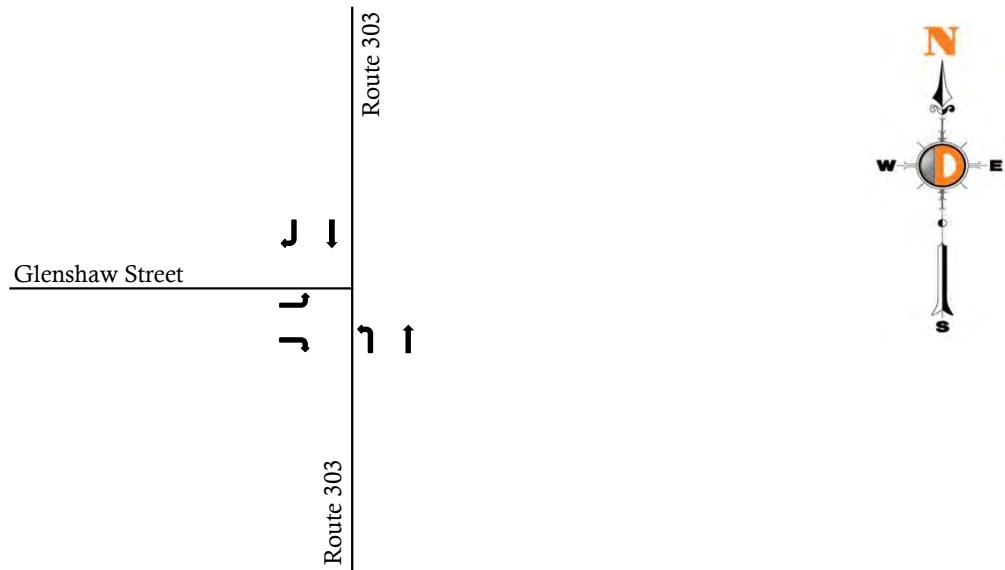
Weekday	Glenshaw Street			Route 303					
	Eastbound			Northbound			Southbound		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
	0	0	0	0	3	0	0	4	0
8:00 AM	0	0	0	0	5	0	0	8	0
9:00 AM	0	0	0	0	5	0	0	2	0
5:00 PM	0	0	0	0	3	0	0	1	0
6:00 PM	0	0	0	0	3	0	0	1	0



Route 303 & Glenshaw Street
Signal Warrant Analysis
2686-99-013T

Figure 9

Total Site Generated Volumes



Weekday	Glenshaw Street			Route 303					
	Eastbound			Northbound			Southbound		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
	10	0	10	46	598	0	0	582	48
8:00 AM	10	0	10	46	598	0	0	582	48
9:00 AM	28	0	27	54	602	0	0	706	54
5:00 PM	38	0	40	11	871	0	0	813	22
6:00 PM	40	0	92	11	716	0	0	889	7

Route 303 & Glenshaw Street
Signal Warrant Analysis
2686-99-013T

Figure 10

Design Horizon Year Build Traffic Volumes

Traffic Signal Warrant Analysis

Intersection: Route 303 & Glenshaw Street
 Location: Orangetown Town, Rockland County, NJ
 Prepared By: A. Ferrante
 Date: 10/3/2022
 Job #: 2686-99-013T



Time	Weekday Volumes			
	Major Road		Minor Road	
	NB	SB	EB	O
12:00 AM	0	0	0	0
1:00 AM	0	0	0	0
2:00 AM	0	0	0	0
3:00 AM	0	0	0	0
4:00 AM	0	0	0	0
5:00 AM	0	0	0	0
6:00 AM	0	0	0	0
7:00 AM	0	0	0	0
8:00 AM	644	630	20	0
9:00 AM	656	760	55	0
10:00 AM	0	0	0	0
11:00 AM	0	0	0	0
12:00 PM	0	0	0	0
1:00 PM	0	0	0	0
2:00 PM	0	0	0	0
3:00 PM	0	0	0	0
4:00 PM	0	0	0	0
5:00 PM	882	835	78	0
6:00 PM	727	896	132	0
7:00 PM	0	0	0	0
8:00 PM	0	0	0	0
9:00 PM	0	0	0	0
10:00 PM	0	0	0	0
11:00 PM	0	0	0	0
Lanes	2	2	1	0
Speed	40	40	30	0

Warrant % Criteria	Warrant Summary			
	1A 100	1B 100	2 100	3 100
	NO	NO	NO	NO
	NO	NO	NO	NO
	NO	NO	NO	NO
	NO	NO	NO	NO
	NO	NO	NO	NO
	NO	NO	NO	NO
	NO	NO	NO	NO
	NO	NO	NO	NO
	NO	NO	NO	NO
	NO	NO	NO	NO
	NO	NO	NO	NO
	NO	NO	NO	NO
	NO	NO	NO	NO
	NO	NO	NO	NO
	NO	NO	NO	NO
	NO	NO	NO	NO
	NO	NO	NO	NO
	NO	NO	NO	NO
	NO	YES	NO	NO
	NO	YES	YES	YES
	NO	NO	NO	NO
	NO	NO	NO	NO
	NO	NO	NO	NO
	NO	NO	NO	NO
	NO	NO	NO	NO
	NO	NO	NO	NO
	Hours Met	0	2	1
	Satisfied?	NO	NO	NO
				YES

Based upon the Traffic Signal Warrants described in Chapter 4C of the Manual on Uniform Traffic Control Devices (MUTCD), 2009 Edition, published by the Federal Highway Administration (FHWA).

Traffic Signal Warrant Analysis

Intersection: Route 303 & Glenshaw Street
 Location: Orangetown Town, Rockland County, NJ
 Prepared By: J. Pesce
 Date: 10/3/2022
 Job #: 2686-99-013T



Warrant 1, Eight-Hour Vehicular Volume (Conditions A and B)

Condition A—Minimum Vehicular Volume

Number of lanes for moving traffic on each approach		Vehicles per hour on major street (total of both approaches)				Vehicles per hour on higher-volume minor-street approach (one direction only)			
Major Street	Minor Street	100% ^a	80% ^b	70% ^c	56% ^d	100% ^a	80% ^b	70% ^c	56% ^d
1	1	500	400	350	280	150	120	105	84
2 or more	1	600	480	420	336	150	120	105	84
2 or more	2 or more	600	480	420	336	200	160	140	112
1	2 or more	500	400	350	280	200	160	140	112

Condition B—Interruption of Continuous Traffic

Number of lanes for moving traffic on each approach		Vehicles per hour on major street (total of both approaches)				Vehicles per hour on higher-volume minor-street approach (one direction only)			
Major Street	Minor Street	100% ^a	80% ^b	70% ^c	56% ^d	100% ^a	80% ^b	70% ^c	56% ^d
1	1	750	600	525	420	75	60	53	42
2 or more	1	900	720	630	504	75	60	53	42
2 or more	2 or more	900	720	630	504	100	80	70	56
1	2 or more	750	600	525	420	100	80	70	56

^a Basic minimum hourly volume

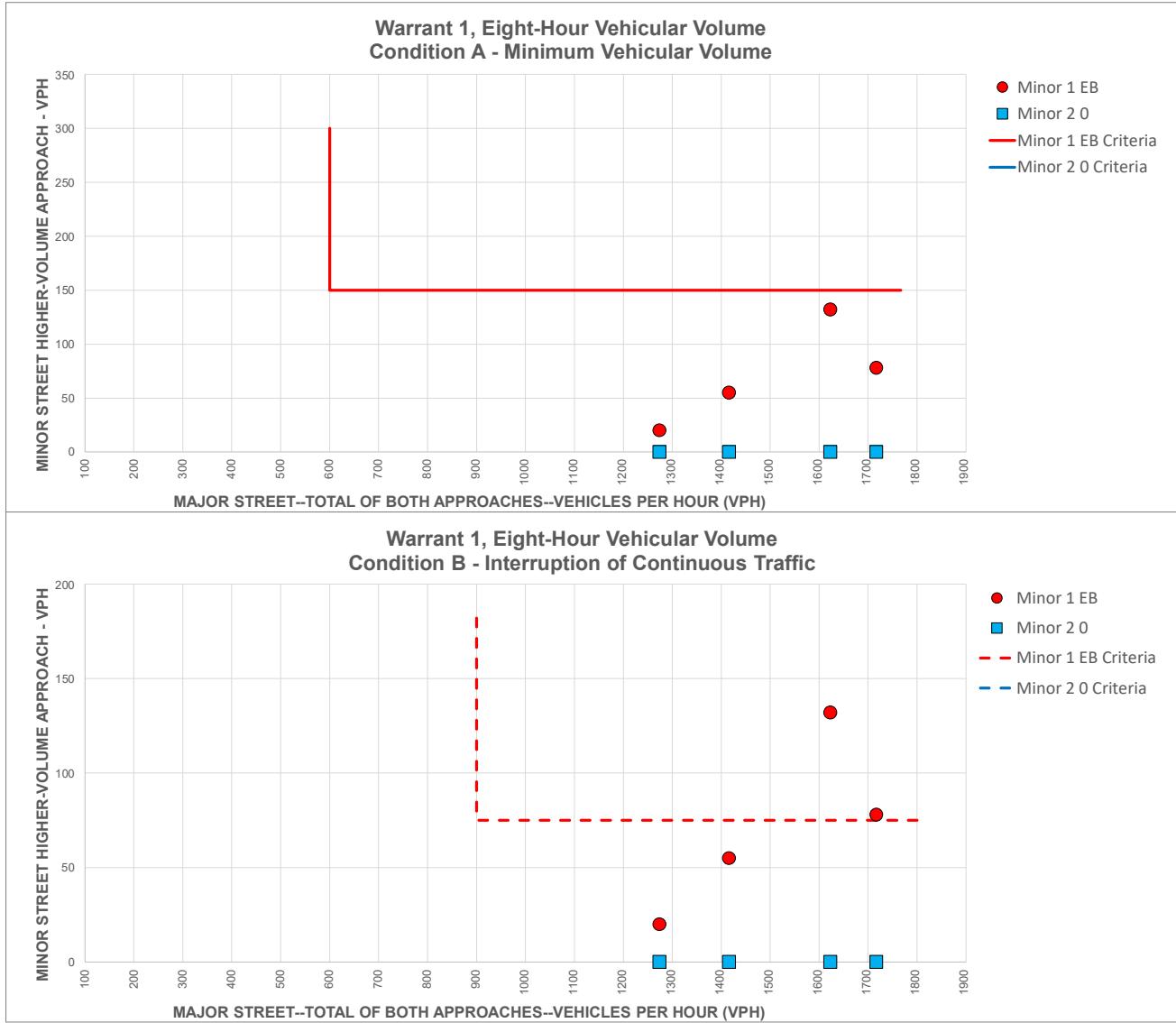
^b Used for combination of Conditions A and B after adequate trial of other remedial measures

^c May be used when the major-street speed exceeds 40 mph or in an isolated community with a population of less than 10,000

^d May be used for combination of Conditions A and B after adequate trial of other remedial measures when the major-street speed exceeds 40 mph or in an isolated community with a population of less than 10,000

Volumes			Condition A Criteria						WARRANT SATISFIED ?	Condition B Criteria						WARRANT SATISFIED ?	
Time	Major TOTAL	Minor EB	Minor 0	Volume %			100				Major	Minor 1	Satisfied?	Major	Minor 2	Satisfied?	
				Major	Minor 1	Satisfied?	Major	Minor 2	Satisfied?								
12:00 AM	0	0	0	600	150	No	-	-	No	NO	900	75	No	-	-	No	NO
1:00 AM	0	0	0	600	150	No	-	-	No	NO	900	75	No	-	-	No	NO
2:00 AM	0	0	0	600	150	No	-	-	No	NO	900	75	No	-	-	No	NO
3:00 AM	0	0	0	600	150	No	-	-	No	NO	900	75	No	-	-	No	NO
4:00 AM	0	0	0	600	150	No	-	-	No	NO	900	75	No	-	-	No	NO
5:00 AM	0	0	0	600	150	No	-	-	No	NO	900	75	No	-	-	No	NO
6:00 AM	0	0	0	600	150	No	-	-	No	NO	900	75	No	-	-	No	NO
7:00 AM	0	0	0	600	150	No	-	-	No	NO	900	75	No	-	-	No	NO
8:00 AM	1274	20	0	600	150	No	-	-	No	NO	900	75	No	-	-	No	NO
9:00 AM	1416	55	0	600	150	No	-	-	No	NO	900	75	No	-	-	No	NO
10:00 AM	0	0	0	600	150	No	-	-	No	NO	900	75	No	-	-	No	NO
11:00 AM	0	0	0	600	150	No	-	-	No	NO	900	75	No	-	-	No	NO
12:00 PM	0	0	0	600	150	No	-	-	No	NO	900	75	No	-	-	No	NO
1:00 PM	0	0	0	600	150	No	-	-	No	NO	900	75	No	-	-	No	NO
2:00 PM	0	0	0	600	150	No	-	-	No	NO	900	75	No	-	-	No	NO
3:00 PM	0	0	0	600	150	No	-	-	No	NO	900	75	No	-	-	No	NO
4:00 PM	0	0	0	600	150	No	-	-	No	NO	900	75	No	-	-	No	NO
5:00 PM	1717	78	0	600	150	No	-	-	No	NO	900	75	Yes	-	-	No	YES
6:00 PM	1623	132	0	600	150	No	-	-	No	NO	900	75	Yes	-	-	No	YES
7:00 PM	0	0	0	600	150	No	-	-	No	NO	900	75	No	-	-	No	NO
8:00 PM	0	0	0	600	150	No	-	-	No	NO	900	75	No	-	-	No	NO
9:00 PM	0	0	0	600	150	No	-	-	No	NO	900	75	No	-	-	No	NO
10:00 PM	0	0	0	600	150	No	-	-	No	NO	900	75	No	-	-	No	NO
11:00 PM	0	0	0	600	150	No	-	-	No	NO	900	75	No	-	-	No	NO
Lanes	2	1	0							Hours Met	0					Hours Met	2
Speed	40	30	0							Satisfied?	NO					Satisfied?	NO

Warrant 1, Eight-Hour Vehicular Volume
 (Conditions A and B)

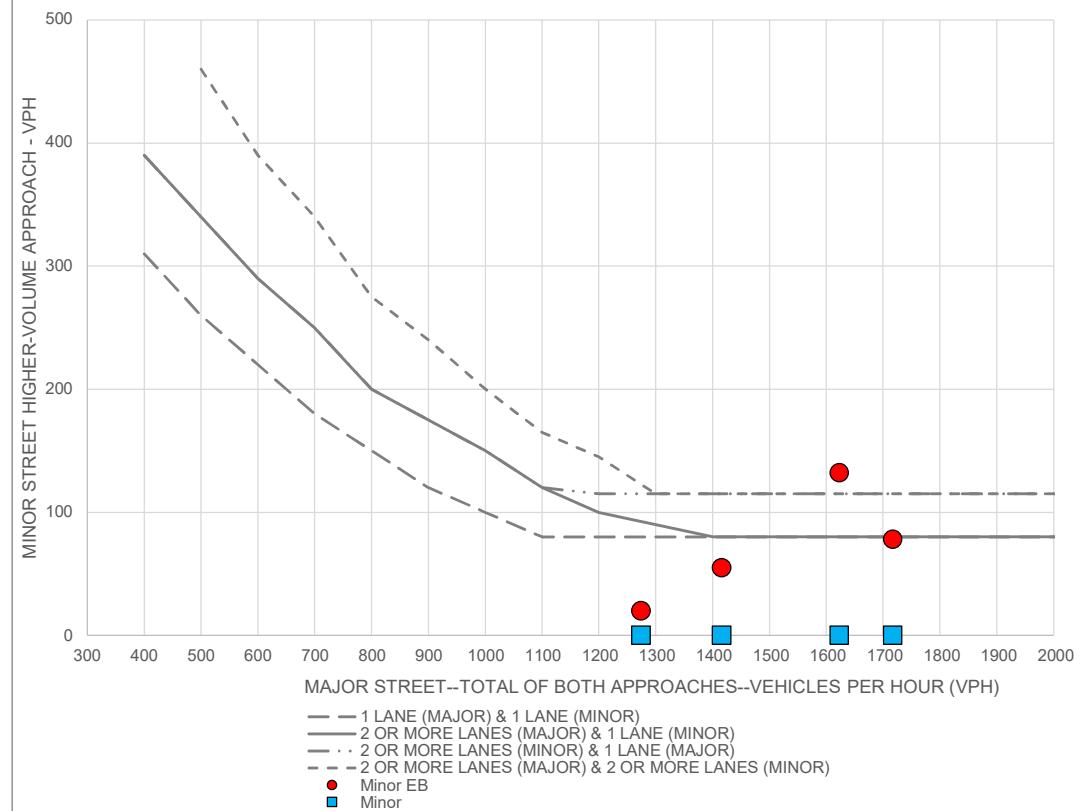


Warrant 2 - Four-Hour Vehicular Volume

(100% Thresholds)

Time	Volumes							WARRANT SATISFIED?
	Major (Total)	Minor EB	Threshold	Satisfied?	Minor	Threshold	Satisfied?	
12:00 AM	0	0	-	No	0	-	No	NO
1:00 AM	0	0	-	No	0	-	No	NO
2:00 AM	0	0	-	No	0	-	No	NO
3:00 AM	0	0	-	No	0	-	No	NO
4:00 AM	0	0	-	No	0	-	No	NO
5:00 AM	0	0	-	No	0	-	No	NO
6:00 AM	0	0	-	No	0	-	No	NO
7:00 AM	0	0	-	No	0	-	No	NO
8:00 AM	1274	20	93	No	0	93	No	NO
9:00 AM	1416	55	80	No	0	80	No	NO
10:00 AM	0	0	-	No	0	-	No	NO
11:00 AM	0	0	-	No	0	-	No	NO
12:00 PM	0	0	-	No	0	-	No	NO
1:00 PM	0	0	-	No	0	-	No	NO
2:00 PM	0	0	-	No	0	-	No	NO
3:00 PM	0	0	-	No	0	-	No	NO
4:00 PM	0	0	-	No	0	-	No	NO
5:00 PM	1717	78	80	No	0	80	No	NO
6:00 PM	1623	132	80	Yes	0	80	No	YES
7:00 PM	0	0	-	No	0	-	No	NO
8:00 PM	0	0	-	No	0	-	No	NO
9:00 PM	0	0	-	No	0	-	No	NO
10:00 PM	0	0	-	No	0	-	No	NO
11:00 PM	0	0	-	No	0	-	No	NO
Lanes	2	1			0			1
Speed	40	30			0		Satisfied?	No

Figure 4C-1. Warrant 2, Four-Hour Vehicular Volume

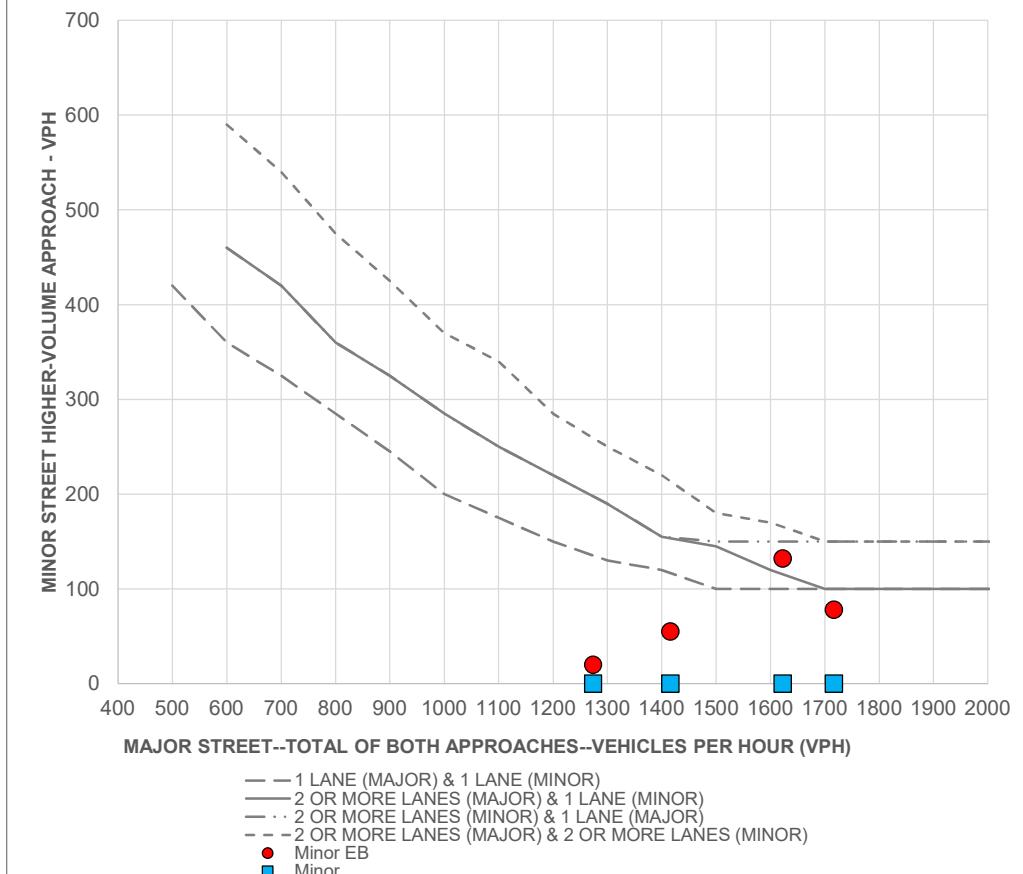


Warrant 3 - Peak Hour Vehicular Volume

(100% Thresholds)

Time	Volumes							WARRANT SATISFIED ?
	Major (Total)	Minor EB	Threshold	Satisfied?	Minor	Threshold	Satisfied?	
12:00 AM	0	0	-	No	0	-	No	NO
1:00 AM	0	0	-	No	0	-	No	NO
2:00 AM	0	0	-	No	0	-	No	NO
3:00 AM	0	0	-	No	0	-	No	NO
4:00 AM	0	0	-	No	0	-	No	NO
5:00 AM	0	0	-	No	0	-	No	NO
6:00 AM	0	0	-	No	0	-	No	NO
7:00 AM	0	0	-	No	0	-	No	NO
8:00 AM	1274	20	198	No	0	198	No	NO
9:00 AM	1416	55	153	No	0	153	No	NO
10:00 AM	0	0	-	No	0	-	No	NO
11:00 AM	0	0	-	No	0	-	No	NO
12:00 PM	0	0	-	No	0	-	No	NO
1:00 PM	0	0	-	No	0	-	No	NO
2:00 PM	0	0	-	No	0	-	No	NO
3:00 PM	0	0	-	No	0	-	No	NO
4:00 PM	0	0	-	No	0	-	No	NO
5:00 PM	1717	78	100	No	0	100	No	NO
6:00 PM	1623	132	115	Yes	0	115	No	YES
7:00 PM	0	0	-	No	0	-	No	NO
8:00 PM	0	0	-	No	0	-	No	NO
9:00 PM	0	0	-	No	0	-	No	NO
10:00 PM	0	0	-	No	0	-	No	NO
11:00 PM	0	0	-	No	0	-	No	NO
Lanes	2	1			0			1
Speed	40	30			0		Satisfied?	Yes

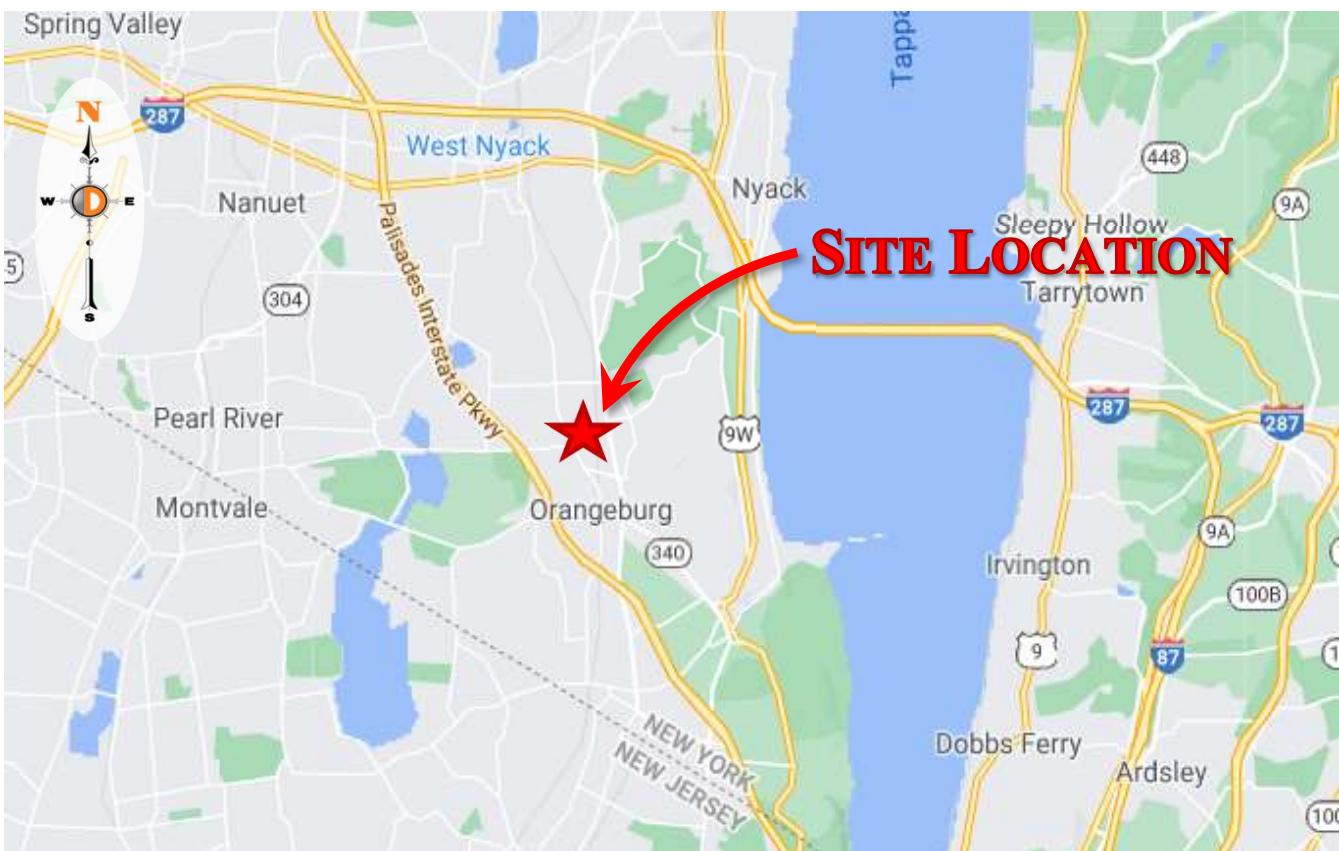
Figure 4C-3. Warrant 3, Peak Hour



Appendix E
Alternate Traffic Volume Figures



SITE LOCATION



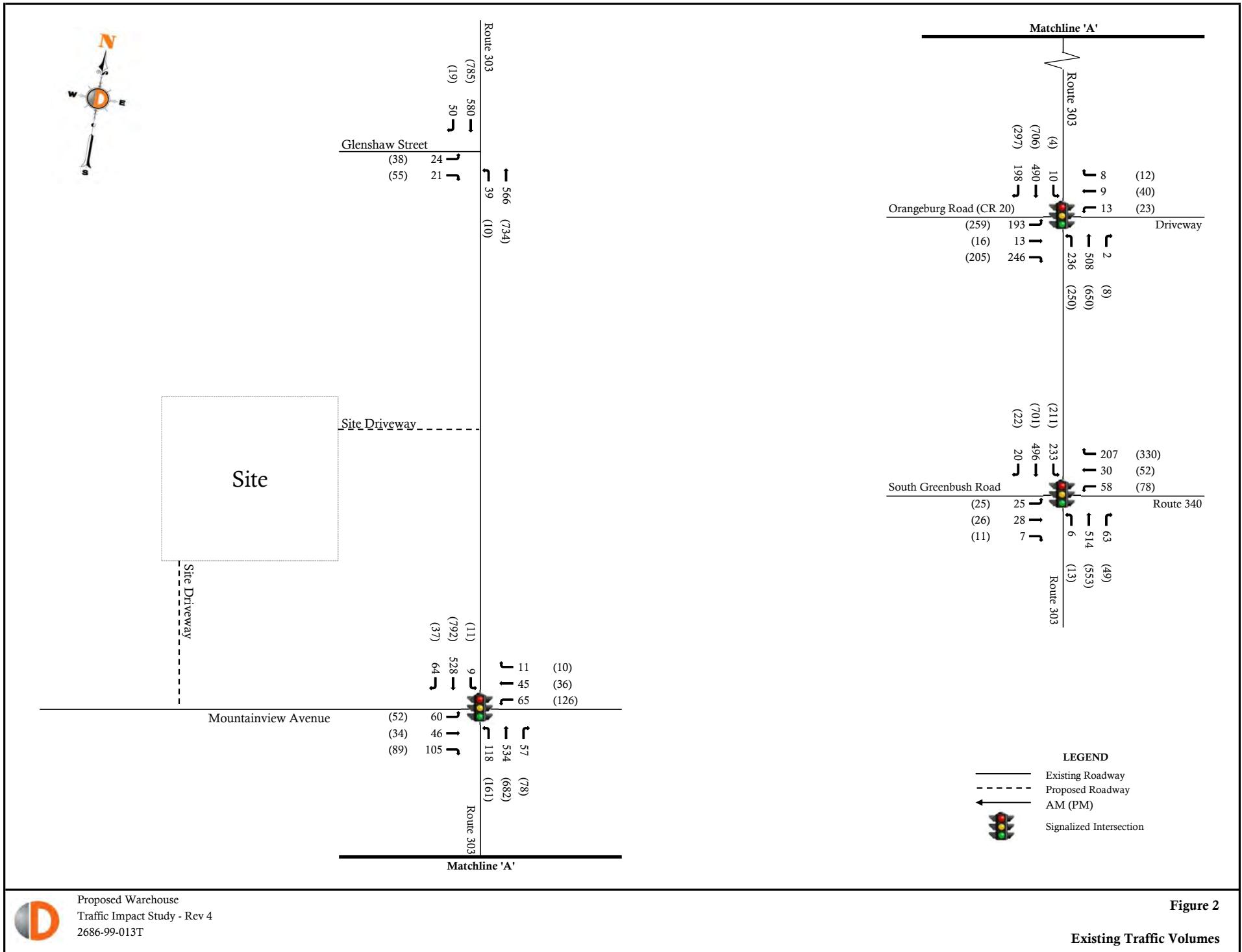
SITE LOCATION

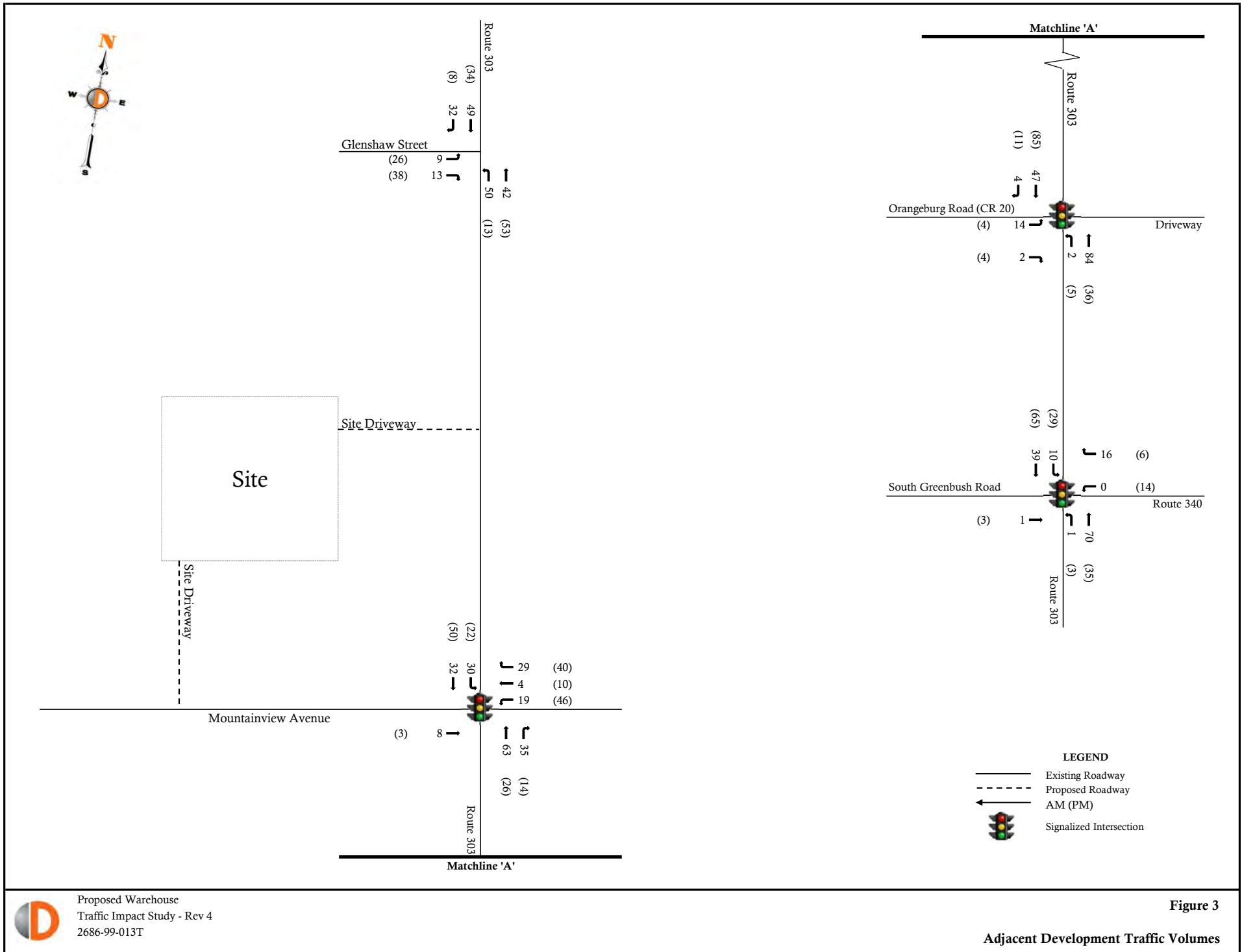


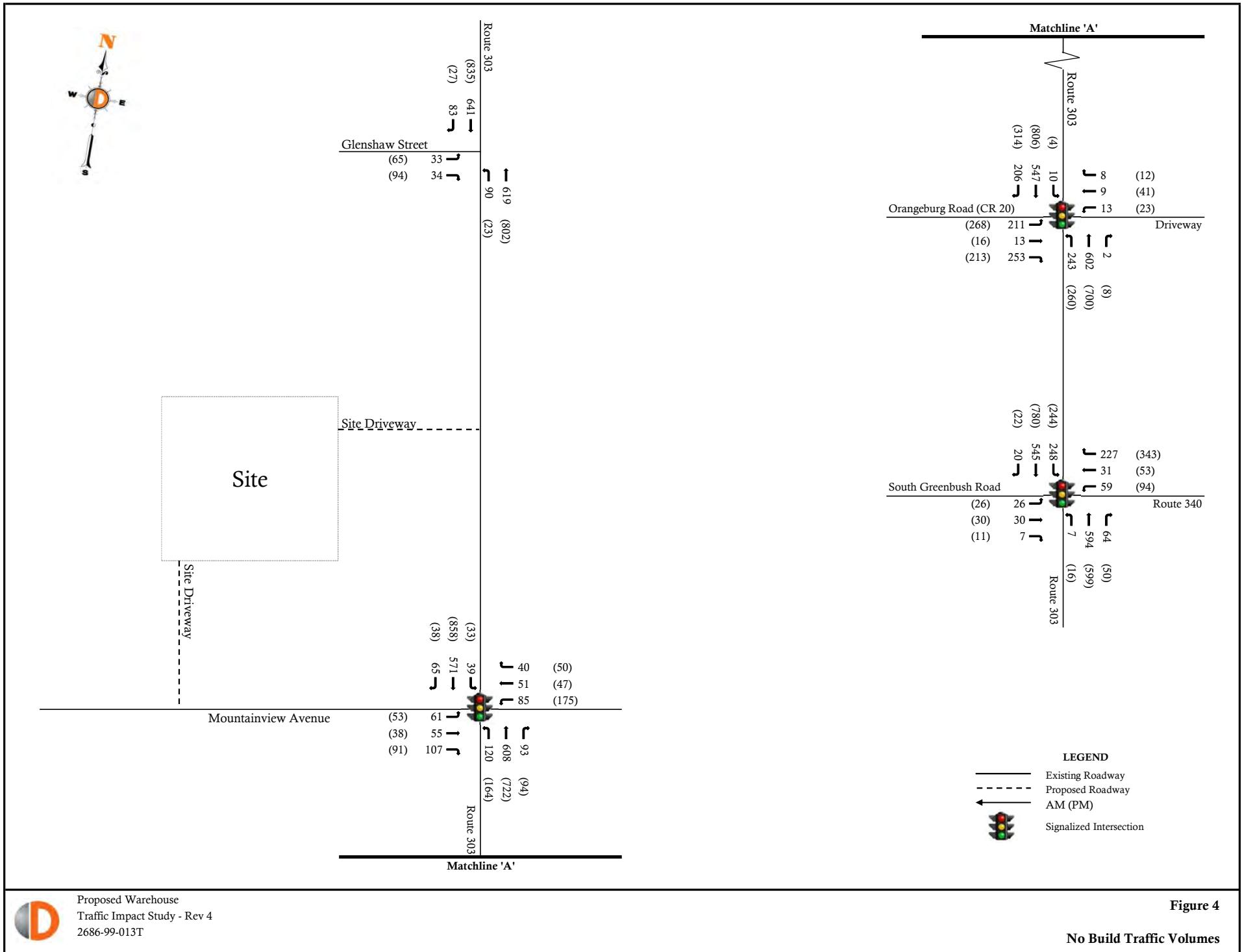
Proposed Warehouse
Traffic Impact Study - Rev 4
2686-99-013T

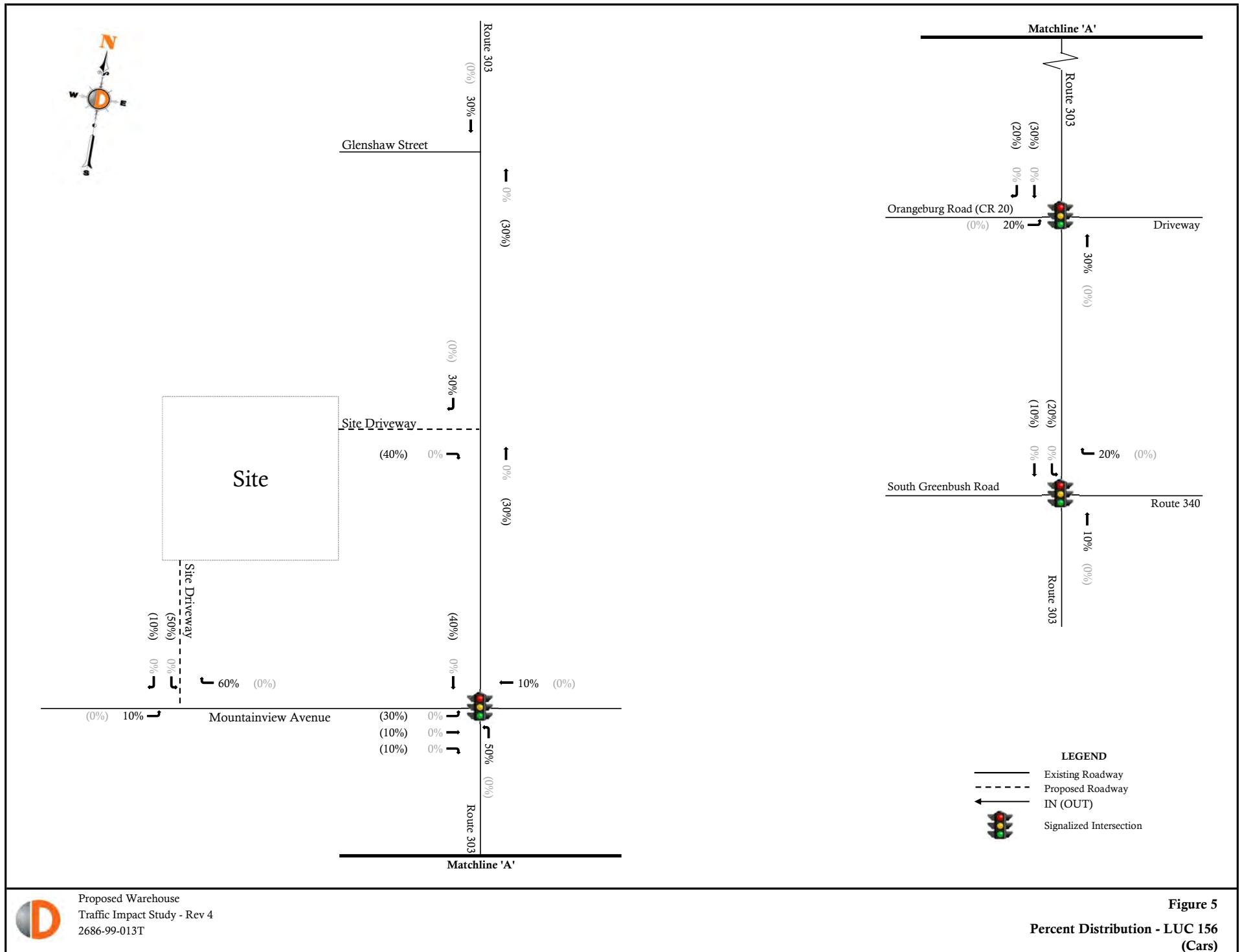
Figure 1

Site Location Map







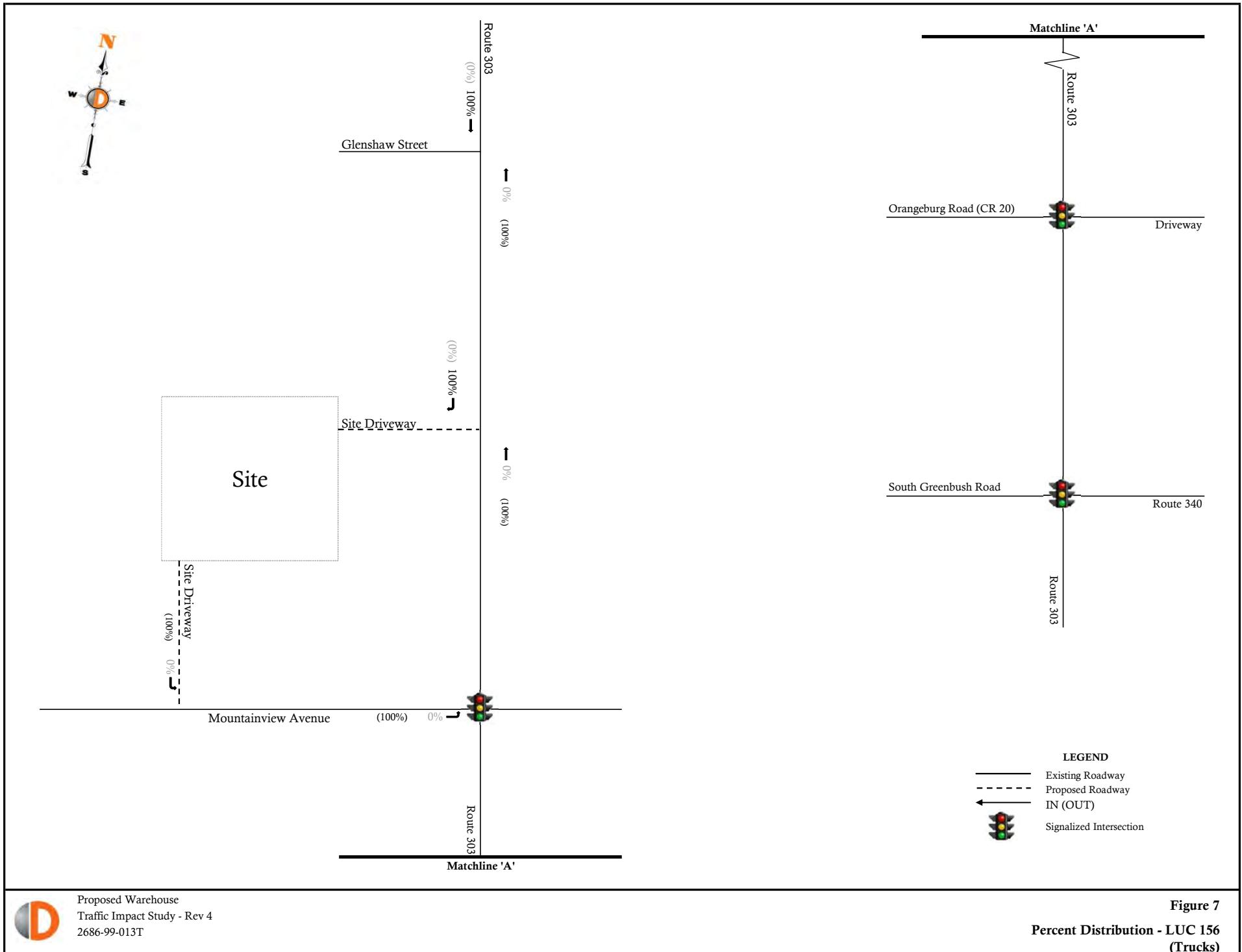


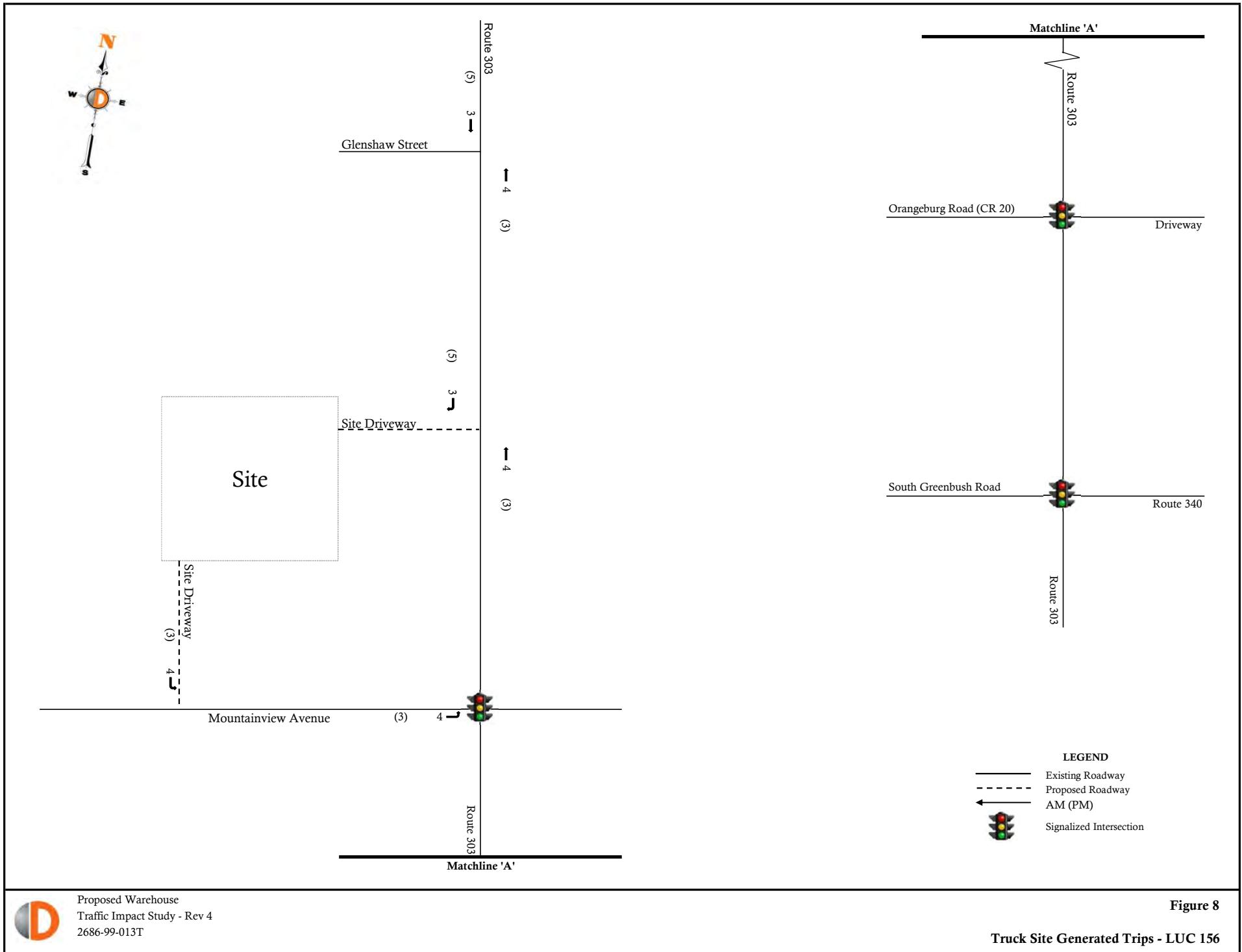
Proposed Warehouse
Traffic Impact Study - Rev 4
2686-99-013T

Figure 5

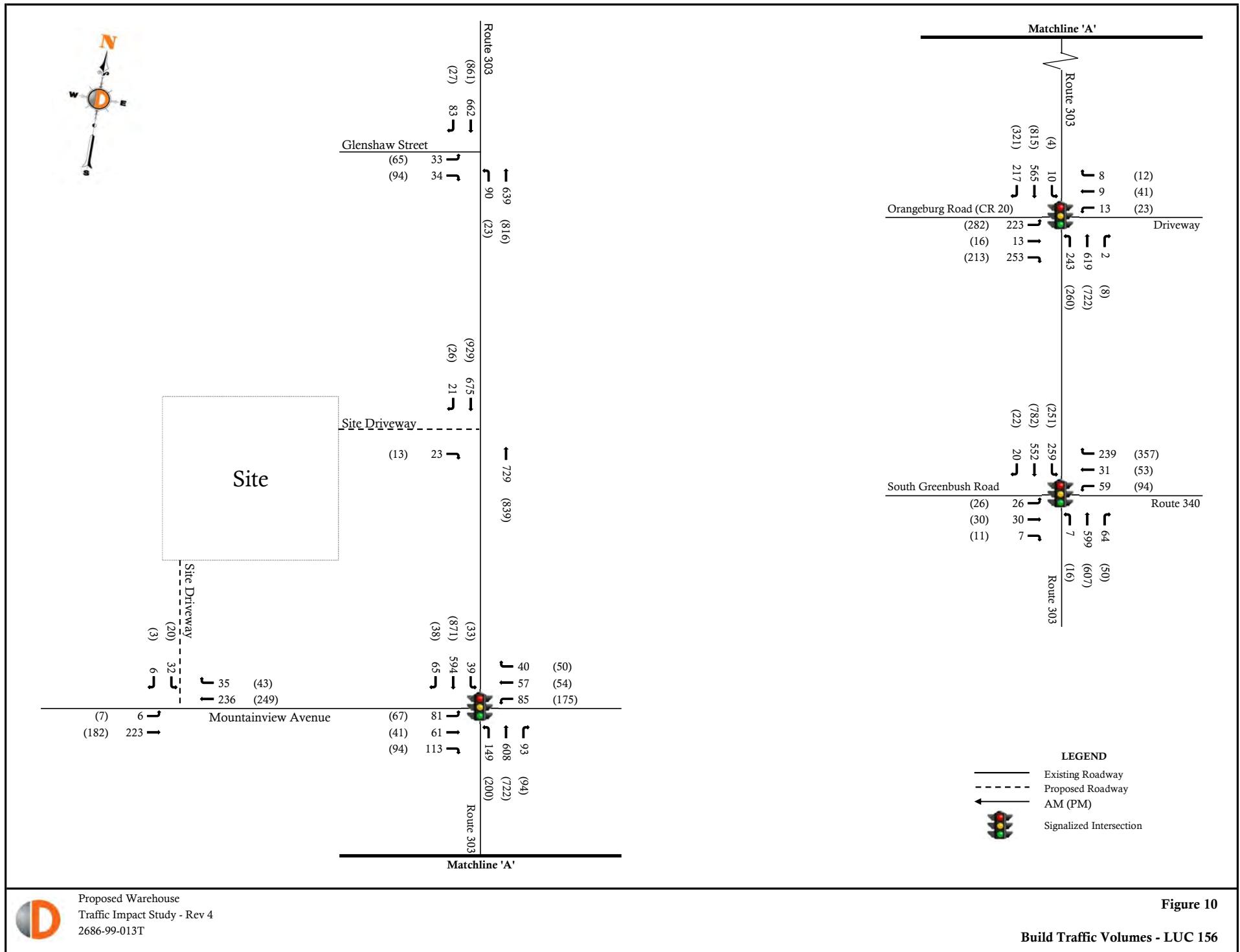
**Percent Distribution - LUC 156
(Cars)**







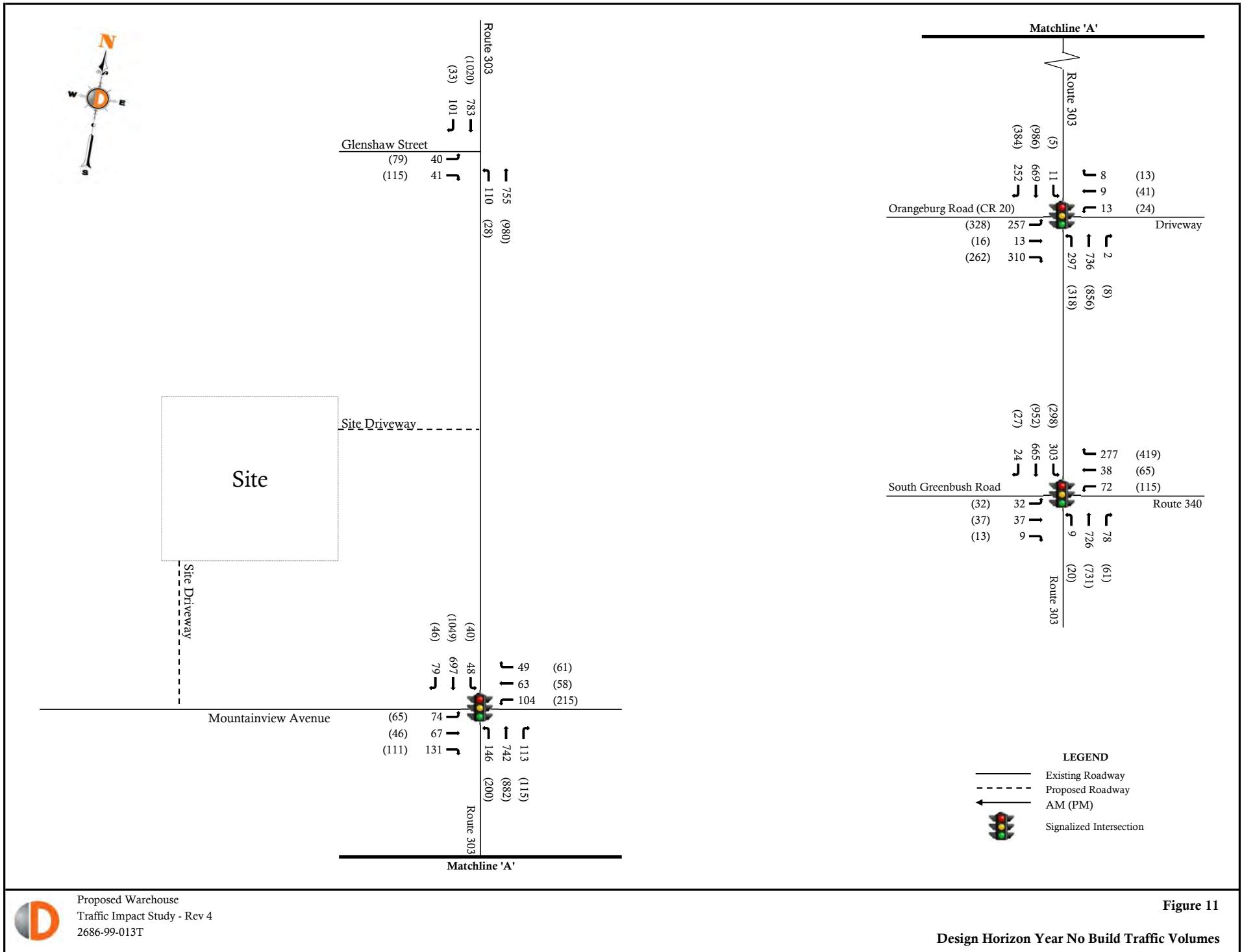


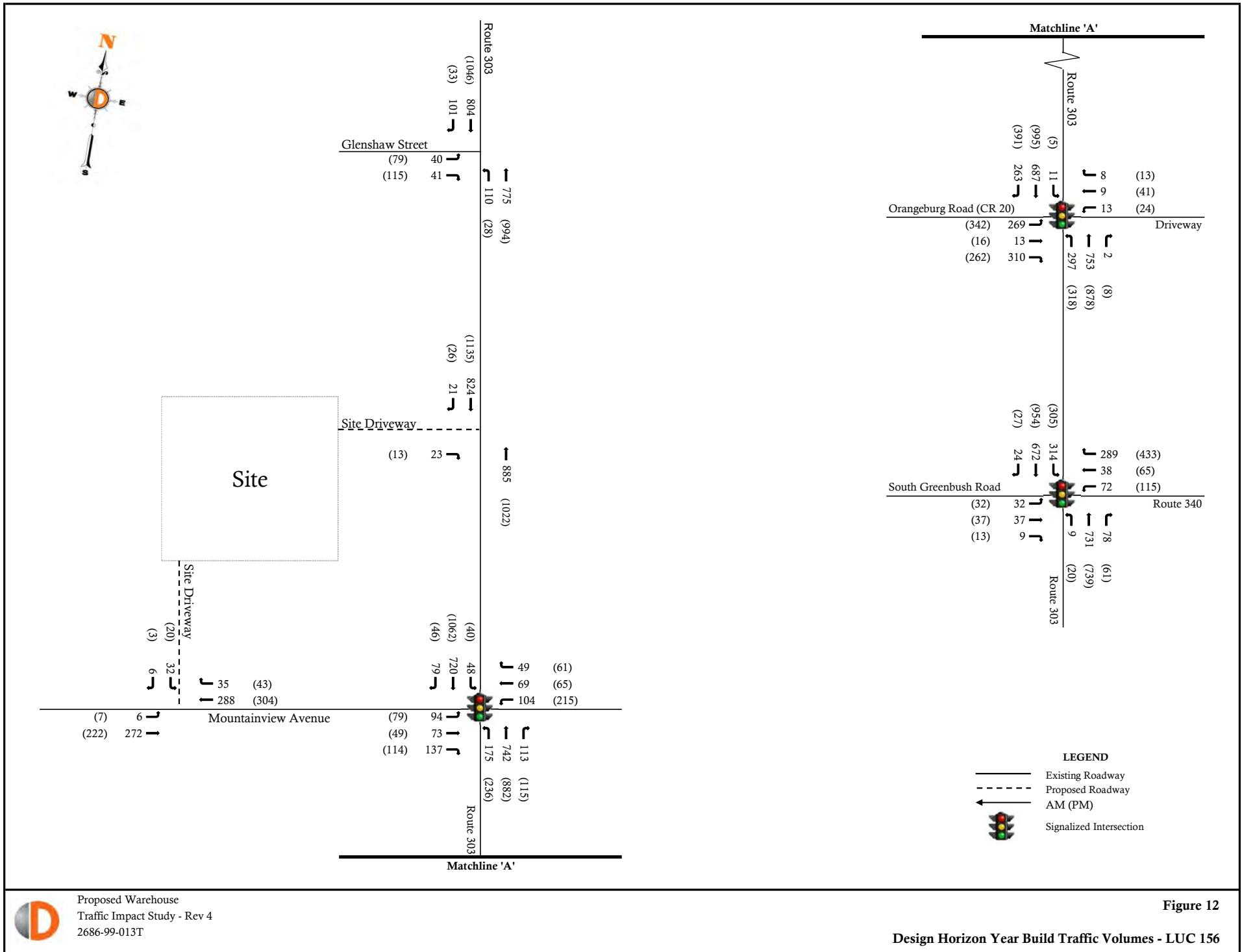


Proposed Warehouse
Traffic Impact Study - Rev 4
2686-99-013T

Figure 10

Build Traffic Volumes - LUC 156





Appendix F
Alternate Capacity Analysis

2686-99-013T

Existing - AM

10: Route 303 & Mountainview Avenue

	→	→	←	←	↑	↑	↓	↓	↗	↙	↖	↙
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	60	46	105	65	45	11	118	534	57	9	528	64
Future Volume (vph)	60	46	105	65	45	11	118	534	57	9	528	64
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	10	10	10	10	10	10
Grade (%)	-3%				-2%			0%			-1%	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Frt		0.933			0.988			0.988			0.984	
Flt Protected		0.986			0.974			0.992			0.999	
Satd. Flow (prot)	0	1724	0	0	1651	0	0	2968	0	0	2950	0
Flt Permitted		0.847			0.538			0.992			0.999	
Satd. Flow (perm)	0	1481	0	0	912	0	0	2968	0	0	2950	0
Right Turn on Red			Yes			Yes			No			Yes
Satd. Flow (RTOR)		42			4						12	
Link Speed (mph)		30			30			40			40	
Link Distance (ft)		498			709			1377			1086	
Travel Time (s)		11.3			16.1			23.5			18.5	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	2%	4%	3%	11%	0%	64%	7%	13%	4%	50%	13%	6%
Adj. Flow (vph)	65	49	113	70	48	12	127	574	61	10	568	69
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	227	0	0	130	0	0	762	0	0	647	0
Turn Type	Perm	NA		Perm	NA		Split	NA		Split	NA	
Protected Phases		4			8		6	6		2	2	
Permitted Phases		4			8							
Detector Phase	4	4		8	8		6	6		2	2	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		34.0	34.0		5.0	5.0	
Minimum Split (s)	10.0	10.0		10.0	10.0		40.0	40.0		11.0	11.0	
Total Split (s)	30.0	30.0		30.0	30.0		40.0	40.0		40.0	40.0	
Total Split (%)	27.3%	27.3%		27.3%	27.3%		36.4%	36.4%		36.4%	36.4%	
Yellow Time (s)	4.0	4.0		4.0	4.0		5.0	5.0		5.0	5.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)		0.0			0.0		0.0			0.0		
Total Lost Time (s)		5.0			5.0			6.0			6.0	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None		None	None		None	None		C-Max	C-Max	
Act Effct Green (s)		17.9			17.9			34.0			41.1	
Actuated g/C Ratio		0.16			0.16			0.31			0.37	
v/c Ratio		0.82			0.86			0.83			0.58	
Control Delay		58.6			85.7			49.2			30.8	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		58.6			85.7			49.2			30.8	
LOS		E			F			D			C	
Approach Delay		58.6			85.7			49.2			30.8	
Approach LOS		E			F			D			C	
Queue Length 50th (ft)		128			87			293			187	
Queue Length 95th (ft)		203			#152			#364			275	

2686-99-013T

Existing - AM
10: Route 303 & Mountainview Avenue



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Internal Link Dist (ft)	418			629			1297			1006		
Turn Bay Length (ft)												
Base Capacity (vph)	369				210			917			1109	
Starvation Cap Reductn	0				0			0			0	
Spillback Cap Reductn	0				0			0			0	
Storage Cap Reductn	0				0			0			0	
Reduced v/c Ratio	0.62				0.62			0.83			0.58	

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 5 (5%), Referenced to phase 2:SBTL, Start of Yellow

Natural Cycle: 80

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.86

Intersection Signal Delay: 46.4

Intersection LOS: D

Intersection Capacity Utilization 72.3%

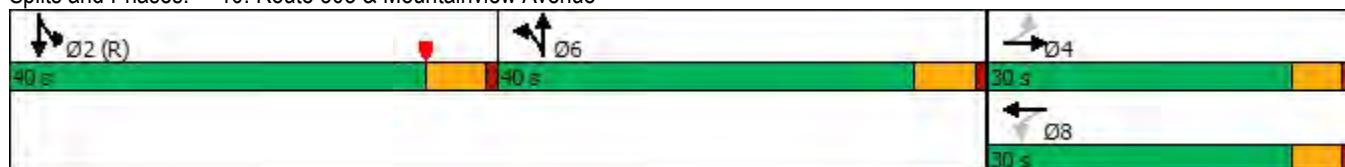
ICU Level of Service C

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 10: Route 303 & Mountainview Avenue



2686-99-013T

Existing - PM

10: Route 303 & Mountainview Avenue

	←	→	↙	↗	↖	↙	↖	↑	↗	↙	↓	↖
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	52	34	89	126	36	10	161	682	78	11	792	37
Future Volume (vph)	52	34	89	126	36	10	161	682	78	11	792	37
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	10	10	10	10	10	10
Grade (%)	-3%				-2%			0%			-1%	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Frt		0.931			0.992			0.987			0.993	
Flt Protected		0.985			0.965			0.991			0.999	
Satd. Flow (prot)	0	1733	0	0	1798	0	0	3065	0	0	3184	0
Flt Permitted		0.871			0.560			0.991			0.999	
Satd. Flow (perm)	0	1533	0	0	1043	0	0	3065	0	0	3184	0
Right Turn on Red			Yes			Yes			No			Yes
Satd. Flow (RTOR)		44			3						4	
Link Speed (mph)		30			30			40			40	
Link Distance (ft)		498			709			1377			1086	
Travel Time (s)		11.3			16.1			23.5			18.5	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	0%	0%	4%	2%	0%	12%	3%	9%	4%	50%	5%	3%
Adj. Flow (vph)	56	37	96	135	39	11	173	733	84	12	852	40
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	189	0	0	185	0	0	990	0	0	904	0
Turn Type	Perm	NA		Perm	NA		Split	NA		Split	NA	
Protected Phases		4			8		6	6		2	2	
Permitted Phases	4			8								
Detector Phase	4	4		8	8		6	6		2	2	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		34.0	34.0		5.0	5.0	
Minimum Split (s)	10.0	10.0		10.0	10.0		40.0	40.0		11.0	11.0	
Total Split (s)	30.0	30.0		30.0	30.0		40.0	40.0		40.0	40.0	
Total Split (%)	27.3%	27.3%		27.3%	27.3%		36.4%	36.4%		36.4%	36.4%	
Yellow Time (s)	4.0	4.0		4.0	4.0		5.0	5.0		5.0	5.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)		0.0		0.0			0.0			0.0		
Total Lost Time (s)		5.0			5.0			6.0			6.0	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None		None	None		None	None		C-Max	C-Max	
Act Effct Green (s)		21.4			21.4			34.0			37.6	
Actuated g/C Ratio		0.19			0.19			0.31			0.34	
v/c Ratio		0.57			0.90			1.05			0.83	
Control Delay		36.7			84.4			73.5			41.8	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		36.7			84.4			73.5			41.8	
LOS		D			F			E			D	
Approach Delay		36.7			84.4			73.5			41.8	
Approach LOS		D			F			E			D	
Queue Length 50th (ft)		90			123			~406			314	
Queue Length 95th (ft)		161			#239			#541			#445	

2686-99-013T

Existing - PM
10: Route 303 & Mountainview Avenue



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Internal Link Dist (ft)	418			629			1297			1006		
Turn Bay Length (ft)												
Base Capacity (vph)	382			239			947			1091		
Starvation Cap Reductn	0			0			0			0		
Spillback Cap Reductn	0			0			0			0		
Storage Cap Reductn	0			0			0			0		
Reduced v/c Ratio	0.49			0.77			1.05			0.83		

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 5 (5%), Referenced to phase 2:SBTL, Start of Yellow

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.05

Intersection Signal Delay: 58.7

Intersection LOS: E

Intersection Capacity Utilization 86.5%

ICU Level of Service E

Analysis Period (min) 15

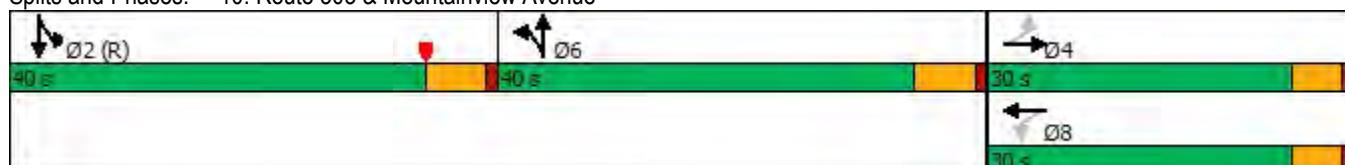
~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 10: Route 303 & Mountainview Avenue



2686-99-013T

No Build - AM

10: Route 303 & Mountainview Avenue

	→	→	←	←	←	↑	↑	↓	↓	←		
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	61	55	107	85	51	40	120	608	93	39	571	65
Future Volume (vph)	61	55	107	85	51	40	120	608	93	39	571	65
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	10	10	10	10	10	10
Grade (%)	-3%				-2%			0%			-1%	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Frt		0.935			0.969			0.983			0.986	
Flt Protected		0.986			0.976			0.993			0.997	
Satd. Flow (prot)	0	1727	0	0	1514	0	0	2960	0	0	2908	0
Flt Permitted		0.830			0.597			0.993			0.997	
Satd. Flow (perm)	0	1453	0	0	926	0	0	2960	0	0	2908	0
Right Turn on Red			Yes			Yes			No			Yes
Satd. Flow (RTOR)		39			12						11	
Link Speed (mph)		30			30			40			40	
Link Distance (ft)		498			709			1377			1086	
Travel Time (s)		11.3			16.1			23.5			18.5	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	2%	4%	3%	11%	0%	64%	7%	13%	4%	50%	13%	6%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	240	0	0	189	0	0	883	0	0	726	0
Turn Type	Perm	NA		Perm	NA		Split	NA		Split	NA	
Protected Phases		4			8		6	6		2	2	
Permitted Phases		4			8							
Detector Phase		4	4		8	8		6	6		2	2
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		34.0	34.0		5.0	5.0	
Minimum Split (s)	10.0	10.0		10.0	10.0		40.0	40.0		11.0	11.0	
Total Split (s)	30.0	30.0		30.0	30.0		40.0	40.0		40.0	40.0	
Total Split (%)	27.3%	27.3%		27.3%	27.3%		36.4%	36.4%		36.4%	36.4%	
Yellow Time (s)	4.0	4.0		4.0	4.0		5.0	5.0		5.0	5.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)		0.0			0.0			0.0			0.0	
Total Lost Time (s)		5.0			5.0			6.0			6.0	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None		None	None		None	None		C-Max	C-Max	
Act Effct Green (s)		22.8			22.8			34.0			36.2	
Actuated g/C Ratio		0.21			0.21			0.31			0.33	
v/c Ratio		0.73			0.94			0.97			0.75	
Control Delay		46.7			91.0			60.5			38.9	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		46.7			91.0			60.5			38.9	
LOS		D			F			E			D	
Approach Delay		46.7			91.0			60.5			38.9	
Approach LOS		D			F			E			D	
Queue Length 50th (ft)		130			122			345			243	
Queue Length 95th (ft)		220			#254			#468			319	
Internal Link Dist (ft)		418			629			1297			1006	

2686-99-013T

No Build - AM

10: Route 303 & Mountainview Avenue



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)												
Base Capacity (vph)	360			219			914			965		
Starvation Cap Reductn	0			0			0			0		
Spillback Cap Reductn	0			0			0			0		
Storage Cap Reductn	0			0			0			0		
Reduced v/c Ratio	0.67			0.86			0.97			0.75		

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 5 (5%), Referenced to phase 2:SBTL, Start of Yellow

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.97

Intersection Signal Delay: 54.0

Intersection LOS: D

Intersection Capacity Utilization 78.6%

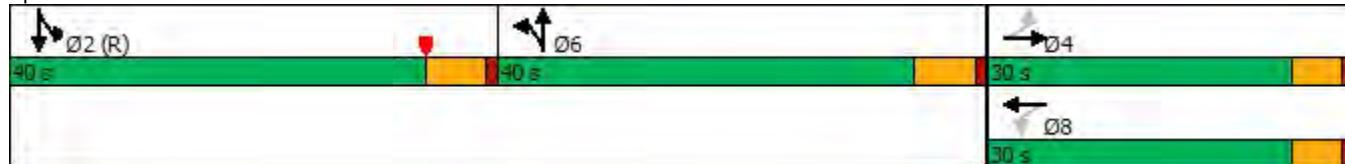
ICU Level of Service D

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 10: Route 303 & Mountainview Avenue



2686-99-013T

No Build - PM

10: Route 303 & Mountainview Avenue

	→	→	←	←	←	↑	↑	↓	↓	←		
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	53	38	91	175	47	50	164	722	94	33	858	38
Future Volume (vph)	53	38	91	175	47	50	164	722	94	33	858	38
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	10	10	10	10	10	10
Grade (%)	-3%				-2%			0%			-1%	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Frt		0.932			0.975			0.986			0.994	
Flt Protected		0.986			0.969			0.992			0.998	
Satd. Flow (prot)	0	1737	0	0	1752	0	0	3065	0	0	3154	0
Flt Permitted		0.849			0.604			0.992			0.998	
Satd. Flow (perm)	0	1496	0	0	1092	0	0	3065	0	0	3154	0
Right Turn on Red			Yes			Yes			No			Yes
Satd. Flow (RTOR)		42			10						4	
Link Speed (mph)		30			30			40			40	
Link Distance (ft)		498			709			1377			1086	
Travel Time (s)		11.3			16.1			23.5			18.5	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	0%	0%	4%	2%	0%	12%	3%	9%	4%	50%	5%	3%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	196	0	0	293	0	0	1053	0	0	999	0
Turn Type	Perm	NA		Perm	NA		Split	NA		Split	NA	
Protected Phases		4			8		6	6		2	2	
Permitted Phases	4			8								
Detector Phase	4	4		8	8		6	6		2	2	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		34.0	34.0		5.0	5.0	
Minimum Split (s)	10.0	10.0		10.0	10.0		40.0	40.0		11.0	11.0	
Total Split (s)	30.0	30.0		30.0	30.0		40.0	40.0		40.0	40.0	
Total Split (%)	27.3%	27.3%		27.3%	27.3%		36.4%	36.4%		36.4%	36.4%	
Yellow Time (s)	4.0	4.0		4.0	4.0		5.0	5.0		5.0	5.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)		0.0			0.0			0.0			0.0	
Total Lost Time (s)		5.0			5.0			6.0			6.0	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None		None	None		None	None		C-Max	C-Max	
Act Effct Green (s)		25.0			25.0			34.0			34.0	
Actuated g/C Ratio		0.23			0.23			0.31			0.31	
v/c Ratio		0.53			1.15			1.11			1.02	
Control Delay		34.9			140.9			94.4			72.6	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		34.9			140.9			94.4			72.6	
LOS	C			F			F			E		
Approach Delay		34.9			140.9			94.4			72.6	
Approach LOS		C			F			F			E	
Queue Length 50th (ft)		95			~238			~457			~394	
Queue Length 95th (ft)		171			#412			m#585			#526	
Internal Link Dist (ft)		418			629			1297			1006	



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)												
Base Capacity (vph)	372			255			947			977		
Starvation Cap Reductn	0			0			0			0		
Spillback Cap Reductn	0			0			0			0		
Storage Cap Reductn	0			0			0			0		
Reduced v/c Ratio	0.53			1.15			1.11			1.02		

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 5 (5%), Referenced to phase 2:SBTL, Start of Yellow

Natural Cycle: 130

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.15

Intersection Signal Delay: 86.6

Intersection LOS: F

Intersection Capacity Utilization 98.3%

ICU Level of Service F

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

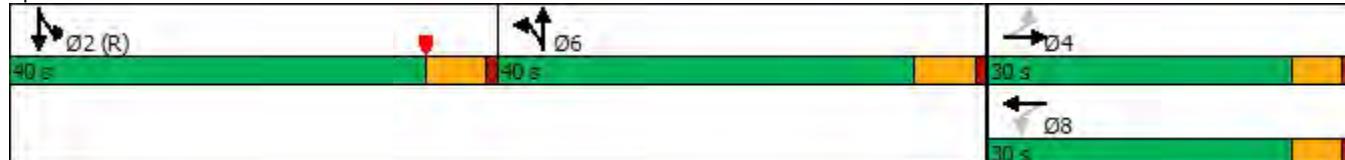
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 10: Route 303 & Mountainview Avenue



	→	→	←	←	↑	↑	↓	↓	↗	↙	↖	↙
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	81	61	113	85	57	40	149	608	93	39	594	65
Future Volume (vph)	81	61	113	85	57	40	149	608	93	39	594	65
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	10	10	10	10	10	10
Grade (%)	-3%				-2%			0%			-1%	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Frt		0.940			0.970			0.984			0.986	
Flt Protected		0.984			0.977			0.991			0.997	
Satd. Flow (prot)	0	1712	0	0	1525	0	0	2961	0	0	2909	0
Flt Permitted		0.791			0.598			0.991			0.997	
Satd. Flow (perm)	0	1376	0	0	933	0	0	2961	0	0	2909	0
Right Turn on Red			Yes			Yes			No			Yes
Satd. Flow (RTOR)		34			12						10	
Link Speed (mph)		30			30			40			40	
Link Distance (ft)		498			709			1377			1086	
Travel Time (s)		11.3			16.1			23.5			18.5	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	6%	4%	3%	11%	0%	64%	7%	13%	4%	50%	13%	6%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	275	0	0	195	0	0	914	0	0	751	0
Turn Type	Perm	NA		Perm	NA		Split	NA		Split	NA	
Protected Phases		4			8		6	6		2	2	
Permitted Phases	4			8								
Detector Phase	4	4		8	8		6	6		2	2	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		34.0	34.0		5.0	5.0	
Minimum Split (s)	10.0	10.0		10.0	10.0		40.0	40.0		11.0	11.0	
Total Split (s)	30.0	30.0		30.0	30.0		40.0	40.0		40.0	40.0	
Total Split (%)	27.3%	27.3%		27.3%	27.3%		36.4%	36.4%		36.4%	36.4%	
Yellow Time (s)	4.0	4.0		4.0	4.0		5.0	5.0		5.0	5.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)		0.0			0.0			0.0			0.0	
Total Lost Time (s)		5.0			5.0			6.0			6.0	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None		None	None		None	None		C-Max	C-Max	
Act Effct Green (s)		23.3			23.3			34.0			35.7	
Actuated g/C Ratio		0.21			0.21			0.31			0.32	
v/c Ratio		0.86			0.95			1.00			0.79	
Control Delay		62.7			90.6			66.9			41.0	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		62.7			90.6			66.9			41.0	
LOS	E			F			E			D		
Approach Delay		62.7			90.6			66.9			41.0	
Approach LOS		E			F			E			D	
Queue Length 50th (ft)		164			126			358			255	
Queue Length 95th (ft)		#303			#264			#492			#335	
Internal Link Dist (ft)		418			629			1297			1006	



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)												
Base Capacity (vph)	339			221			915			952		
Starvation Cap Reductn	0			0			0			0		
Spillback Cap Reductn	0			0			0			0		
Storage Cap Reductn	0			0			0			0		
Reduced v/c Ratio	0.81			0.88			1.00			0.79		

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 5 (5%), Referenced to phase 2:SBTL, Start of Yellow

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.00

Intersection Signal Delay: 59.4

Intersection LOS: E

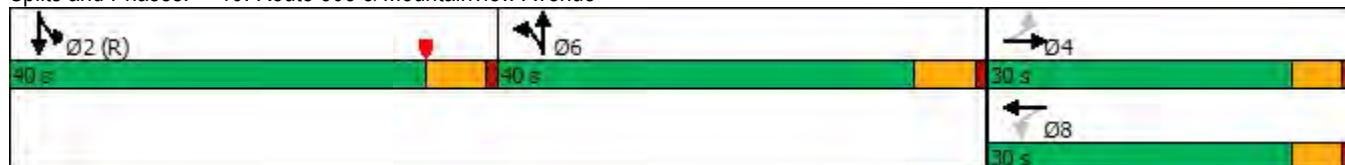
Intersection Capacity Utilization 78.7%

ICU Level of Service D

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 10: Route 303 & Mountainview Avenue

	→	→	←	←	↑	↑	↓	↓	↗	↙	↖	↙
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	67	41	94	175	54	50	200	722	94	33	871	38
Future Volume (vph)	67	41	94	175	54	50	200	722	94	33	871	38
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	10	10	10	10	10	10
Grade (%)	-3%				-2%			0%			-1%	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Frt		0.937			0.976			0.986			0.994	
Flt Protected		0.984			0.970			0.990			0.998	
Satd. Flow (prot)	0	1723	0	0	1757	0	0	3064	0	0	3155	0
Flt Permitted		0.819			0.600			0.990			0.998	
Satd. Flow (perm)	0	1434	0	0	1087	0	0	3064	0	0	3155	0
Right Turn on Red			Yes			Yes			No			Yes
Satd. Flow (RTOR)		37			9						4	
Link Speed (mph)		30			30			40			40	
Link Distance (ft)		498			709			1377			1086	
Travel Time (s)		11.3			16.1			23.5			18.5	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	4%	0%	4%	2%	0%	12%	3%	9%	4%	50%	5%	3%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	217	0	0	300	0	0	1092	0	0	1013	0
Turn Type	Perm	NA		Perm	NA		Split	NA		Split	NA	
Protected Phases		4			8		6	6		2	2	
Permitted Phases	4			8								
Detector Phase	4	4		8	8		6	6		2	2	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		34.0	34.0		5.0	5.0	
Minimum Split (s)	10.0	10.0		10.0	10.0		40.0	40.0		11.0	11.0	
Total Split (s)	30.0	30.0		30.0	30.0		40.0	40.0		40.0	40.0	
Total Split (%)	27.3%	27.3%		27.3%	27.3%		36.4%	36.4%		36.4%	36.4%	
Yellow Time (s)	4.0	4.0		4.0	4.0		5.0	5.0		5.0	5.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)		0.0			0.0			0.0			0.0	
Total Lost Time (s)		5.0			5.0			6.0			6.0	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None		None	None		None	None		C-Max	C-Max	
Act Effct Green (s)		25.0			25.0			34.0			34.0	
Actuated g/C Ratio		0.23			0.23			0.31			0.31	
v/c Ratio		0.61			1.18			1.15			1.04	
Control Delay		39.9			152.1			109.8			76.4	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		39.9			152.1			109.8			76.4	
LOS	D			F			F			E		
Approach Delay		39.9			152.1			109.8			76.4	
Approach LOS		D		F			F			E		
Queue Length 50th (ft)		115			~251			~489			~405	
Queue Length 95th (ft)		198			#426			m#601			#538	
Internal Link Dist (ft)		418			629			1297			1006	



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)												
Base Capacity (vph)		354			254			947			977	
Starvation Cap Reductn		0			0			0			0	
Spillback Cap Reductn		0			0			0			0	
Storage Cap Reductn		0			0			0			0	
Reduced v/c Ratio		0.61			1.18			1.15			1.04	

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 5 (5%), Referenced to phase 2:SBTL, Start of Yellow

Natural Cycle: 150

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.18

Intersection Signal Delay: 96.0

Intersection LOS: F

Intersection Capacity Utilization 97.4%

ICU Level of Service F

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

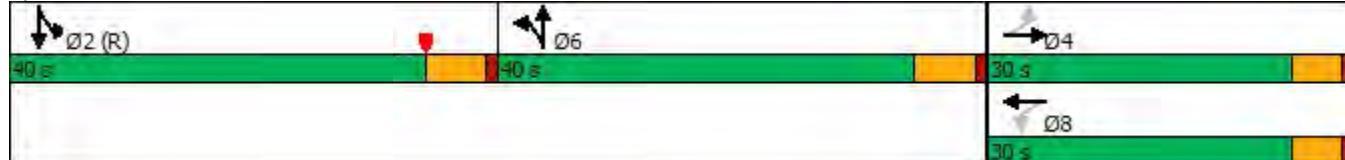
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 10: Route 303 & Mountainview Avenue



2686-99-013T

Build w Mitigation - AM
10: Route 303 & Mountainview Avenue

	→	→	→	←	←	↑	↑	↓	↓	←	→	
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1	2	3	4	5	6	7	8	9	10	11	12
Traffic Volume (vph)	81	61	113	85	57	40	149	608	93	39	594	65
Future Volume (vph)	81	61	113	85	57	40	149	608	93	39	594	65
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	10	10	10	10	10	10
Grade (%)	-3%				-2%			0%			-1%	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Frt		0.903			0.970			0.984			0.986	
Flt Protected	0.950				0.977			0.991			0.997	
Satd. Flow (prot)	1728	1685	0	0	1525	0	0	2961	0	0	2909	0
Flt Permitted	0.572				0.592			0.991			0.997	
Satd. Flow (perm)	1041	1685	0	0	924	0	0	2961	0	0	2909	0
Right Turn on Red			Yes			Yes			No			Yes
Satd. Flow (RTOR)	78				12						10	
Link Speed (mph)	30				30			40			40	
Link Distance (ft)	498				709			1377			1086	
Travel Time (s)	11.3				16.1			23.5			18.5	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	6%	4%	3%	11%	0%	64%	7%	13%	4%	50%	13%	6%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	87	188	0	0	195	0	0	914	0	0	751	0
Turn Type	Perm	NA		Perm	NA		Split	NA		Split	NA	
Protected Phases		4			8		6	6		2	2	
Permitted Phases	4			8								
Detector Phase	4	4		8	8		6	6		2	2	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		34.0	34.0		5.0	5.0	
Minimum Split (s)	10.0	10.0		10.0	10.0		40.0	40.0		11.0	11.0	
Total Split (s)	30.0	30.0		30.0	30.0		40.0	40.0		40.0	40.0	
Total Split (%)	27.3%	27.3%		27.3%	27.3%		36.4%	36.4%		36.4%	36.4%	
Maximum Green (s)	25.0	25.0		25.0	25.0		34.0	34.0		34.0	34.0	
Yellow Time (s)	4.0	4.0		4.0	4.0		5.0	5.0		5.0	5.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)	0.0	0.0		0.0			0.0			0.0		
Total Lost Time (s)	5.0	5.0		5.0			6.0			6.0		
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	2.0	2.0		2.0	2.0		2.0	2.0		3.0	3.0	
Recall Mode	None	None		None	None		None	None		C-Max	C-Max	
Walk Time (s)	7.0	7.0								7.0	7.0	
Flash Dont Walk (s)	18.0	18.0								15.0	15.0	
Pedestrian Calls (#/hr)	0	0								0	0	
Act Effct Green (s)	23.2	23.2		23.2			34.0				35.8	
Actuated g/C Ratio	0.21	0.21		0.21			0.31				0.33	
v/c Ratio	0.40	0.45		0.96			1.00				0.79	
Control Delay	42.6	24.9		93.5			66.9				40.8	
Queue Delay	0.0	0.0		0.0			0.0				0.0	
Total Delay	42.6	24.9		93.5			66.9				40.8	
LOS	D	C		F			E			D		



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Approach Delay		30.5			93.5			66.9			40.8	
Approach LOS		C			F			E			D	
Queue Length 50th (ft)	52	65			127			358			255	
Queue Length 95th (ft)	102	133			#265			#492			#335	
Internal Link Dist (ft)		418			629			1297			1006	
Turn Bay Length (ft)												
Base Capacity (vph)	236	443			219			915			953	
Starvation Cap Reductn	0	0			0			0			0	
Spillback Cap Reductn	0	0			0			0			0	
Storage Cap Reductn	0	0			0			0			0	
Reduced v/c Ratio	0.37	0.42			0.89			1.00			0.79	

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 5 (5%), Referenced to phase 2:SBTL, Start of Yellow

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.00

Intersection Signal Delay: 55.5

Intersection LOS: E

Intersection Capacity Utilization 86.6%

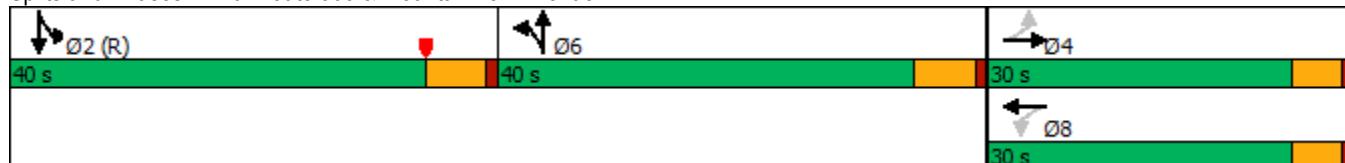
ICU Level of Service E

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 10: Route 303 & Mountainview Avenue



2686-99-013T

Build w Mitigation - PM
10: Route 303 & Mountainview Avenue

	↗	→	↘	↶	←	↖	↑	↗	↘	↓	↶	
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑			↔			↔		↔	↔	
Traffic Volume (vph)	67	41	94	175	54	50	200	722	94	33	871	38
Future Volume (vph)	67	41	94	175	54	50	200	722	94	33	871	38
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	10	10	10	10	10	10
Grade (%)	-3%				-2%			0%			-1%	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Frt		0.896			0.976			0.986			0.994	
Flt Protected		0.950			0.970			0.990			0.998	
Satd. Flow (prot)	1762	1681	0	0	1757	0	0	3064	0	0	3155	0
Flt Permitted		0.608			0.654			0.990			0.998	
Satd. Flow (perm)	1127	1681	0	0	1184	0	0	3064	0	0	3155	0
Right Turn on Red			Yes			Yes			No			Yes
Satd. Flow (RTOR)		97			9						4	
Link Speed (mph)		30			30			40			40	
Link Distance (ft)		498			709			1377			1086	
Travel Time (s)		11.3			16.1			23.5			18.5	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	4%	0%	4%	2%	0%	12%	3%	9%	4%	50%	5%	3%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	72	145	0	0	300	0	0	1092	0	0	1013	0
Turn Type	Perm	NA		Perm	NA		Split	NA		Split	NA	
Protected Phases		4			8		6	6		2	2	
Permitted Phases		4			8							
Detector Phase	4	4		8	8		6	6		2	2	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		34.0	34.0		5.0	5.0	
Minimum Split (s)	10.0	10.0		10.0	10.0		40.0	40.0		11.0	11.0	
Total Split (s)	30.0	30.0		30.0	30.0		40.0	40.0		40.0	40.0	
Total Split (%)	27.3%	27.3%		27.3%	27.3%		36.4%	36.4%		36.4%	36.4%	
Maximum Green (s)	25.0	25.0		25.0	25.0		34.0	34.0		34.0	34.0	
Yellow Time (s)	4.0	4.0		4.0	4.0		5.0	5.0		5.0	5.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)	0.0	0.0		0.0			0.0			0.0		
Total Lost Time (s)	5.0	5.0			5.0			6.0			6.0	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	2.0	2.0		2.0	2.0		2.0	2.0		3.0	3.0	
Recall Mode	None	None		None	None		None	None		C-Max	C-Max	
Walk Time (s)	7.0	7.0								7.0	7.0	
Flash Dont Walk (s)	18.0	18.0								15.0	15.0	
Pedestrian Calls (#/hr)	0	0								0	0	
Act Effct Green (s)	25.0	25.0		25.0			34.0			34.0		
Actuated g/C Ratio	0.23	0.23		0.23			0.31			0.31		
v/c Ratio	0.28	0.32		1.09			1.15			1.04		
Control Delay	38.7	15.4		119.3			109.8			76.4		
Queue Delay	0.0	0.0		0.0			0.0			0.0		
Total Delay	38.7	15.4		119.3			109.8			76.4		
LOS	D	B		F			F			E		



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Approach Delay		23.1			119.3			109.8			76.4	
Approach LOS		C			F			F			E	
Queue Length 50th (ft)	42	27			~234			~489			~405	
Queue Length 95th (ft)	86	81			#410			m#601			#538	
Internal Link Dist (ft)		418			629			1297			1006	
Turn Bay Length (ft)												
Base Capacity (vph)	256	457			276			947			977	
Starvation Cap Reductn	0	0			0			0			0	
Spillback Cap Reductn	0	0			0			0			0	
Storage Cap Reductn	0	0			0			0			0	
Reduced v/c Ratio	0.28	0.32			1.09			1.15			1.04	

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 5 (5%), Referenced to phase 2:SBTL, Start of Yellow

Natural Cycle: 130

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.15

Intersection Signal Delay: 90.8

Intersection LOS: F

Intersection Capacity Utilization 96.9%

ICU Level of Service F

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

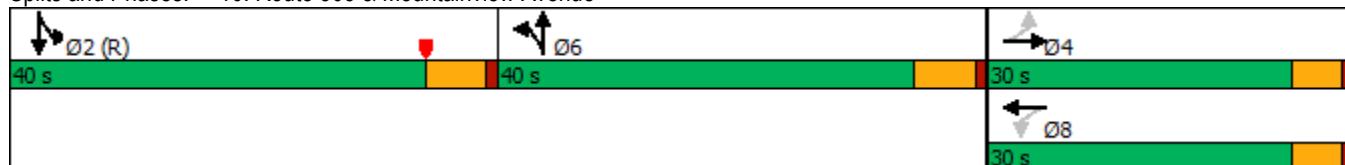
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 10: Route 303 & Mountainview Avenue



2686-99-013T

Design Horizon Year No Build - AM

10: Route 303 & Mountainview Avenue

	→	→	→	←	←	↑	↑	↓	↓	←	→	
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	74	67	131	104	63	49	146	742	113	48	697	79
Future Volume (vph)	74	67	131	104	63	49	146	742	113	48	697	79
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	10	10	10	10	10	10
Grade (%)	-3%				-2%			0%			-1%	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Frt		0.935			0.969			0.983			0.986	
Flt Protected		0.987			0.977			0.993			0.997	
Satd. Flow (prot)	0	1728	0	0	1516	0	0	2960	0	0	2907	0
Flt Permitted		0.811			0.566			0.993			0.997	
Satd. Flow (perm)	0	1420	0	0	878	0	0	2960	0	0	2907	0
Right Turn on Red			Yes			Yes			No			Yes
Satd. Flow (RTOR)		39			12						11	
Link Speed (mph)		30			30			40			40	
Link Distance (ft)		498			709			1377			1086	
Travel Time (s)		11.3			16.1			23.5			18.5	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	2%	4%	3%	11%	0%	64%	7%	13%	4%	50%	13%	6%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	293	0	0	233	0	0	1077	0	0	886	0
Turn Type	Perm	NA		Perm	NA		Split	NA		Split	NA	
Protected Phases		4			8		6	6		2	2	
Permitted Phases	4			8								
Detector Phase	4	4		8	8		6	6		2	2	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		34.0	34.0		5.0	5.0	
Minimum Split (s)	10.0	10.0		10.0	10.0		40.0	40.0		11.0	11.0	
Total Split (s)	30.0	30.0		30.0	30.0		40.0	40.0		40.0	40.0	
Total Split (%)	27.3%	27.3%		27.3%	27.3%		36.4%	36.4%		36.4%	36.4%	
Yellow Time (s)	4.0	4.0		4.0	4.0		5.0	5.0		5.0	5.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)		0.0			0.0			0.0			0.0	
Total Lost Time (s)		5.0			5.0			6.0			6.0	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None		None	None		None	None		C-Max	C-Max	
Act Effct Green (s)		25.0			25.0			34.0			34.0	
Actuated g/C Ratio		0.23			0.23			0.31			0.31	
v/c Ratio		0.83			1.12			1.18			0.98	
Control Delay		56.1			136.9			121.6			62.8	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		56.1			136.9			121.6			62.8	
LOS	E			F			F			E		
Approach Delay		56.1			136.9			121.6			62.8	
Approach LOS		E			F			F			E	
Queue Length 50th (ft)		174			~183			~489			322	
Queue Length 95th (ft)		#321			#342			m#590			#461	
Internal Link Dist (ft)		418			629			1297			1006	



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)												
Base Capacity (vph)	352			208			914			906		
Starvation Cap Reductn	0			0			0			0		
Spillback Cap Reductn	0			0			0			0		
Storage Cap Reductn	0			0			0			0		
Reduced v/c Ratio	0.83			1.12			1.18			0.98		

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 5 (5%), Referenced to phase 2:SBTL, Start of Yellow

Natural Cycle: 130

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.18

Intersection Signal Delay: 94.4

Intersection LOS: F

Intersection Capacity Utilization 86.7%

ICU Level of Service E

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

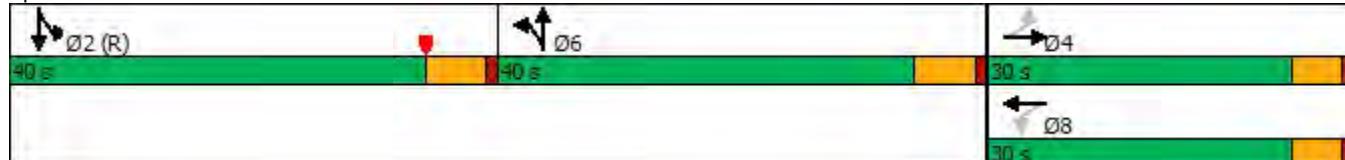
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 10: Route 303 & Mountainview Avenue



2686-99-013T

Design Horizon Year No Build - PM

10: Route 303 & Mountainview Avenue

	→	→	→	←	←	↑	↑	↓	↓	↗	↙	
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	65	46	111	215	58	61	200	882	115	40	1049	46
Future Volume (vph)	65	46	111	215	58	61	200	882	115	40	1049	46
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	10	10	10	10	10	10
Grade (%)	-3%				-2%			0%			-1%	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Frt		0.932			0.975			0.986			0.994	
Flt Protected		0.986			0.969			0.992			0.998	
Satd. Flow (prot)	0	1737	0	0	1752	0	0	3065	0	0	3154	0
Flt Permitted		0.844			0.559			0.992			0.998	
Satd. Flow (perm)	0	1487	0	0	1011	0	0	3065	0	0	3154	0
Right Turn on Red			Yes			Yes			No			Yes
Satd. Flow (RTOR)		42			10						4	
Link Speed (mph)		30			30			40			40	
Link Distance (ft)		498			709			1377			1086	
Travel Time (s)		11.3			16.1			23.5			18.5	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	0%	0%	4%	2%	0%	12%	3%	9%	4%	50%	5%	3%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	238	0	0	359	0	0	1287	0	0	1220	0
Turn Type	Perm	NA		Perm	NA		Split	NA		Split	NA	
Protected Phases		4			8		6	6		2	2	
Permitted Phases	4			8								
Detector Phase	4	4		8	8		6	6		2	2	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		34.0	34.0		5.0	5.0	
Minimum Split (s)	10.0	10.0		10.0	10.0		40.0	40.0		11.0	11.0	
Total Split (s)	30.0	30.0		30.0	30.0		40.0	40.0		40.0	40.0	
Total Split (%)	27.3%	27.3%		27.3%	27.3%		36.4%	36.4%		36.4%	36.4%	
Yellow Time (s)	4.0	4.0		4.0	4.0		5.0	5.0		5.0	5.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)		0.0			0.0			0.0			0.0	
Total Lost Time (s)		5.0			5.0			6.0			6.0	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None		None	None		None	None		C-Max	C-Max	
Act Effct Green (s)		25.0			25.0			34.0			34.0	
Actuated g/C Ratio		0.23			0.23			0.31			0.31	
v/c Ratio		0.64			1.51			1.36			1.25	
Control Delay		40.5			283.4			192.9			154.3	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		40.5			283.4			192.9			154.3	
LOS	D			F			F			F		
Approach Delay		40.5			283.4			192.9			154.3	
Approach LOS		D		F			F			F		
Queue Length 50th (ft)		126			~351			~645			~569	
Queue Length 95th (ft)		214			#537			m#607			#706	
Internal Link Dist (ft)		418			629			1297			1006	



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)												
Base Capacity (vph)	370			237			947			977		
Starvation Cap Reductn	0			0			0			0		
Spillback Cap Reductn	0			0			0			0		
Storage Cap Reductn	0			0			0			0		
Reduced v/c Ratio	0.64			1.51			1.36			1.25		

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 5 (5%), Referenced to phase 2:SBTL, Start of Yellow

Natural Cycle: 150

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.51

Intersection Signal Delay: 176.5

Intersection LOS: F

Intersection Capacity Utilization 115.3%

ICU Level of Service H

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

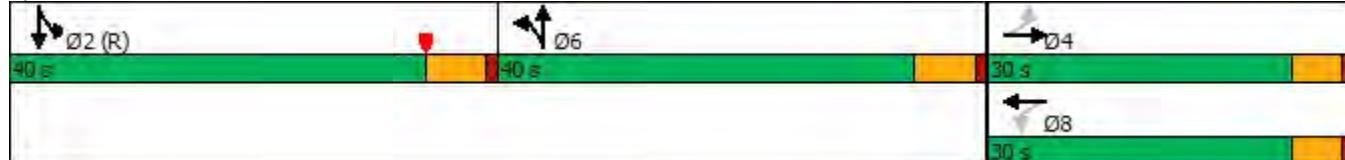
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 10: Route 303 & Mountainview Avenue



2686-99-013T

Design Horizon Year Build - AM

10: Route 303 & Mountainview Avenue

	→	→	→	←	←	↑	↑	↓	↓	←	→	
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	94	73	137	104	69	49	175	742	113	48	720	79
Future Volume (vph)	94	73	137	104	69	49	175	742	113	48	720	79
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	10	10	10	10	10	10
Grade (%)	-3%				-2%			0%			-1%	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Frt		0.939			0.970			0.983			0.986	
Flt Protected		0.985			0.977			0.992			0.997	
Satd. Flow (prot)	0	1712	0	0	1524	0	0	2960	0	0	2908	0
Flt Permitted		0.779			0.569			0.992			0.997	
Satd. Flow (perm)	0	1354	0	0	887	0	0	2960	0	0	2908	0
Right Turn on Red			Yes			Yes			No			Yes
Satd. Flow (RTOR)		35			12						10	
Link Speed (mph)		30			30			40			40	
Link Distance (ft)		498			709			1377			1086	
Travel Time (s)		11.3			16.1			23.5			18.5	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	6%	4%	3%	11%	0%	64%	7%	13%	4%	50%	13%	6%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	326	0	0	239	0	0	1108	0	0	911	0
Turn Type	Perm	NA		Perm	NA		Split	NA		Split	NA	
Protected Phases		4			8		6	6		2	2	
Permitted Phases	4			8								
Detector Phase	4	4		8	8		6	6		2	2	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		34.0	34.0		5.0	5.0	
Minimum Split (s)	10.0	10.0		10.0	10.0		40.0	40.0		11.0	11.0	
Total Split (s)	30.0	30.0		30.0	30.0		40.0	40.0		40.0	40.0	
Total Split (%)	27.3%	27.3%		27.3%	27.3%		36.4%	36.4%		36.4%	36.4%	
Yellow Time (s)	4.0	4.0		4.0	4.0		5.0	5.0		5.0	5.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)		0.0			0.0			0.0			0.0	
Total Lost Time (s)		5.0			5.0			6.0			6.0	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None		None	None		None	None		C-Max	C-Max	
Act Effct Green (s)		25.0			25.0			34.0			34.0	
Actuated g/C Ratio		0.23			0.23			0.31			0.31	
v/c Ratio		0.98			1.14			1.21			1.01	
Control Delay		82.4			142.3			135.0			69.6	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		82.4			142.3			135.0			69.6	
LOS		F			F			F			E	
Approach Delay		82.4			142.3			135.0			69.6	
Approach LOS		F			F			F			E	
Queue Length 50th (ft)		208			~190			~514			~338	
Queue Length 95th (ft)		#393			#351			m#594			#482	
Internal Link Dist (ft)		418			629			1297			1006	



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)												
Base Capacity (vph)	334			210			914			905		
Starvation Cap Reductn	0			0			0			0		
Spillback Cap Reductn	0			0			0			0		
Storage Cap Reductn	0			0			0			0		
Reduced v/c Ratio	0.98				1.14			1.21			1.01	

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 5 (5%), Referenced to phase 2:SBTL, Start of Yellow

Natural Cycle: 140

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.21

Intersection Signal Delay: 106.0

Intersection LOS: F

Intersection Capacity Utilization 86.8%

ICU Level of Service E

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

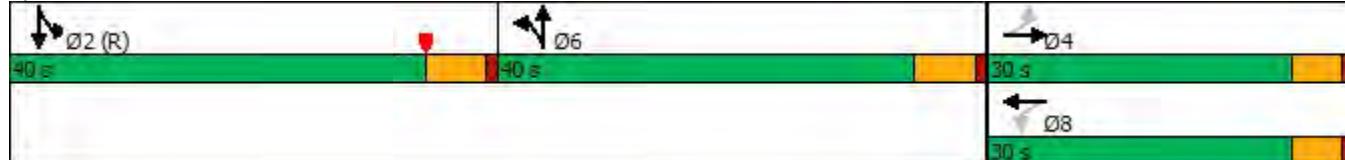
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 10: Route 303 & Mountainview Avenue



	→	→	→	←	←	↑	↑	↓	↓	↗	↙	
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	79	49	114	215	65	61	236	882	115	40	1062	46
Future Volume (vph)	79	49	114	215	65	61	236	882	115	40	1062	46
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	10	10	10	10	10	10
Grade (%)	-3%				-2%			0%			-1%	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Frt		0.936			0.976			0.986			0.994	
Flt Protected		0.984			0.969			0.991			0.998	
Satd. Flow (prot)	0	1721	0	0	1755	0	0	3066	0	0	3154	0
Flt Permitted		0.819			0.556			0.991			0.998	
Satd. Flow (perm)	0	1433	0	0	1007	0	0	3066	0	0	3154	0
Right Turn on Red			Yes			Yes			No			Yes
Satd. Flow (RTOR)		38			9						4	
Link Speed (mph)		30			30			40			40	
Link Distance (ft)		498			709			1377			1086	
Travel Time (s)		11.3			16.1			23.5			18.5	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	4%	0%	4%	2%	0%	12%	3%	9%	4%	50%	5%	3%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	261	0	0	367	0	0	1326	0	0	1234	0
Turn Type	Perm	NA		Perm	NA		Split	NA		Split	NA	
Protected Phases		4			8		6	6		2	2	
Permitted Phases	4			8								
Detector Phase	4	4		8	8		6	6		2	2	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		34.0	34.0		5.0	5.0	
Minimum Split (s)	10.0	10.0		10.0	10.0		40.0	40.0		11.0	11.0	
Total Split (s)	30.0	30.0		30.0	30.0		40.0	40.0		40.0	40.0	
Total Split (%)	27.3%	27.3%		27.3%	27.3%		36.4%	36.4%		36.4%	36.4%	
Yellow Time (s)	4.0	4.0		4.0	4.0		5.0	5.0		5.0	5.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)		0.0			0.0			0.0			0.0	
Total Lost Time (s)		5.0			5.0			6.0			6.0	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None		None	None		None	None		C-Max	C-Max	
Act Effct Green (s)		25.0			25.0			34.0			34.0	
Actuated g/C Ratio		0.23			0.23			0.31			0.31	
v/c Ratio		0.74			1.56			1.40			1.26	
Control Delay		47.2			303.1			210.9			160.2	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		47.2			303.1			210.9			160.2	
LOS	D			F			F			F		
Approach Delay		47.2			303.1			210.9			160.2	
Approach LOS		D		F			F			F		
Queue Length 50th (ft)		148			~365			~676			~580	
Queue Length 95th (ft)		#265			#553			m#614			#718	
Internal Link Dist (ft)		418			629			1297			1006	



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)												
Base Capacity (vph)	355			235			947			977		
Starvation Cap Reductn	0			0			0			0		
Spillback Cap Reductn	0			0			0			0		
Storage Cap Reductn	0			0			0			0		
Reduced v/c Ratio	0.74			1.56			1.40			1.26		

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 5 (5%), Referenced to phase 2:SBTL, Start of Yellow

Natural Cycle: 150

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.56

Intersection Signal Delay: 188.5

Intersection LOS: F

Intersection Capacity Utilization 116.0%

ICU Level of Service H

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

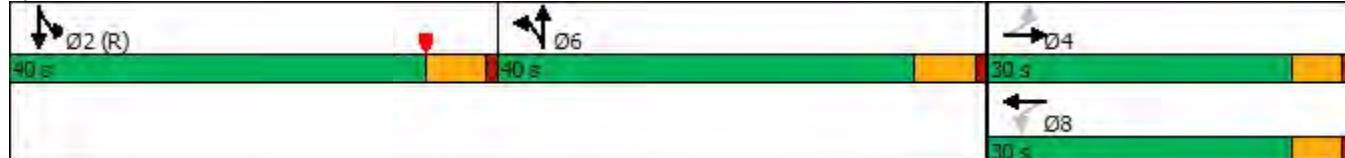
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 10: Route 303 & Mountainview Avenue



2686-99-013T

Design Horizon Year Build w Mitigation - AM

10: Route 303 & Mountainview Avenue

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↓			↔			↔			↔	
Traffic Volume (vph)	94	73	137	104	69	49	175	742	113	48	720	79
Future Volume (vph)	94	73	137	104	69	49	175	742	113	48	720	79
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	10	10	10	10	10	10
Grade (%)	-3%				-2%			0%			-1%	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Frt		0.902			0.970			0.983			0.986	
Flt Protected	0.950				0.977			0.992			0.997	
Satd. Flow (prot)	1728	1683	0	0	1524	0	0	2960	0	0	2908	0
Flt Permitted	0.547				0.530			0.992			0.997	
Satd. Flow (perm)	995	1683	0	0	827	0	0	2960	0	0	2908	0
Right Turn on Red			Yes			Yes			No			Yes
Satd. Flow (RTOR)	80				12						10	
Link Speed (mph)	30				30			40			40	
Link Distance (ft)	498				709			1377			1086	
Travel Time (s)	11.3				16.1			23.5			18.5	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	6%	4%	3%	11%	0%	64%	7%	13%	4%	50%	13%	6%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	101	225	0	0	239	0	0	1108	0	0	911	0
Turn Type	Perm	NA		Perm	NA		Split	NA		Split	NA	
Protected Phases		4			8		6	6		2	2	
Permitted Phases	4			8								
Detector Phase	4	4		8	8		6	6		2	2	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		34.0	34.0		5.0	5.0	
Minimum Split (s)	10.0	10.0		10.0	10.0		40.0	40.0		11.0	11.0	
Total Split (s)	30.0	30.0		30.0	30.0		40.0	40.0		40.0	40.0	
Total Split (%)	27.3%	27.3%		27.3%	27.3%		36.4%	36.4%		36.4%	36.4%	
Maximum Green (s)	25.0	25.0		25.0	25.0		34.0	34.0		34.0	34.0	
Yellow Time (s)	4.0	4.0		4.0	4.0		5.0	5.0		5.0	5.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)	0.0	0.0		0.0			0.0			0.0		
Total Lost Time (s)	5.0	5.0			5.0			6.0			6.0	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	2.0	2.0		2.0	2.0		2.0	2.0		3.0	3.0	
Recall Mode	None	None		None	None		None	None		C-Max	C-Max	
Walk Time (s)	7.0	7.0								7.0	7.0	
Flash Dont Walk (s)	18.0	18.0								15.0	15.0	
Pedestrian Calls (#/hr)	0	0								0	0	
Act Effct Green (s)	25.0	25.0		25.0			34.0			34.0		
Actuated g/C Ratio	0.23	0.23		0.23			0.31			0.31		
v/c Ratio	0.45	0.51			1.21		1.21			1.01		
Control Delay	44.0	28.0		169.7			135.0			69.6		
Queue Delay	0.0	0.0		0.0			0.0			0.0		
Total Delay	44.0	28.0		169.7			135.0			69.6		
LOS	D	C		F			F			E		



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Approach Delay		33.0			169.7			135.0			69.6	
Approach LOS		C			F			F			E	
Queue Length 50th (ft)	62	88			~201			~514			~338	
Queue Length 95th (ft)	117	166			#361			m#594			#482	
Internal Link Dist (ft)		418			629			1297			1006	
Turn Bay Length (ft)												
Base Capacity (vph)	226	444			197			914			905	
Starvation Cap Reductn	0	0			0			0			0	
Spillback Cap Reductn	0	0			0			0			0	
Storage Cap Reductn	0	0			0			0			0	
Reduced v/c Ratio	0.45	0.51			1.21			1.21			1.01	

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 5 (5%), Referenced to phase 2:SBTL, Start of Yellow

Natural Cycle: 140

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.21

Intersection Signal Delay: 102.3

Intersection LOS: F

Intersection Capacity Utilization 96.0%

ICU Level of Service F

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

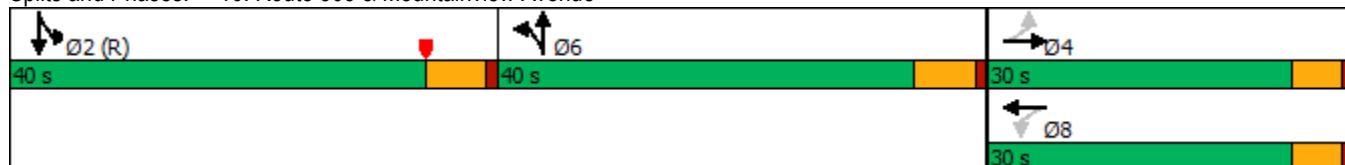
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 10: Route 303 & Mountainview Avenue



2686-99-013T

Design Horizon Year Build w Mitigation - PM

10: Route 303 & Mountainview Avenue

	→	→	→	←	←	↑	↑	↓	↓	←	→	
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1	2	3	4	5	6	7	8	9	10	11	12
Traffic Volume (vph)	79	49	114	215	65	61	236	882	115	40	1062	46
Future Volume (vph)	79	49	114	215	65	61	236	882	115	40	1062	46
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	10	10	10	10	10	10
Grade (%)	-3%				-2%			0%			-1%	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Frt		0.895			0.976			0.986			0.994	
Flt Protected		0.950			0.969			0.991			0.998	
Satd. Flow (prot)	1762	1679	0	0	1755	0	0	3066	0	0	3154	0
Flt Permitted		0.608			0.588			0.991			0.998	
Satd. Flow (perm)	1127	1679	0	0	1065	0	0	3066	0	0	3154	0
Right Turn on Red			Yes			Yes			No			Yes
Satd. Flow (RTOR)		98			9						4	
Link Speed (mph)		30			30			40			40	
Link Distance (ft)		498			709			1377			1086	
Travel Time (s)		11.3			16.1			23.5			18.5	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	4%	0%	4%	2%	0%	12%	3%	9%	4%	50%	5%	3%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	85	176	0	0	367	0	0	1326	0	0	1234	0
Turn Type	Perm	NA		Perm	NA		Split	NA		Split	NA	
Protected Phases		4			8		6	6		2	2	
Permitted Phases		4			8							
Detector Phase	4	4		8	8		6	6		2	2	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		34.0	34.0		5.0	5.0	
Minimum Split (s)	10.0	10.0		10.0	10.0		40.0	40.0		11.0	11.0	
Total Split (s)	30.0	30.0		30.0	30.0		40.0	40.0		40.0	40.0	
Total Split (%)	27.3%	27.3%		27.3%	27.3%		36.4%	36.4%		36.4%	36.4%	
Maximum Green (s)	25.0	25.0		25.0	25.0		34.0	34.0		34.0	34.0	
Yellow Time (s)	4.0	4.0		4.0	4.0		5.0	5.0		5.0	5.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)	0.0	0.0		0.0			0.0			0.0		
Total Lost Time (s)	5.0	5.0		5.0			6.0			6.0		
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	2.0	2.0		2.0	2.0		2.0	2.0		3.0	3.0	
Recall Mode	None	None		None	None		None	None		C-Max	C-Max	
Walk Time (s)	7.0	7.0								7.0	7.0	
Flash Dont Walk (s)	18.0	18.0								15.0	15.0	
Pedestrian Calls (#/hr)	0	0								0	0	
Act Effct Green (s)	25.0	25.0		25.0			34.0			34.0		
Actuated g/C Ratio	0.23	0.23		0.23			0.31			0.31		
v/c Ratio	0.33	0.39		1.47			1.40			1.26		
Control Delay	39.9	19.0		265.8			210.9			160.2		
Queue Delay	0.0	0.0		0.0			0.0			0.0		
Total Delay	39.9	19.0		265.8			210.9			160.2		
LOS	D	B		F			F			F		



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Approach Delay		25.8			265.8			210.9			160.2	
Approach LOS			C			F			F			F
Queue Length 50th (ft)	50	45			~355			~676			~580	
Queue Length 95th (ft)	99	108			#543			m#614			#718	
Internal Link Dist (ft)		418			629			1297			1006	
Turn Bay Length (ft)												
Base Capacity (vph)	256	457			249			947			977	
Starvation Cap Reductn	0	0			0			0			0	
Spillback Cap Reductn	0	0			0			0			0	
Storage Cap Reductn	0	0			0			0			0	
Reduced v/c Ratio	0.33	0.39			1.47			1.40			1.26	

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 5 (5%), Referenced to phase 2:SBTL, Start of Yellow

Natural Cycle: 150

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.47

Intersection Signal Delay: 182.5

Intersection LOS: F

Intersection Capacity Utilization 113.8%

ICU Level of Service H

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

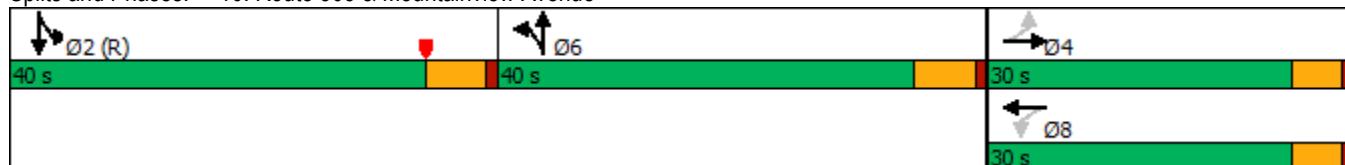
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 10: Route 303 & Mountainview Avenue



2686-99-013T

Existing - AM

20: Route 303 & Orangeburg Road (CR 20)/Driveway

	→	→	←	←	↑	↑	↑	↑	↓	↓	←	
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	193	13	246	13	9	8	236	508	2	10	490	198
Future Volume (vph)	193	13	246	13	9	8	236	508	2	10	490	198
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	12	12	12	11	11	11	11	11	11
Grade (%)	-3%				0%			-4%			-1%	
Storage Length (ft)	685			0	0		0	0		0	0	0
Storage Lanes	0			1	0		1	0		0	0	0
Taper Length (ft)	110				25			25			25	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Ped Bike Factor				0.99		1.00			1.00			1.00
Fr _t				0.850			0.850					0.957
Flt Protected				0.955			0.972			0.984		0.999
Satd. Flow (prot)	0	1686	1481	0	1847	1615	0	3182	0	0	2995	0
Flt Permitted				0.955			0.701			0.575		0.941
Satd. Flow (perm)	0	1686	1462	0	1331	1615	0	1859	0	0	2821	0
Right Turn on Red				Yes			Yes			Yes		Yes
Satd. Flow (RTOR)				262			119					65
Link Speed (mph)				30			25			40		40
Link Distance (ft)				1228			609			621		1377
Travel Time (s)				27.9			16.6			10.6		23.5
Confl. Peds. (#/hr)				1		1				4		4
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Heavy Vehicles (%)	6%	0%	7%	0%	0%	0%	6%	12%	0%	38%	11%	13%
Adj. Flow (vph)	205	14	262	14	10	9	251	540	2	11	521	211
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	219	262	0	24	9	0	793	0	0	743	0
Turn Type	Split	NA	pm+ov	Perm	NA	Perm	custom	NA		Perm	NA	
Protected Phases	4	4	6		8			6	16			5
Permitted Phases				4	8		8	1			5	
Detector Phase	4	4	4	8	8	8	6	16		5	5	
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0			45.0	45.0	
Minimum Split (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0			50.0	50.0	
Total Split (s)	30.0	30.0	15.0	15.0	15.0	15.0	15.0			50.0	50.0	
Total Split (%)	27.3%	27.3%	13.6%	13.6%	13.6%	13.6%	13.6%			45.5%	45.5%	
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0			4.0	4.0	
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0			1.0	1.0	
Lost Time Adjust (s)				0.0	0.0	0.0	0.0				0.0	
Total Lost Time (s)				5.0	5.0	5.0	5.0				5.0	
Lead/Lag				Lag			Lag			Lead	Lead	
Lead-Lag Optimize?				Yes			Yes			Yes	Yes	
Recall Mode	None			C-Max	C-Max							
Act Effct Green (s)		18.6	28.6		7.0	7.0		73.5			58.5	
Actuated g/C Ratio		0.17	0.26		0.06	0.06		0.67			0.53	
v/c Ratio		0.77	0.46		0.29	0.04		0.58			0.49	
Control Delay		60.7	5.8		56.9	0.4		15.5			21.9	
Queue Delay		0.0	0.0		0.0	0.0		0.0			0.0	
Total Delay		60.7	5.8		56.9	0.4		15.5			21.9	

Lane Group	Ø1
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Width (ft)	
Grade (%)	
Storage Length (ft)	
Storage Lanes	
Taper Length (ft)	
Lane Util. Factor	
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Confl. Peds. (#/hr)	
Peak Hour Factor	
Heavy Vehicles (%)	
Adj. Flow (vph)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Turn Type	
Protected Phases	1
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	45.0
Minimum Split (s)	50.0
Total Split (s)	65.0
Total Split (%)	59%
Yellow Time (s)	4.0
All-Red Time (s)	1.0
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Recall Mode	Max
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	

2686-99-013T

Existing - AM

20: Route 303 & Orangeburg Road (CR 20)/Driveway



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
LOS	E	A		E	A		B			C		
Approach Delay		30.8			41.5			15.5			21.9	
Approach LOS		C			D		B			C		
Queue Length 50th (ft)	149	0		17	0		89			104		
Queue Length 95th (ft)	218	53		43	0		137			219		
Internal Link Dist (ft)	1148			529			541			1297		
Turn Bay Length (ft)												
Base Capacity (vph)	383	645		121	255		1362			1530		
Starvation Cap Reductn	0	0		0	0		0			0		
Spillback Cap Reductn	0	0		0	0		0			0		
Storage Cap Reductn	0	0		0	0		0			0		
Reduced v/c Ratio	0.57	0.41		0.20	0.04		0.58			0.49		

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 0 (0%), Referenced to phase 5:SBTL, Start of Yellow

Natural Cycle: 80

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.77

Intersection Signal Delay: 21.8

Intersection LOS: C

Intersection Capacity Utilization 105.5%

ICU Level of Service G

Analysis Period (min) 15

Splits and Phases: 20: Route 303 & Orangeburg Road (CR 20)/Driveway



Lane Group	Ø1
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (ft)	
Queue Length 95th (ft)	
Internal Link Dist (ft)	
Turn Bay Length (ft)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

2686-99-013T

Existing - PM

20: Route 303 & Orangeburg Road (CR 20)/Driveway

	→	→	←	←	↑	↑	↑	↑	↓	↓	←	
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	259	16	205	23	40	12	250	650	8	4	706	297
Future Volume (vph)	259	16	205	23	40	12	250	650	8	4	706	297
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	12	12	12	11	11	11	11	11	11
Grade (%)	-3%				0%			-4%			-1%	
Storage Length (ft)	685			0	0		0	0		0	0	0
Storage Lanes	0			1	0		1	0		0	0	0
Taper Length (ft)	110				25			25			25	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Ped Bike Factor	1.00					0.99						
Fr _t			0.850			0.850		0.999			0.956	
Flt Protected		0.955				0.982		0.986				
Satd. Flow (prot)	0	1613	1538	0	1866	1615	0	3344	0	0	3230	0
Flt Permitted		0.955			0.761			0.507			0.952	
Satd. Flow (perm)	0	1610	1538	0	1446	1592	0	1719	0	0	3075	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		216				119		1			71	
Link Speed (mph)		30			25			40			40	
Link Distance (ft)		1228			609			621			1377	
Travel Time (s)		27.9			16.6			10.6			23.5	
Confl. Peds. (#/hr)	2				2							
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	11%	0%	3%	0%	0%	0%	2%	6%	0%	0%	5%	1%
Adj. Flow (vph)	273	17	216	24	42	13	263	684	8	4	743	313
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	290	216	0	66	13	0	955	0	0	1060	0
Turn Type	Split	NA	pm+ov	Perm	NA	Perm	custom	NA		Perm	NA	
Protected Phases	4	4	6		8			6	16		5	
Permitted Phases		4	8			8	1			5		
Detector Phase	4	4	4	8	8	8	6	16		5	5	
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0			45.0	45.0	
Minimum Split (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0			50.0	50.0	
Total Split (s)	30.0	30.0	15.0	15.0	15.0	15.0	15.0			50.0	50.0	
Total Split (%)	27.3%	27.3%	13.6%	13.6%	13.6%	13.6%	13.6%			45.5%	45.5%	
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0			4.0	4.0	
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0			1.0	1.0	
Lost Time Adjust (s)		0.0	0.0		0.0	0.0					0.0	
Total Lost Time (s)		5.0	5.0		5.0	5.0					5.0	
Lead/Lag			Lag				Lag			Lead	Lead	
Lead-Lag Optimize?		Yes				Yes				Yes	Yes	
Recall Mode	None			C-Max	C-Max							
Act Effct Green (s)	22.6	37.6		8.6	8.6			65.9			50.9	
Actuated g/C Ratio	0.21	0.34		0.08	0.08			0.60			0.46	
v/c Ratio	0.88	0.32		0.59	0.06			0.81			0.73	
Control Delay	68.5	4.6		69.7	0.4			27.7			36.2	
Queue Delay	0.0	0.0		0.0	0.0			0.0			0.0	
Total Delay	68.5	4.6		69.7	0.4			27.7			36.2	

Lane Group	Ø1
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Width (ft)	
Grade (%)	
Storage Length (ft)	
Storage Lanes	
Taper Length (ft)	
Lane Util. Factor	
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Confl. Peds. (#/hr)	
Peak Hour Factor	
Heavy Vehicles (%)	
Adj. Flow (vph)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Turn Type	
Protected Phases	1
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	45.0
Minimum Split (s)	50.0
Total Split (s)	65.0
Total Split (%)	59%
Yellow Time (s)	4.0
All-Red Time (s)	1.0
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Recall Mode	Max
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	

2686-99-013T

Existing - PM

20: Route 303 & Orangeburg Road (CR 20)/Driveway



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
LOS	E	A		E	A		C			D		
Approach Delay	41.3			58.3			27.7			36.2		
Approach LOS	D			E			C			D		
Queue Length 50th (ft)	194	0		45	0		151			256		
Queue Length 95th (ft)	#327	49		92	0		m#205			323		
Internal Link Dist (ft)	1148			529			541			1297		
Turn Bay Length (ft)												
Base Capacity (vph)	366	696		131	252		1177			1459		
Starvation Cap Reductn	0	0		0	0		0			0		
Spillback Cap Reductn	0	0		0	0		0			0		
Storage Cap Reductn	0	0		0	0		0			0		
Reduced v/c Ratio	0.79	0.31		0.50	0.05		0.81			0.73		

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 0 (0%), Referenced to phase 5:SBTL, Start of Yellow

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.88

Intersection Signal Delay: 34.7

Intersection LOS: C

Intersection Capacity Utilization 109.4%

ICU Level of Service H

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 20: Route 303 & Orangeburg Road (CR 20)/Driveway



Lane Group	Ø1
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (ft)	
Queue Length 95th (ft)	
Internal Link Dist (ft)	
Turn Bay Length (ft)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

2686-99-013T

No Build - AM

20: Route 303 & Orangeburg Road (CR 20)/Driveway

	→	→	←	←	↑	↑	↑	↑	↓	↓	←	
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	211	13	253	13	9	8	243	602	2	10	547	206
Future Volume (vph)	211	13	253	13	9	8	243	602	2	10	547	206
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	12	12	12	11	11	11	11	11	11
Grade (%)	-3%				0%			-4%			-1%	
Storage Length (ft)	685		0	0		0	0		0	0		0
Storage Lanes	0		1	0		1	0		0	0		0
Taper Length (ft)	110			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Ped Bike Factor			0.99			1.00			1.00			1.00
Fr			0.850			0.850						0.960
Flt Protected			0.955			0.972			0.986			0.999
Satd. Flow (prot)	0	1685	1481	0	1847	1615	0	3183	0	0	3006	0
Flt Permitted			0.955			0.691			0.564			0.939
Satd. Flow (perm)	0	1685	1462	0	1312	1615	0	1821	0	0	2825	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			269			119						58
Link Speed (mph)			30			25			40			40
Link Distance (ft)			1228			609			621			1377
Travel Time (s)			27.9			16.6			10.6			23.5
Confl. Peds. (#/hr)			1		1				4		4	
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Heavy Vehicles (%)	6%	0%	7%	0%	0%	0%	6%	12%	0%	38%	11%	13%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	238	269	0	24	9	0	901	0	0	812	0
Turn Type	Split	NA	pm+ov	Perm	NA	Perm	custom	NA		Perm	NA	
Protected Phases	4	4	6		8		6	16			5	
Permitted Phases			4	8		8	1			5		
Detector Phase	4	4	4	8	8	8	6	16		5	5	
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0			45.0	45.0	
Minimum Split (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0			50.0	50.0	
Total Split (s)	30.0	30.0	15.0	15.0	15.0	15.0	15.0			50.0	50.0	
Total Split (%)	27.3%	27.3%	13.6%	13.6%	13.6%	13.6%	13.6%	13.6%		45.5%	45.5%	
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0			4.0	4.0	
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0			1.0	1.0	
Lost Time Adjust (s)			0.0	0.0		0.0	0.0				0.0	
Total Lost Time (s)			5.0	5.0		5.0	5.0				5.0	
Lead/Lag				Lag			Lag			Lead	Lead	
Lead-Lag Optimize?			Yes			Yes				Yes	Yes	
Recall Mode	None			C-Max	C-Max							
Act Effct Green (s)		19.5	29.5		7.0	7.0		72.6			57.6	
Actuated g/C Ratio		0.18	0.27		0.06	0.06		0.66			0.52	
v/c Ratio		0.80	0.46		0.29	0.04		0.68			0.54	
Control Delay		62.2	5.6		57.1	0.4		17.7			29.2	
Queue Delay		0.0	0.0		0.0	0.0		0.0			0.0	
Total Delay		62.2	5.6		57.1	0.4		17.7			29.2	
LOS		E	A		E	A		B			C	

Lane Group	Ø1
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Width (ft)	
Grade (%)	
Storage Length (ft)	
Storage Lanes	
Taper Length (ft)	
Lane Util. Factor	
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Confl. Peds. (#/hr)	
Peak Hour Factor	
Heavy Vehicles (%)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Turn Type	
Protected Phases	1
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	45.0
Minimum Split (s)	50.0
Total Split (s)	65.0
Total Split (%)	59%
Yellow Time (s)	4.0
All-Red Time (s)	1.0
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Recall Mode	Max
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	

2686-99-013T

No Build - AM

20: Route 303 & Orangeburg Road (CR 20)/Driveway



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Approach Delay		32.2			41.7			17.7			29.2	
Approach LOS		C			D			B			C	
Queue Length 50th (ft)	162	0		17	0		88			191		
Queue Length 95th (ft)	238	54		43	0		162			m240		
Internal Link Dist (ft)	1148			529			541			1297		
Turn Bay Length (ft)												
Base Capacity (vph)	382	650		119	255		1325			1506		
Starvation Cap Reductn	0	0		0	0		0			0		
Spillback Cap Reductn	0	0		0	0		0			0		
Storage Cap Reductn	0	0		0	0		0			0		
Reduced v/c Ratio	0.62	0.41		0.20	0.04		0.68			0.54		

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 0 (0%), Referenced to phase 5:SBTL, Start of Yellow

Natural Cycle: 80

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.80

Intersection Signal Delay: 25.5

Intersection LOS: C

Intersection Capacity Utilization 106.5%

ICU Level of Service G

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 20: Route 303 & Orangeburg Road (CR 20)/Driveway



Lane Group	Ø1
Approach Delay	
Approach LOS	
Queue Length 50th (ft)	
Queue Length 95th (ft)	
Internal Link Dist (ft)	
Turn Bay Length (ft)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

2686-99-013T

No Build - PM

20: Route 303 & Orangeburg Road (CR 20)/Driveway

	→	→	←	←	↑	↑	↑	↑	↓	↓	←	
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	268	16	213	23	41	12	260	700	8	4	806	314
Future Volume (vph)	268	16	213	23	41	12	260	700	8	4	806	314
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	12	12	12	11	11	11	11	11	11
Grade (%)	-3%				0%			-4%			-1%	
Storage Length (ft)	685		0	0		0	0		0	0		0
Storage Lanes	0		1	0		1	0		0	0		0
Taper Length (ft)	110			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Ped Bike Factor	1.00					0.99						
Fr _t		0.850				0.850		0.999			0.958	
Flt Protected		0.955				0.982		0.987				
Satd. Flow (prot)	0	1613	1538	0	1866	1615	0	3346	0	0	3235	0
Flt Permitted		0.955			0.760		0.505				0.952	
Satd. Flow (perm)	0	1610	1538	0	1444	1592	0	1712	0	0	3080	0
Right Turn on Red		Yes				Yes			Yes			Yes
Satd. Flow (RTOR)		198				119		1			63	
Link Speed (mph)		30			25			40			40	
Link Distance (ft)		1228			609			621			1377	
Travel Time (s)		27.9			16.6			10.6			23.5	
Confl. Peds. (#/hr)	2				2							
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	11%	0%	3%	0%	0%	0%	2%	6%	0%	0%	5%	1%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	299	224	0	67	13	0	1019	0	0	1183	0
Turn Type	Split	NA	pm+ov	Perm	NA	Perm	custom	NA		Perm	NA	
Protected Phases	4	4	6		8		6	16			5	
Permitted Phases			4	8		8	1				5	
Detector Phase	4	4	4	8	8	8	6	16		5	5	
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0			45.0	45.0	
Minimum Split (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0			50.0	50.0	
Total Split (s)	30.0	30.0	15.0	15.0	15.0	15.0	15.0			50.0	50.0	
Total Split (%)	27.3%	27.3%	13.6%	13.6%	13.6%	13.6%	13.6%	13.6%		45.5%	45.5%	
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0			4.0	4.0	
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0			1.0	1.0	
Lost Time Adjust (s)		0.0	0.0		0.0	0.0					0.0	
Total Lost Time (s)		5.0	5.0		5.0	5.0					5.0	
Lead/Lag			Lag				Lag			Lead	Lead	
Lead-Lag Optimize?			Yes				Yes			Yes	Yes	
Recall Mode	None			C-Max	C-Max							
Act Effct Green (s)	22.9	37.9		8.6	8.6			65.5			50.5	
Actuated g/C Ratio	0.21	0.34		0.08	0.08			0.60			0.46	
v/c Ratio	0.89	0.34		0.60	0.06			1.01dl			0.82	
Control Delay	70.6	6.4		70.1	0.4			30.4			40.4	
Queue Delay		0.0	0.0		0.0	0.0		0.0			0.0	
Total Delay	70.6	6.4		70.1	0.4			30.4			40.4	
LOS	E	A		E	A			C			D	

Lane Group	Ø1
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Width (ft)	
Grade (%)	
Storage Length (ft)	
Storage Lanes	
Taper Length (ft)	
Lane Util. Factor	
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Confl. Peds. (#/hr)	
Peak Hour Factor	
Heavy Vehicles (%)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Turn Type	
Protected Phases	1
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	45.0
Minimum Split (s)	50.0
Total Split (s)	65.0
Total Split (%)	59%
Yellow Time (s)	4.0
All-Red Time (s)	1.0
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Recall Mode	Max
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	

2686-99-013T

No Build - PM

20: Route 303 & Orangeburg Road (CR 20)/Driveway



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Approach Delay		43.1			58.8			30.4			40.4	
Approach LOS		D			E			C			D	
Queue Length 50th (ft)	202		12		46	0		171			302	
Queue Length 95th (ft)	#343		64		93	0		m#231			m293	
Internal Link Dist (ft)	1148				529			541			1297	
Turn Bay Length (ft)												
Base Capacity (vph)	366	685		131	252		1168			1449		
Starvation Cap Reductn	0	0		0	0		0			0		
Spillback Cap Reductn	0	0		0	0		0			0		
Storage Cap Reductn	0	0		0	0		0			0		
Reduced v/c Ratio	0.82	0.33		0.51	0.05		0.87			0.82		

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 0 (0%), Referenced to phase 5:SBTL, Start of Yellow

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.89

Intersection Signal Delay: 37.8

Intersection LOS: D

Intersection Capacity Utilization 109.9%

ICU Level of Service H

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

dl Defacto Left Lane. Recode with 1 though lane as a left lane.

Splits and Phases: 20: Route 303 & Orangeburg Road (CR 20)/Driveway



Lane Group	Ø1
Approach Delay	
Approach LOS	
Queue Length 50th (ft)	
Queue Length 95th (ft)	
Internal Link Dist (ft)	
Turn Bay Length (ft)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

	→	→	←	←	↑	↑	↑	↑	↓	↓	←	
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	223	13	253	13	9	8	243	619	2	10	565	217
Future Volume (vph)	223	13	253	13	9	8	243	619	2	10	565	217
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	12	12	12	11	11	11	11	11	11
Grade (%)	-3%				0%			-4%			-1%	
Storage Length (ft)	685		0	0		0	0		0	0		0
Storage Lanes	0		1	0		1	0		0	0		0
Taper Length (ft)	110			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Ped Bike Factor			0.99			1.00			1.00			1.00
Fr _t			0.850			0.850						0.959
Flt Protected		0.955				0.972			0.986			0.999
Satd. Flow (prot)	0	1685	1481	0	1847	1615	0	3182	0	0	3003	0
Flt Permitted		0.955			0.685			0.557			0.940	
Satd. Flow (perm)	0	1685	1462	0	1301	1615	0	1798	0	0	2825	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		269			119						60	
Link Speed (mph)		30			25			40			40	
Link Distance (ft)		1228			609			621			1377	
Travel Time (s)		27.9			16.6			10.6			23.5	
Confl. Peds. (#/hr)		1	1					4	4			
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Heavy Vehicles (%)	6%	0%	7%	0%	0%	0%	6%	12%	0%	38%	11%	13%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	251	269	0	24	9	0	920	0	0	843	0
Turn Type	Split	NA	pm+ov	Perm	NA	Perm	custom	NA		Perm	NA	
Protected Phases	4	4	6		8		6	16			5	
Permitted Phases			4	8		8	1			5		
Detector Phase	4	4	4	8	8	8	6	16		5	5	
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0			45.0	45.0	
Minimum Split (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0			50.0	50.0	
Total Split (s)	30.0	30.0	15.0	15.0	15.0	15.0	15.0			50.0	50.0	
Total Split (%)	27.3%	27.3%	13.6%	13.6%	13.6%	13.6%	13.6%	13.6%		45.5%	45.5%	
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0			4.0	4.0	
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0			1.0	1.0	
Lost Time Adjust (s)		0.0	0.0		0.0	0.0				0.0		
Total Lost Time (s)		5.0	5.0		5.0	5.0				5.0		
Lead/Lag			Lag			Lag			Lead	Lead		
Lead-Lag Optimize?			Yes			Yes			Yes	Yes		
Recall Mode	None		C-Max	C-Max								
Act Effct Green (s)	20.2	30.2		7.0	7.0			72.0			57.0	
Actuated g/C Ratio	0.18	0.27		0.06	0.06		0.65			0.52		
v/c Ratio	0.81	0.45		0.29	0.04		0.71			0.57		
Control Delay	63.2	5.5		57.3	0.4		19.0			30.5		
Queue Delay		0.0	0.0		0.0	0.0		0.0			0.0	
Total Delay	63.2	5.5		57.3	0.4		19.0			30.5		
LOS	E	A		E	A		B			C		

Lane Group	Ø1
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Width (ft)	
Grade (%)	
Storage Length (ft)	
Storage Lanes	
Taper Length (ft)	
Lane Util. Factor	
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Confl. Peds. (#/hr)	
Peak Hour Factor	
Heavy Vehicles (%)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Turn Type	
Protected Phases	1
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	45.0
Minimum Split (s)	50.0
Total Split (s)	65.0
Total Split (%)	59%
Yellow Time (s)	4.0
All-Red Time (s)	1.0
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Recall Mode	Max
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Approach Delay		33.4			41.7			19.0			30.5	
Approach LOS		C			D			B			C	
Queue Length 50th (ft)	171	0		17	0		93			200		
Queue Length 95th (ft)	251	54		43	0		168			m248		
Internal Link Dist (ft)	1148			529			541			1297		
Turn Bay Length (ft)												
Base Capacity (vph)	382	650		118	255		1302			1491		
Starvation Cap Reductn	0	0		0	0		0			0		
Spillback Cap Reductn	0	0		0	0		0			0		
Storage Cap Reductn	0	0		0	0		0			0		
Reduced v/c Ratio	0.66	0.41		0.20	0.04		0.71			0.57		

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 0 (0%), Referenced to phase 5:SBTL, Start of Yellow

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.81

Intersection Signal Delay: 26.7

Intersection LOS: C

Intersection Capacity Utilization 107.2%

ICU Level of Service G

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 20: Route 303 & Orangeburg Road (CR 20)/Driveway



Lane Group	Ø1
Approach Delay	
Approach LOS	
Queue Length 50th (ft)	
Queue Length 95th (ft)	
Internal Link Dist (ft)	
Turn Bay Length (ft)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

	→	→	←	←	↔	↔	↑	↑	↓	↓	↗	
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	282	16	213	23	41	12	260	722	8	4	815	321
Future Volume (vph)	282	16	213	23	41	12	260	722	8	4	815	321
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	12	12	12	11	11	11	11	11	11
Grade (%)	-3%				0%			-4%			-1%	
Storage Length (ft)	685		0	0		0	0		0	0		0
Storage Lanes	0		1	0		1	0		0	0		0
Taper Length (ft)	110			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Ped Bike Factor	1.00					0.99						
Fr _t		0.850				0.850		0.999			0.958	
Flt Protected		0.955				0.982		0.987				
Satd. Flow (prot)	0	1613	1538	0	1866	1615	0	3346	0	0	3235	0
Flt Permitted		0.955			0.754			0.506			0.952	
Satd. Flow (perm)	0	1609	1538	0	1433	1592	0	1715	0	0	3080	0
Right Turn on Red		Yes				Yes			Yes			Yes
Satd. Flow (RTOR)		196				119		1			64	
Link Speed (mph)		30			25			40			40	
Link Distance (ft)		1228			609			621			1377	
Travel Time (s)		27.9			16.6			10.6			23.5	
Confl. Peds. (#/hr)	2				2							
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	11%	0%	3%	0%	0%	0%	2%	6%	0%	0%	5%	1%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	314	224	0	67	13	0	1042	0	0	1200	0
Turn Type	Split	NA	pm+ov	Perm	NA	Perm	custom	NA		Perm	NA	
Protected Phases	4	4	6		8		6	16			5	
Permitted Phases			4	8		8	1				5	
Detector Phase	4	4	6	8	8	8	6	16		5	5	
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0			45.0	45.0	
Minimum Split (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0			50.0	50.0	
Total Split (s)	30.0	30.0	15.0	15.0	15.0	15.0	15.0			50.0	50.0	
Total Split (%)	27.3%	27.3%	13.6%	13.6%	13.6%	13.6%	13.6%			45.5%	45.5%	
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0			4.0	4.0	
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0			1.0	1.0	
Lost Time Adjust (s)		0.0	0.0		0.0	0.0					0.0	
Total Lost Time (s)		5.0	5.0		5.0	5.0					5.0	
Lead/Lag			Lag				Lag			Lead	Lead	
Lead-Lag Optimize?			Yes				Yes			Yes	Yes	
Recall Mode	None			C-Max	C-Max							
Act Effct Green (s)	23.6	38.6		8.6	8.6			64.9			49.9	
Actuated g/C Ratio	0.21	0.35		0.08	0.08			0.59			0.45	
v/c Ratio	0.91	0.34		0.60	0.06			1.05dl			0.84	
Control Delay	72.9	6.5		70.6	0.4			32.6			40.9	
Queue Delay		0.0	0.0		0.0	0.0		0.0			0.0	
Total Delay	72.9	6.5		70.6	0.4			32.6			40.9	
LOS	E	A		E	A			C			D	

Lane Group	Ø1
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Width (ft)	
Grade (%)	
Storage Length (ft)	
Storage Lanes	
Taper Length (ft)	
Lane Util. Factor	
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Confl. Peds. (#/hr)	
Peak Hour Factor	
Heavy Vehicles (%)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Turn Type	
Protected Phases	1
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	45.0
Minimum Split (s)	50.0
Total Split (s)	65.0
Total Split (%)	59%
Yellow Time (s)	4.0
All-Red Time (s)	1.0
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Recall Mode	Max
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Approach Delay		45.2			59.2			32.6			40.9	
Approach LOS		D			E			C			D	
Queue Length 50th (ft)	214		13		46	0		180			306	
Queue Length 95th (ft)	#368		65		93	0		m#243			m293	
Internal Link Dist (ft)	1148				529			541			1297	
Turn Bay Length (ft)												
Base Capacity (vph)	366	666		130	252		1160			1432		
Starvation Cap Reductn	0	0		0	0		0			0		
Spillback Cap Reductn	0	0		0	0		0			0		
Storage Cap Reductn	0	0		0	0		0			0		
Reduced v/c Ratio	0.86	0.34		0.52	0.05		0.90			0.84		

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 0 (0%), Referenced to phase 5:SBTL, Start of Yellow

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.91

Intersection Signal Delay: 39.2

Intersection LOS: D

Intersection Capacity Utilization 110.6%

ICU Level of Service H

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

dl Defacto Left Lane. Recode with 1 though lane as a left lane.

Splits and Phases: 20: Route 303 & Orangeburg Road (CR 20)/Driveway



Lane Group	Ø1
Approach Delay	
Approach LOS	
Queue Length 50th (ft)	
Queue Length 95th (ft)	
Internal Link Dist (ft)	
Turn Bay Length (ft)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

2686-99-013T

Design Horizon Year No Build - AM

20: Route 303 & Orangeburg Road (CR 20)/Driveway

	→	→	←	←	↑	↑	↑	↑	↓	↓	←	
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	257	13	310	13	9	8	297	736	2	11	669	252
Future Volume (vph)	257	13	310	13	9	8	297	736	2	11	669	252
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	12	12	12	11	11	11	11	11	11
Grade (%)	-3%				0%			-4%			-1%	
Storage Length (ft)	685		0	0		0	0		0	0		0
Storage Lanes	0		1	0		1	0		0	0		0
Taper Length (ft)	110			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Ped Bike Factor			0.99			1.00			1.00			1.00
Fr _t			0.850			0.850						0.959
Flt Protected		0.955				0.972			0.986			0.999
Satd. Flow (prot)	0	1684	1481	0	1847	1615	0	3183	0	0	3004	0
Flt Permitted		0.955			0.667			0.522			0.936	
Satd. Flow (perm)	0	1684	1462	0	1267	1615	0	1685	0	0	2814	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		252			119						58	
Link Speed (mph)		30			25			40			40	
Link Distance (ft)		1228			609			621			1377	
Travel Time (s)		27.9			16.6			10.6			23.5	
Confl. Peds. (#/hr)		1	1					4	4			
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Heavy Vehicles (%)	6%	0%	7%	0%	0%	0%	6%	12%	0%	38%	11%	13%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	287	330	0	24	9	0	1101	0	0	992	0
Turn Type	Split	NA	pm+ov	Perm	NA	Perm	custom	NA		Perm	NA	
Protected Phases	4	4	6		8		6	16			5	
Permitted Phases			4	8		8	1			5		
Detector Phase	4	4	4	8	8	8	6	16		5	5	
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0			45.0	45.0	
Minimum Split (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0			50.0	50.0	
Total Split (s)	30.0	30.0	15.0	15.0	15.0	15.0	15.0			50.0	50.0	
Total Split (%)	27.3%	27.3%	13.6%	13.6%	13.6%	13.6%	13.6%	13.6%		45.5%	45.5%	
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0			4.0	4.0	
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0			1.0	1.0	
Lost Time Adjust (s)		0.0	0.0		0.0	0.0				0.0		
Total Lost Time (s)		5.0	5.0		5.0	5.0				5.0		
Lead/Lag			Lag			Lag			Lead	Lead		
Lead-Lag Optimize?			Yes			Yes			Yes	Yes		
Recall Mode	None		C-Max	C-Max								
Act Effct Green (s)	21.9	31.9		7.0	7.0			70.1			55.1	
Actuated g/C Ratio	0.20	0.29		0.06	0.06		0.64			0.50		
v/c Ratio	0.86	0.55		0.30	0.04		0.91			0.69		
Control Delay	65.8	10.7		57.8	0.4		29.8			35.4		
Queue Delay		0.0	0.0		0.0	0.0		0.0		0.0		
Total Delay	65.8	10.7		57.8	0.4		29.8			35.4		
LOS	E	B		E	A		C			D		

Lane Group	Ø1
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Width (ft)	
Grade (%)	
Storage Length (ft)	
Storage Lanes	
Taper Length (ft)	
Lane Util. Factor	
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Confl. Peds. (#/hr)	
Peak Hour Factor	
Heavy Vehicles (%)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Turn Type	
Protected Phases	1
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	45.0
Minimum Split (s)	50.0
Total Split (s)	65.0
Total Split (%)	59%
Yellow Time (s)	4.0
All-Red Time (s)	1.0
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Recall Mode	Max
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Approach Delay		36.3			42.1			29.8			35.4	
Approach LOS		D			D			C			D	
Queue Length 50th (ft)	193		37		17	0		192			245	
Queue Length 95th (ft)	#309		112		43	0		#381			m253	
Internal Link Dist (ft)	1148				529			541			1297	
Turn Bay Length (ft)												
Base Capacity (vph)	382	638		115	255		1210			1439		
Starvation Cap Reductn	0	0		0	0		0			0		
Spillback Cap Reductn	0	0		0	0		0			0		
Storage Cap Reductn	0	0		0	0		0			0		
Reduced v/c Ratio	0.75	0.52		0.21	0.04		0.91			0.69		

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 0 (0%), Referenced to phase 5:SBTL, Start of Yellow

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.91

Intersection Signal Delay: 33.4

Intersection LOS: C

Intersection Capacity Utilization 109.1%

ICU Level of Service H

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 20: Route 303 & Orangeburg Road (CR 20)/Driveway



Lane Group	Ø1
Approach Delay	
Approach LOS	
Queue Length 50th (ft)	
Queue Length 95th (ft)	
Internal Link Dist (ft)	
Turn Bay Length (ft)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

2686-99-013T

Design Horizon Year No Build - PM

20: Route 303 & Orangeburg Road (CR 20)/Driveway

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Configurations												
Traffic Volume (vph)	328	16	262	24	41	13	318	856	8	5	986	384
Future Volume (vph)	328	16	262	24	41	13	318	856	8	5	986	384
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	12	12	12	11	11	11	11	11	11
Grade (%)	-3%				0%			-4%			-1%	
Storage Length (ft)	685			0	0		0	0		0	0	0
Storage Lanes	0			1	0		1	0		0	0	0
Taper Length (ft)	110				25			25			25	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Ped Bike Factor		1.00				0.99						
Fr1			0.850			0.850		0.999			0.958	
Flt Protected		0.955				0.982		0.987				
Satd. Flow (prot)	0	1611	1538	0	1866	1615	0	3346	0	0	3235	0
Flt Permitted		0.955			0.731			0.523			0.950	
Satd. Flow (perm)	0	1608	1538	0	1389	1592	0	1773	0	0	3073	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		169				119		1			63	
Link Speed (mph)		30			25			40			40	
Link Distance (ft)		1228			609			621			1377	
Travel Time (s)		27.9			16.6			10.6			23.5	
Confl. Peds. (#/hr)	2				2							
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	11%	0%	3%	0%	0%	0%	2%	6%	0%	0%	5%	1%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	362	276	0	68	14	0	1244	0	0	1447	0
Turn Type	Split	NA	pm+ov	Perm	NA	Perm	custom	NA		Perm	NA	
Protected Phases	4	4	6		8		6	16			5	
Permitted Phases			4	8		8	1				5	
Detector Phase	4	4	6	8	8	8	6	16		5	5	
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0			45.0	45.0	
Minimum Split (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0			50.0	50.0	
Total Split (s)	30.0	30.0	15.0	15.0	15.0	15.0	15.0			50.0	50.0	
Total Split (%)	27.3%	27.3%	13.6%	13.6%	13.6%	13.6%	13.6%			45.5%	45.5%	
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0			4.0	4.0	
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0			1.0	1.0	
Lost Time Adjust (s)		0.0	0.0		0.0	0.0					0.0	
Total Lost Time (s)		5.0	5.0		5.0	5.0					5.0	
Lead/Lag			Lag			Lag			Lead	Lead		
Lead-Lag Optimize?			Yes			Yes			Yes	Yes		
Recall Mode	None		C-Max	C-Max								
Act Effct Green (s)	25.0	40.0		8.7	8.7		63.4			48.4		
Actuated g/C Ratio	0.23	0.36		0.08	0.08		0.58			0.44		
v/c Ratio	0.99	0.41		0.62	0.06		1.48dl			1.04		
Control Delay	87.8	12.0		72.7	0.5		65.2			64.3		
Queue Delay		0.0	0.0		0.0	0.0		0.0			0.0	
Total Delay	87.8	12.0		72.7	0.5		65.2			64.3		
LOS	F	B		E	A		E			E		

Lane Group	Ø1
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Width (ft)	
Grade (%)	
Storage Length (ft)	
Storage Lanes	
Taper Length (ft)	
Lane Util. Factor	
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Confl. Peds. (#/hr)	
Peak Hour Factor	
Heavy Vehicles (%)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Turn Type	
Protected Phases	1
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	45.0
Minimum Split (s)	50.0
Total Split (s)	65.0
Total Split (%)	59%
Yellow Time (s)	4.0
All-Red Time (s)	1.0
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Recall Mode	Max
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Approach Delay		55.0			60.3			65.2			64.3	
Approach LOS		E			E			E			E	
Queue Length 50th (ft)	256	51		47	0		~341			~434		
Queue Length 95th (ft)	#448	122		#100	0		m#364			m296		
Internal Link Dist (ft)	1148			529			541			1297		
Turn Bay Length (ft)												
Base Capacity (vph)	366	666		126	252		1165			1386		
Starvation Cap Reductn	0	0		0	0		0			0		
Spillback Cap Reductn	0	0		0	0		0			0		
Storage Cap Reductn	0	0		0	0		0			0		
Reduced v/c Ratio	0.99	0.41		0.54	0.06		1.07			1.04		

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 0 (0%), Referenced to phase 5:SBTL, Start of Yellow

Natural Cycle: 130

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.07

Intersection Signal Delay: 62.8

Intersection LOS: E

Intersection Capacity Utilization 115.4%

ICU Level of Service H

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

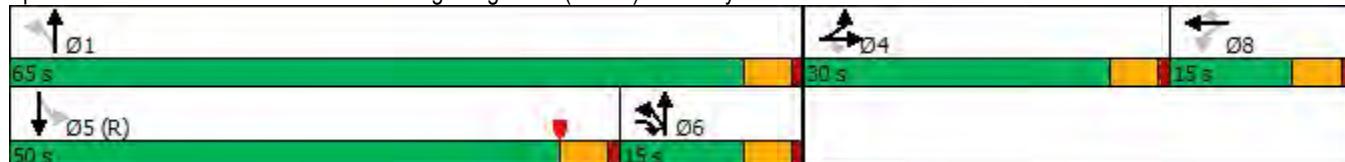
95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

dl Defacto Left Lane. Recode with 1 though lane as a left lane.

Splits and Phases: 20: Route 303 & Orangeburg Road (CR 20)/Driveway



Lane Group	Ø1
Approach Delay	
Approach LOS	
Queue Length 50th (ft)	
Queue Length 95th (ft)	
Internal Link Dist (ft)	
Turn Bay Length (ft)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Configurations												
Traffic Volume (vph)	269	13	310	13	9	8	297	753	2	11	687	263
Future Volume (vph)	269	13	310	13	9	8	297	753	2	11	687	263
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	12	12	12	11	11	11	11	11	11
Grade (%)	-3%				0%			-4%			-1%	
Storage Length (ft)	685			0	0		0	0		0	0	0
Storage Lanes	0			1	0		1	0		0	0	0
Taper Length (ft)	110				25			25			25	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Ped Bike Factor				0.99		1.00			1.00			1.00
Fr				0.850			0.850					0.959
Flt Protected				0.955			0.972			0.986		0.999
Satd. Flow (prot)	0	1684	1481	0	1847	1615	0	3182	0	0	3004	0
Flt Permitted				0.955			0.660			0.516		0.936
Satd. Flow (perm)	0	1684	1462	0	1253	1615	0	1665	0	0	2814	0
Right Turn on Red				Yes			Yes			Yes		Yes
Satd. Flow (RTOR)				247			119					60
Link Speed (mph)				30			25			40		40
Link Distance (ft)				1228			609			621		1377
Travel Time (s)				27.9			16.6			10.6		23.5
Confl. Peds. (#/hr)				1		1				4		4
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Heavy Vehicles (%)	6%	0%	7%	0%	0%	0%	6%	12%	0%	38%	11%	13%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	300	330	0	24	9	0	1119	0	0	1023	0
Turn Type	Split	NA	pm+ov	Perm	NA	Perm	custom	NA		Perm	NA	
Protected Phases	4	4	6		8		6	16			5	
Permitted Phases				4	8		8	1			5	
Detector Phase	4	4	4	8	8	8	6	16		5	5	
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0			45.0	45.0	
Minimum Split (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0			50.0	50.0	
Total Split (s)	30.0	30.0	15.0	15.0	15.0	15.0	15.0			50.0	50.0	
Total Split (%)	27.3%	27.3%	13.6%	13.6%	13.6%	13.6%	13.6%	13.6%		45.5%	45.5%	
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0			4.0	4.0	
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0			1.0	1.0	
Lost Time Adjust (s)				0.0	0.0	0.0	0.0				0.0	
Total Lost Time (s)				5.0	5.0	5.0	5.0				5.0	
Lead/Lag				Lag			Lag			Lead	Lead	
Lead-Lag Optimize?				Yes			Yes			Yes	Yes	
Recall Mode	None			C-Max	C-Max							
Act Effct Green (s)	22.5	32.5		7.0	7.0			69.6			54.6	
Actuated g/C Ratio	0.20	0.30		0.06	0.06		0.63				0.50	
v/c Ratio	0.87	0.54		0.30	0.04		0.96dl				0.72	
Control Delay	67.3	10.9		58.1	0.4		33.7				35.5	
Queue Delay				0.0	0.0		0.0				0.0	
Total Delay	67.3	10.9		58.1	0.4		33.7				35.5	
LOS	E	B		E	A		C			D		

Lane Group	Ø1
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Width (ft)	
Grade (%)	
Storage Length (ft)	
Storage Lanes	
Taper Length (ft)	
Lane Util. Factor	
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Confl. Peds. (#/hr)	
Peak Hour Factor	
Heavy Vehicles (%)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Turn Type	
Protected Phases	1
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	45.0
Minimum Split (s)	50.0
Total Split (s)	65.0
Total Split (%)	59%
Yellow Time (s)	4.0
All-Red Time (s)	1.0
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Recall Mode	Max
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Approach Delay		37.8			42.4			33.7			35.5	
Approach LOS		D			D			C			D	
Queue Length 50th (ft)	201	39		17	0		~127			254		
Queue Length 95th (ft)	#332	116		43	0		#462			m251		
Internal Link Dist (ft)	1148			529			541			1297		
Turn Bay Length (ft)												
Base Capacity (vph)	382	635		113	255		1190			1426		
Starvation Cap Reductn	0	0		0	0		0			0		
Spillback Cap Reductn	0	0		0	0		0			0		
Storage Cap Reductn	0	0		0	0		0			0		
Reduced v/c Ratio	0.79	0.52		0.21	0.04		0.94			0.72		

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 0 (0%), Referenced to phase 5:SBTL, Start of Yellow

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.94

Intersection Signal Delay: 35.4

Intersection LOS: D

Intersection Capacity Utilization 109.8%

ICU Level of Service H

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

dl Defacto Left Lane. Recode with 1 though lane as a left lane.

Splits and Phases: 20: Route 303 & Orangeburg Road (CR 20)/Driveway



Lane Group	Ø1
Approach Delay	
Approach LOS	
Queue Length 50th (ft)	
Queue Length 95th (ft)	
Internal Link Dist (ft)	
Turn Bay Length (ft)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

2686-99-013T

Design Horizon Year Build - PM
20: Route 303 & Orangeburg Road (CR 20)/Driveway

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	342	16	262	24	41	13	318	878	8	5	995	391
Future Volume (vph)	342	16	262	24	41	13	318	878	8	5	995	391
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	12	12	12	11	11	11	11	11	11
Grade (%)	-3%				0%			-4%			-1%	
Storage Length (ft)	685			0	0		0	0		0	0	0
Storage Lanes	0			1	0		1	0		0	0	0
Taper Length (ft)	110				25			25			25	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Ped Bike Factor		1.00				0.99						
Fr _t			0.850			0.850		0.999			0.958	
Flt Protected		0.954				0.982		0.987				
Satd. Flow (prot)	0	1609	1538	0	1866	1615	0	3346	0	0	3235	0
Flt Permitted		0.954			0.725			0.524			0.950	
Satd. Flow (perm)	0	1606	1538	0	1377	1592	0	1776	0	0	3073	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			168			119		1			64	
Link Speed (mph)		30			25			40			40	
Link Distance (ft)		1228			609			621			1377	
Travel Time (s)		27.9			16.6			10.6			23.5	
Confl. Peds. (#/hr)	2				2							
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	11%	0%	3%	0%	0%	0%	2%	6%	0%	0%	5%	1%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	377	276	0	68	14	0	1267	0	0	1464	0
Turn Type	Split	NA	pm+ov	Perm	NA	Perm	custom	NA		Perm	NA	
Protected Phases	4	4	6		8		6	16			5	
Permitted Phases			4	8		8	1			5		
Detector Phase	4	4	4	8	8	8	6	16		5	5	
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0			45.0	45.0	
Minimum Split (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0			50.0	50.0	
Total Split (s)	30.0	30.0	15.0	15.0	15.0	15.0	15.0			50.0	50.0	
Total Split (%)	27.3%	27.3%	13.6%	13.6%	13.6%	13.6%	13.6%	13.6%		45.5%	45.5%	
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0			4.0	4.0	
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0			1.0	1.0	
Lost Time Adjust (s)		0.0	0.0		0.0	0.0					0.0	
Total Lost Time (s)		5.0	5.0		5.0	5.0					5.0	
Lead/Lag			Lag			Lag			Lead	Lead		
Lead-Lag Optimize?			Yes			Yes			Yes	Yes		
Recall Mode	None		C-Max	C-Max								
Act Effct Green (s)		25.0	40.0		8.7	8.7		63.4			48.4	
Actuated g/C Ratio		0.23	0.36		0.08	0.08		0.58			0.44	
v/c Ratio		1.03	0.41		0.62	0.06		1.48dl			1.06	
Control Delay		98.3	12.1		73.2	0.5		72.2			68.9	
Queue Delay		0.0	0.0		0.0	0.0		0.0			0.0	
Total Delay		98.3	12.1		73.2	0.5		72.2			68.9	
LOS		F	B		E	A		E			E	

Lane Group	Ø1
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Width (ft)	
Grade (%)	
Storage Length (ft)	
Storage Lanes	
Taper Length (ft)	
Lane Util. Factor	
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Confl. Peds. (#/hr)	
Peak Hour Factor	
Heavy Vehicles (%)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Turn Type	
Protected Phases	1
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	45.0
Minimum Split (s)	50.0
Total Split (s)	65.0
Total Split (%)	59%
Yellow Time (s)	4.0
All-Red Time (s)	1.0
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Recall Mode	Max
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Approach Delay		61.9			60.8			72.2			68.9	
Approach LOS		E			E			E			E	
Queue Length 50th (ft)		~286	52		47	0		~407			~444	
Queue Length 95th (ft)		#473	122		#101	0		m#391			m296	
Internal Link Dist (ft)		1148			529			541			1297	
Turn Bay Length (ft)												
Base Capacity (vph)	365	666		125	252		1165			1386		
Starvation Cap Reductn	0	0		0	0		0			0		
Spillback Cap Reductn	0	0		0	0		0			0		
Storage Cap Reductn	0	0		0	0		0			0		
Reduced v/c Ratio	1.03	0.41		0.54	0.06		1.09			1.06		

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 0 (0%), Referenced to phase 5:SBTL, Start of Yellow

Natural Cycle: 130

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.09

Intersection Signal Delay: 68.6

Intersection LOS: E

Intersection Capacity Utilization 116.6%

ICU Level of Service H

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

dl Defacto Left Lane. Recode with 1 though lane as a left lane.

Splits and Phases: 20: Route 303 & Orangeburg Road (CR 20)/Driveway



Lane Group	Ø1
Approach Delay	
Approach LOS	
Queue Length 50th (ft)	
Queue Length 95th (ft)	
Internal Link Dist (ft)	
Turn Bay Length (ft)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

2686-99-013T

Existing - AM

30: Route 303 & South Greenbush Road/Route 340

	↗	→	↘	↙	←	↖	↑	↗	↘	↓	↙	
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	25	28	7	58	30	207	6	514	63	233	496	20
Future Volume (vph)	25	28	7	58	30	207	6	514	63	233	496	20
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	16	16	16	12	12	12	11	11	11	11	11	11
Grade (%)	-2%				-3%			-3%			-1%	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Ped Bike Factor									1.00		1.00	
Frt		0.985				0.905			0.984			0.996
Flt Protected		0.979				0.990						0.985
Satd. Flow (prot)	0	1871	0	0	1617	0	0	3141	0	0	3139	0
Flt Permitted		0.620				0.927			0.948			0.602
Satd. Flow (perm)	0	1185	0	0	1514	0	0	2978	0	0	1918	0
Right Turn on Red			Yes				Yes			Yes		No
Satd. Flow (RTOR)		6				113			14			
Link Speed (mph)		30				40			40			40
Link Distance (ft)		280				324			945			621
Travel Time (s)		6.4				5.5			16.1			10.6
Confl. Peds. (#/hr)								1				1
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	21%	7%	0%	12%	3%	6%	17%	11%	10%	8%	10%	17%
Adj. Flow (vph)	26	29	7	61	32	218	6	541	66	245	522	21
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	62	0	0	311	0	0	613	0	0	788	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		custom	NA	
Protected Phases		4				8			2		1	1 6
Permitted Phases	4				8			2			6	
Detector Phase	4	4		8	8			2	2		1	1 6
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		45.0	45.0			5.0	
Minimum Split (s)	10.0	10.0		10.0	10.0		50.0	50.0			10.0	
Total Split (s)	40.0	40.0		40.0	40.0		50.0	50.0			20.0	
Total Split (%)	36.4%	36.4%		36.4%	36.4%		45.5%	45.5%			18.2%	
Yellow Time (s)	4.0	4.0		4.0	4.0		4.0	4.0			4.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0			1.0	
Lost Time Adjust (s)		0.0			0.0			0.0				
Total Lost Time (s)		5.0			5.0			5.0				
Lead/Lag							Lag	Lag		Lead		
Lead-Lag Optimize?							Yes	Yes		Yes		
Recall Mode	None	None		None	None		C-Min	C-Min		None		
Act Effect Green (s)	20.1			20.1				61.7			79.9	
Actuated g/C Ratio	0.18			0.18				0.56			0.73	
v/c Ratio	0.28			0.84				0.37			0.51	
Control Delay	35.4			46.7				15.5			9.1	
Queue Delay	0.0			0.0				0.0			0.0	
Total Delay	35.4			46.7				15.5			9.1	
LOS	D			D				B			A	
Approach Delay	35.4			46.7				15.5			9.1	
Approach LOS	D			D				B			A	

Lane Group	Ø6
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Width (ft)	
Grade (%)	
Lane Util. Factor	
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Confl. Peds. (#/hr)	
Peak Hour Factor	
Heavy Vehicles (%)	
Adj. Flow (vph)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Turn Type	
Protected Phases	6
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	45.0
Minimum Split (s)	50.0
Total Split (s)	70.0
Total Split (%)	64%
Yellow Time (s)	4.0
All-Red Time (s)	1.0
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Recall Mode	C-Min
Act Effect Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	

2686-99-013T

Existing - AM

30: Route 303 & South Greenbush Road/Route 340



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Queue Length 50th (ft)	34			139			121			105		
Queue Length 95th (ft)	66			220			197			107		
Internal Link Dist (ft)	200			244			865			541		
Turn Bay Length (ft)												
Base Capacity (vph)	381			558			1675			1559		
Starvation Cap Reductn	0			0			0			0		
Spillback Cap Reductn	0			0			0			0		
Storage Cap Reductn	0			0			0			0		
Reduced v/c Ratio	0.16			0.56			0.37			0.51		

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBTL, Start of Yellow

Natural Cycle: 75

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.84

Intersection Signal Delay: 18.8

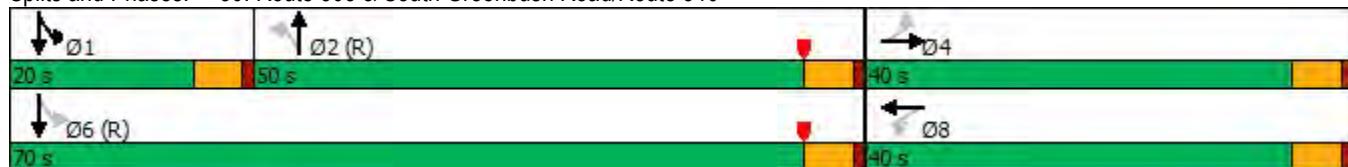
Intersection LOS: B

Intersection Capacity Utilization 90.1%

ICU Level of Service E

Analysis Period (min) 15

Splits and Phases: 30: Route 303 & South Greenbush Road/Route 340



Lane Group	Ø6
Queue Length 50th (ft)	
Queue Length 95th (ft)	
Internal Link Dist (ft)	
Turn Bay Length (ft)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

2686-99-013T

Existing - PM

30: Route 303 & South Greenbush Road/Route 340

	→	→	←	←	↑	↑	↓	↓	↗	↙	↖	↙
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	25	26	11	78	52	330	13	553	49	211	701	22
Future Volume (vph)	25	26	11	78	52	330	13	553	49	211	701	22
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	16	16	16	12	12	12	11	11	11	11	11	11
Grade (%)	-2%			-3%			-3%			-1%		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Frt		0.976			0.903			0.988			0.996	
Flt Protected		0.981			0.992			0.999			0.989	
Satd. Flow (prot)	0	2050	0	0	1654	0	0	3369	0	0	3310	0
Flt Permitted		0.681			0.928			0.923			0.589	
Satd. Flow (perm)	0	1423	0	0	1547	0	0	3113	0	0	1971	0
Right Turn on Red			Yes			Yes			Yes			No
Satd. Flow (RTOR)		10			122			10				
Link Speed (mph)		30			40			40			40	
Link Distance (ft)		280			324			945			621	
Travel Time (s)		6.4			5.5			16.1			10.6	
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Heavy Vehicles (%)	4%	0%	0%	1%	6%	5%	0%	4%	2%	4%	4%	20%
Adj. Flow (vph)	27	29	12	86	57	363	14	608	54	232	770	24
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	68	0	0	506	0	0	676	0	0	1026	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		custom	NA	
Protected Phases		4			8			2		1	16	
Permitted Phases	4			8			2			6		
Detector Phase	4	4		8	8		2	2		1	16	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		45.0	45.0		5.0		
Minimum Split (s)	10.0	10.0		10.0	10.0		50.0	50.0		10.0		
Total Split (s)	40.0	40.0		40.0	40.0		50.0	50.0		20.0		
Total Split (%)	36.4%	36.4%		36.4%	36.4%		45.5%	45.5%		18.2%		
Yellow Time (s)	4.0	4.0		4.0	4.0		4.0	4.0		4.0		
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0		
Lost Time Adjust (s)		0.0			0.0			0.0				
Total Lost Time (s)		5.0			5.0			5.0				
Lead/Lag							Lag	Lag		Lead		
Lead-Lag Optimize?							Yes	Yes		Yes		
Recall Mode	None	None		None	None		C-Min	C-Min		None		
Act Effct Green (s)		32.0			32.0			48.0			68.0	
Actuated g/C Ratio		0.29			0.29			0.44			0.62	
v/c Ratio		0.16			0.94			0.50			0.73	
Control Delay		24.5			56.0			24.2			18.3	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		24.5			56.0			24.2			18.3	
LOS		C			E			C			B	
Approach Delay		24.5			56.0			24.2			18.3	
Approach LOS		C			E			C			B	
Queue Length 50th (ft)		29			266			183			123	
Queue Length 95th (ft)		63			#466			240			201	

Lane Group	Ø6
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Width (ft)	
Grade (%)	
Lane Util. Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Peak Hour Factor	
Heavy Vehicles (%)	
Adj. Flow (vph)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Turn Type	
Protected Phases	6
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	45.0
Minimum Split (s)	50.0
Total Split (s)	70.0
Total Split (%)	64%
Yellow Time (s)	4.0
All-Red Time (s)	1.0
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Recall Mode	C-Min
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (ft)	
Queue Length 95th (ft)	

2686-99-013T

Existing - PM

30: Route 303 & South Greenbush Road/Route 340



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Internal Link Dist (ft)		200			244			865			541	
Turn Bay Length (ft)												
Base Capacity (vph)	459				575			1363			1400	
Starvation Cap Reductn	0				0			0			0	
Spillback Cap Reductn	0				0			0			0	
Storage Cap Reductn	0				0			0			0	
Reduced v/c Ratio	0.15				0.88			0.50			0.73	

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBTL, Start of Yellow

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.94

Intersection Signal Delay: 28.6

Intersection LOS: C

Intersection Capacity Utilization 105.5%

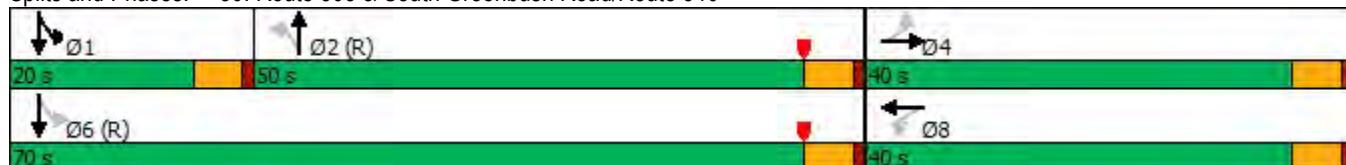
ICU Level of Service G

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 30: Route 303 & South Greenbush Road/Route 340



Lane Group	Ø6
Internal Link Dist (ft)	
Turn Bay Length (ft)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

2686-99-013T

No Build - AM

30: Route 303 & South Greenbush Road/Route 340

	↗	→	↘	↙	←	↖	↑	↗	↘	↓	↙	
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	26	30	7	59	31	227	7	594	64	248	545	20
Future Volume (vph)	26	30	7	59	31	227	7	594	64	248	545	20
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	16	16	16	12	12	12	11	11	11	11	11	11
Grade (%)	-2%				-3%			-3%			-1%	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Ped Bike Factor											1.00	
Frt		0.986				0.903			0.986			0.996
Flt Protected		0.980				0.991			0.999			0.985
Satd. Flow (prot)	0	1877	0	0	1616	0	0	3144	0	0	3139	0
Flt Permitted		0.621				0.929			0.947			0.578
Satd. Flow (perm)	0	1189	0	0	1515	0	0	2981	0	0	1842	0
Right Turn on Red			Yes				Yes			Yes		No
Satd. Flow (RTOR)		6				121			12			
Link Speed (mph)		30				40			40			40
Link Distance (ft)		280				324			945			621
Travel Time (s)		6.4				5.5			16.1			10.6
Confl. Peds. (#/hr)								1				1
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	21%	7%	0%	12%	3%	6%	17%	11%	10%	8%	10%	17%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	66	0	0	334	0	0	699	0	0	856	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		custom	NA	
Protected Phases		4				8			2		1	1 6
Permitted Phases	4				8			2			6	
Detector Phase	4	4		8	8		2	2			1	1 6
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		45.0	45.0			5.0	
Minimum Split (s)	10.0	10.0		10.0	10.0		50.0	50.0			10.0	
Total Split (s)	40.0	40.0		40.0	40.0		50.0	50.0			20.0	
Total Split (%)	36.4%	36.4%		36.4%	36.4%		45.5%	45.5%			18.2%	
Yellow Time (s)	4.0	4.0		4.0	4.0		4.0	4.0			4.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0			1.0	
Lost Time Adjust (s)		0.0			0.0			0.0				
Total Lost Time (s)		5.0			5.0			5.0				
Lead/Lag							Lag	Lag		Lead		
Lead-Lag Optimize?							Yes	Yes		Yes		
Recall Mode	None	None		None	None		C-Min	C-Min		None		
Act Effct Green (s)		21.4				21.4			59.8			78.6
Actuated g/C Ratio		0.19				0.19			0.54			0.71
v/c Ratio		0.28				0.85			0.43			0.58
Control Delay		34.3				46.3			17.4			10.2
Queue Delay		0.0				0.0			0.0			0.0
Total Delay		34.3				46.3			17.4			10.2
LOS		C				D			B			B
Approach Delay		34.3				46.3			17.4			10.2
Approach LOS		C				D			B			B
Queue Length 50th (ft)		36				150			148			69

Lane Group	Ø6
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Width (ft)	
Grade (%)	
Lane Util. Factor	
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Confl. Peds. (#/hr)	
Peak Hour Factor	
Heavy Vehicles (%)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Turn Type	
Protected Phases	6
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	45.0
Minimum Split (s)	50.0
Total Split (s)	70.0
Total Split (%)	64%
Yellow Time (s)	4.0
All-Red Time (s)	1.0
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Recall Mode	C-Min
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (ft)	

2686-99-013T

No Build - AM

30: Route 303 & South Greenbush Road/Route 340



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Queue Length 95th (ft)	68			232			240			143		
Internal Link Dist (ft)	200			244			865			541		
Turn Bay Length (ft)												
Base Capacity (vph)	382				564			1624			1493	
Starvation Cap Reductn	0				0			0			0	
Spillback Cap Reductn	0				0			0			0	
Storage Cap Reductn	0				0			0			0	
Reduced v/c Ratio	0.17				0.59			0.43			0.57	

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBTL, Start of Yellow

Natural Cycle: 75

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.85

Intersection Signal Delay: 19.8

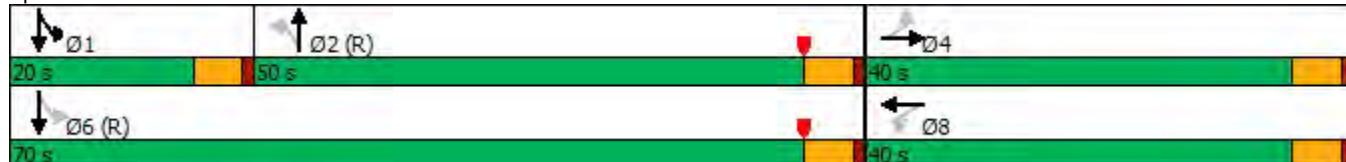
Intersection LOS: B

Intersection Capacity Utilization 93.2%

ICU Level of Service F

Analysis Period (min) 15

Splits and Phases: 30: Route 303 & South Greenbush Road/Route 340



Lane Group	Ø6
Queue Length 95th (ft)	
Internal Link Dist (ft)	
Turn Bay Length (ft)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

2686-99-013T

No Build - PM

30: Route 303 & South Greenbush Road/Route 340

	↗	→	↘	↙	←	↖	↑	↗	↘	↓	↙	
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	34	19	6	82	37	251	17	591	44	175	653	17
Future Volume (vph)	34	19	6	82	37	251	17	591	44	175	653	17
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	16	16	16	12	12	12	11	11	11	11	11	11
Grade (%)	-2%				-3%			-3%			-1%	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Ped Bike Factor		1.00			1.00			1.00			1.00	
Frt		0.987			0.908			0.990			0.997	
Flt Protected		0.972			0.989			0.999			0.990	
Satd. Flow (prot)	0	2048	0	0	1675	0	0	3340	0	0	3324	0
Flt Permitted		0.573			0.913			0.919			0.608	
Satd. Flow (perm)	0	1207	0	0	1546	0	0	3073	0	0	2041	0
Right Turn on Red		Yes			Yes			Yes			No	
Satd. Flow (RTOR)		5			101			8				
Link Speed (mph)		30			40			40			40	
Link Distance (ft)		280			324			945			621	
Travel Time (s)		6.4			5.5			16.1			10.6	
Confl. Peds. (#/hr)			1	1			1					1
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	3%	0%	0%	3%	0%	4%	0%	5%	5%	3%	4%	19%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	62	0	0	389	0	0	686	0	0	889	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		custom	NA	
Protected Phases		4			8			2		1	16	
Permitted Phases	4			8			2			6		
Detector Phase	4	4		8	8		2	2		1	16	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		45.0	45.0			5.0	
Minimum Split (s)	10.0	10.0		10.0	10.0		50.0	50.0			10.0	
Total Split (s)	40.0	40.0		40.0	40.0		50.0	50.0			20.0	
Total Split (%)	36.4%	36.4%		36.4%	36.4%		45.5%	45.5%			18.2%	
Maximum Green (s)	35.0	35.0		35.0	35.0		45.0	45.0			15.0	
Yellow Time (s)	4.0	4.0		4.0	4.0		4.0	4.0			4.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0			1.0	
Lost Time Adjust (s)		0.0		0.0			0.0					
Total Lost Time (s)		5.0			5.0			5.0				
Lead/Lag							Lag	Lag		Lead		
Lead-Lag Optimize?							Yes	Yes		Yes		
Vehicle Extension (s)	2.0	2.0		2.0	2.0		2.0	2.0		2.0		
Recall Mode	None	None		None	None		C-Max	C-Max		None		
Walk Time (s)	7.0	7.0		7.0	7.0		7.0	7.0				
Flash Dont Walk (s)	23.0	23.0		19.0	19.0		19.0	19.0				
Pedestrian Calls (#/hr)	1	1		1	1		1	1				
Act Effect Green (s)		26.0			26.0			54.8			74.0	
Actuated g/C Ratio		0.24			0.24			0.50			0.67	
v/c Ratio		0.21			0.88			0.45			0.58	
Control Delay		30.1			50.1			20.4			7.9	
Queue Delay		0.0			0.0			0.0			0.0	

Lane Group	Ø6
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Width (ft)	
Grade (%)	
Lane Util. Factor	
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Confl. Peds. (#/hr)	
Peak Hour Factor	
Heavy Vehicles (%)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Turn Type	
Protected Phases	6
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	45.0
Minimum Split (s)	50.0
Total Split (s)	70.0
Total Split (%)	64%
Maximum Green (s)	65.0
Yellow Time (s)	4.0
All-Red Time (s)	1.0
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	2.0
Recall Mode	C-Max
Walk Time (s)	7.0
Flash Dont Walk (s)	23.0
Pedestrian Calls (#/hr)	1
Act Effect Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	

2686-99-013T

No Build - PM

30: Route 303 & South Greenbush Road/Route 340



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Delay	30.1				50.1			20.4			7.9	
LOS		C			D			C			A	
Approach Delay	30.1				50.1			20.4			7.9	
Approach LOS		C			D			C			A	
Queue Length 50th (ft)	32				201			161			27	
Queue Length 95th (ft)	62				294			246			139	
Internal Link Dist (ft)	200				244			865			541	
Turn Bay Length (ft)												
Base Capacity (vph)	387				560			1533			1547	
Starvation Cap Reductn	0				0			0			0	
Spillback Cap Reductn	0				0			0			0	
Storage Cap Reductn	0				0			0			0	
Reduced v/c Ratio	0.16				0.69			0.45			0.57	

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 0 (0%), Referenced to phase 2:NBTL and 6:SBTL, Start of Yellow

Natural Cycle: 80

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.88

Intersection Signal Delay: 20.9

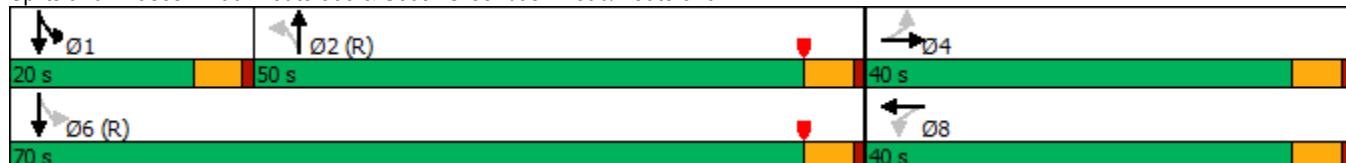
Intersection LOS: C

Intersection Capacity Utilization 96.0%

ICU Level of Service F

Analysis Period (min) 15

Splits and Phases: 30: Route 303 & South Greenbush Road/Route 340



Lane Group	Ø6
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (ft)	
Queue Length 95th (ft)	
Internal Link Dist (ft)	
Turn Bay Length (ft)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	26	30	7	59	31	239	7	599	64	259	552	20
Future Volume (vph)	26	30	7	59	31	239	7	599	64	259	552	20
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	16	16	16	12	12	12	11	11	11	11	11	11
Grade (%)	-2%				-3%			-3%			-1%	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Ped Bike Factor									1.00		1.00	
Frt		0.986				0.902			0.986			0.996
Flt Protected		0.980				0.991						0.985
Satd. Flow (prot)	0	1877	0	0	1614	0	0	3147	0	0	3139	0
Flt Permitted		0.618				0.931			0.946			0.573
Satd. Flow (perm)	0	1183	0	0	1517	0	0	2977	0	0	1826	0
Right Turn on Red			Yes				Yes			Yes		No
Satd. Flow (RTOR)		6			127			12				
Link Speed (mph)		30			40			40			40	
Link Distance (ft)		280			324			945			621	
Travel Time (s)		6.4			5.5			16.1			10.6	
Confl. Peds. (#/hr)								1				1
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	21%	7%	0%	12%	3%	6%	17%	11%	10%	8%	10%	17%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	66	0	0	347	0	0	705	0	0	875	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		custom	NA	
Protected Phases		4			8			2		1	16	
Permitted Phases	4			8			2			6		
Detector Phase	4	4		8	8		2	2		1	16	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		45.0	45.0			5.0	
Minimum Split (s)	10.0	10.0		10.0	10.0		50.0	50.0			10.0	
Total Split (s)	40.0	40.0		40.0	40.0		50.0	50.0			20.0	
Total Split (%)	36.4%	36.4%		36.4%	36.4%		45.5%	45.5%			18.2%	
Yellow Time (s)	4.0	4.0		4.0	4.0		4.0	4.0			4.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0			1.0	
Lost Time Adjust (s)		0.0			0.0			0.0				
Total Lost Time (s)		5.0			5.0			5.0				
Lead/Lag							Lag	Lag		Lead		
Lead-Lag Optimize?							Yes	Yes		Yes		
Recall Mode	None	None		None	None		C-Min	C-Min		None		
Act Effct Green (s)		22.0			22.0			59.0			78.0	
Actuated g/C Ratio		0.20			0.20			0.54			0.71	
v/c Ratio		0.27			0.86			0.44			0.60	
Control Delay		33.6			46.0			18.0			10.7	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		33.6			46.0			18.0			10.7	
LOS		C			D			B			B	
Approach Delay		33.6			46.0			18.0			10.7	
Approach LOS		C			D			B			B	
Queue Length 50th (ft)		36			155			152			70	

Lane Group	Ø6
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Width (ft)	
Grade (%)	
Lane Util. Factor	
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Confl. Peds. (#/hr)	
Peak Hour Factor	
Heavy Vehicles (%)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Turn Type	
Protected Phases	6
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	45.0
Minimum Split (s)	50.0
Total Split (s)	70.0
Total Split (%)	64%
Yellow Time (s)	4.0
All-Red Time (s)	1.0
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Recall Mode	C-Min
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (ft)	



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Queue Length 95th (ft)	68			239			246			155		
Internal Link Dist (ft)	200			244			865			541		
Turn Bay Length (ft)												
Base Capacity (vph)	380				569			1603			1474	
Starvation Cap Reductn	0				0			0			0	
Spillback Cap Reductn	0				0			0			0	
Storage Cap Reductn	0				0			0			0	
Reduced v/c Ratio	0.17				0.61			0.44			0.59	

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBTL, Start of Yellow

Natural Cycle: 80

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.86

Intersection Signal Delay: 20.2

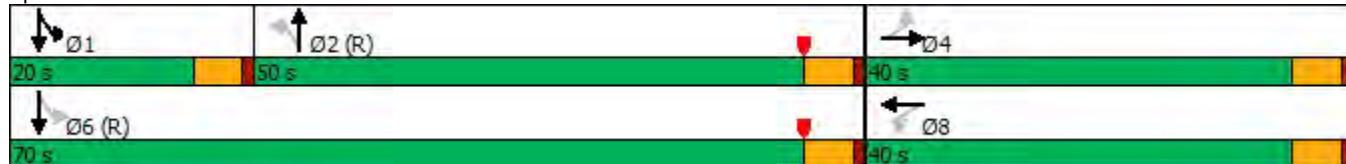
Intersection LOS: C

Intersection Capacity Utilization 94.4%

ICU Level of Service F

Analysis Period (min) 15

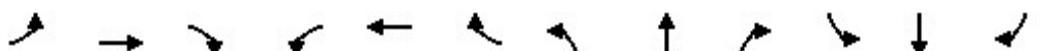
Splits and Phases: 30: Route 303 & South Greenbush Road/Route 340



Lane Group	Ø6
Queue Length 95th (ft)	
Internal Link Dist (ft)	
Turn Bay Length (ft)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

	→	→	←	←	↑	↑	↓	↓	↗	↙	↖	↙
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	26	30	11	94	53	357	16	607	50	251	782	22
Future Volume (vph)	26	30	11	94	53	357	16	607	50	251	782	22
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	16	16	16	12	12	12	11	11	11	11	11	11
Grade (%)	-2%			-3%			-3%			-1%		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Frt		0.978			0.904			0.989			0.997	
Flt Protected		0.981			0.991			0.999			0.988	
Satd. Flow (prot)	0	2054	0	0	1655	0	0	3373	0	0	3311	0
Flt Permitted		0.686			0.921			0.906			0.559	
Satd. Flow (perm)	0	1437	0	0	1539	0	0	3059	0	0	1873	0
Right Turn on Red			Yes			Yes			Yes			No
Satd. Flow (RTOR)		9			117			9				
Link Speed (mph)		30			40			40			40	
Link Distance (ft)		280			324			945			621	
Travel Time (s)		6.4			5.5			16.1			10.6	
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Heavy Vehicles (%)	4%	0%	0%	1%	6%	5%	0%	4%	2%	4%	4%	20%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	74	0	0	553	0	0	740	0	0	1159	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		custom	NA	
Protected Phases		4			8			2		1	16	
Permitted Phases	4			8			2			6		
Detector Phase	4	4		8	8		2	2		1	16	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		45.0	45.0			5.0	
Minimum Split (s)	10.0	10.0		10.0	10.0		50.0	50.0			10.0	
Total Split (s)	40.0	40.0		40.0	40.0		50.0	50.0			20.0	
Total Split (%)	36.4%	36.4%		36.4%	36.4%		45.5%	45.5%			18.2%	
Yellow Time (s)	4.0	4.0		4.0	4.0		4.0	4.0			4.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0			1.0	
Lost Time Adjust (s)		0.0			0.0			0.0				
Total Lost Time (s)		5.0			5.0			5.0				
Lead/Lag							Lag	Lag		Lead		
Lead-Lag Optimize?							Yes	Yes		Yes		
Recall Mode	None	None		None	None		C-Min	C-Min		None		
Act Effct Green (s)		34.7			34.7			45.3			65.3	
Actuated g/C Ratio		0.32			0.32			0.41			0.59	
v/c Ratio		0.16			0.98			0.58			0.89	
Control Delay		24.8			63.4			27.1			26.9	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		24.8			63.4			27.1			26.9	
LOS	C		E			C			C		C	
Approach Delay		24.8			63.4			27.1			26.9	
Approach LOS		C		E			C			C		C
Queue Length 50th (ft)		33			319			207			198	
Queue Length 95th (ft)		69			#550			270			#291	
Internal Link Dist (ft)		200			244			865			541	

Lane Group	Ø6
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Width (ft)	
Grade (%)	
Lane Util. Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Peak Hour Factor	
Heavy Vehicles (%)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Turn Type	
Protected Phases	6
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	45.0
Minimum Split (s)	50.0
Total Split (s)	70.0
Total Split (%)	64%
Yellow Time (s)	4.0
All-Red Time (s)	1.0
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Recall Mode	C-Min
Act Effect Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (ft)	
Queue Length 95th (ft)	
Internal Link Dist (ft)	



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)												
Base Capacity (vph)		463			569			1265			1308	
Starvation Cap Reductn		0			0			0			0	
Spillback Cap Reductn		0			0			0			0	
Storage Cap Reductn		0			0			0			0	
Reduced v/c Ratio		0.16			0.97			0.58			0.89	

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 0 (0%), Referenced to phase 2:NBTL and 6:SBTL, Start of Yellow

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.98

Intersection Signal Delay: 34.9

Intersection LOS: C

Intersection Capacity Utilization 112.3%

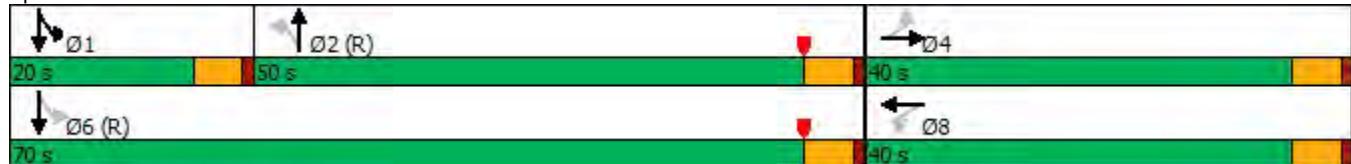
ICU Level of Service H

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 30: Route 303 & South Greenbush Road/Route 340



Lane Group	Ø6
Turn Bay Length (ft)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

2686-99-013T

Design Horizon Year No Build - AM

30: Route 303 & South Greenbush Road/Route 340

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	32	37	9	72	38	277	9	726	78	303	665	24
Future Volume (vph)	32	37	9	72	38	277	9	726	78	303	665	24
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	16	16	16	12	12	12	11	11	11	11	11	11
Grade (%)	-2%				-3%			-3%			-1%	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Ped Bike Factor											1.00	
Frt		0.985				0.903			0.986			0.996
Flt Protected		0.980				0.991			0.999			0.985
Satd. Flow (prot)	0	1874	0	0	1616	0	0	3144	0	0	3139	0
Flt Permitted		0.624				0.925			0.942			0.532
Satd. Flow (perm)	0	1193	0	0	1508	0	0	2965	0	0	1695	0
Right Turn on Red			Yes				Yes			Yes		No
Satd. Flow (RTOR)		6				121			12			
Link Speed (mph)		30				40			40			40
Link Distance (ft)		280				324			945			621
Travel Time (s)		6.4				5.5			16.1			10.6
Confl. Peds. (#/hr)								1				1
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	21%	7%	0%	12%	3%	6%	17%	11%	10%	8%	10%	17%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	82	0	0	408	0	0	855	0	0	1044	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		custom	NA	
Protected Phases		4				8			2		1	16
Permitted Phases	4				8			2			6	
Detector Phase	4	4		8	8		2	2			1	16
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		45.0	45.0			5.0	
Minimum Split (s)	10.0	10.0		10.0	10.0		50.0	50.0			10.0	
Total Split (s)	40.0	40.0		40.0	40.0		50.0	50.0			20.0	
Total Split (%)	36.4%	36.4%		36.4%	36.4%		45.5%	45.5%			18.2%	
Yellow Time (s)	4.0	4.0		4.0	4.0		4.0	4.0			4.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0			1.0	
Lost Time Adjust (s)		0.0			0.0			0.0				
Total Lost Time (s)		5.0			5.0			5.0				
Lead/Lag							Lag	Lag		Lead		
Lead-Lag Optimize?							Yes	Yes		Yes		
Recall Mode	None	None		None	None		C-Min	C-Min		None		
Act Effct Green (s)		26.6				26.6			53.4			73.4
Actuated g/C Ratio		0.24				0.24			0.49			0.67
v/c Ratio		0.28				0.89			0.59			0.79
Control Delay		31.4				50.1			23.7			19.0
Queue Delay		0.0				0.0			0.0			0.0
Total Delay		31.4				50.1			23.7			19.0
LOS		C				D			C			B
Approach Delay		31.4				50.1			23.7			19.0
Approach LOS		C				D			C			B
Queue Length 50th (ft)		43				202			222			140

Lane Group	Ø6
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Width (ft)	
Grade (%)	
Lane Util. Factor	
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Confl. Peds. (#/hr)	
Peak Hour Factor	
Heavy Vehicles (%)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Turn Type	
Protected Phases	6
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	45.0
Minimum Split (s)	50.0
Total Split (s)	70.0
Total Split (%)	64%
Yellow Time (s)	4.0
All-Red Time (s)	1.0
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Recall Mode	C-Min
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (ft)	



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Queue Length 95th (ft)		79			303			329			#264	
Internal Link Dist (ft)		200			244			865			541	
Turn Bay Length (ft)												
Base Capacity (vph)		383			562			1445			1328	
Starvation Cap Reductn		0			0			0			0	
Spillback Cap Reductn		0			0			0			0	
Storage Cap Reductn		0			0			0			0	
Reduced v/c Ratio		0.21			0.73			0.59			0.79	

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBTL, Start of Yellow

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.89

Intersection Signal Delay: 26.4

Intersection LOS: C

Intersection Capacity Utilization 102.8%

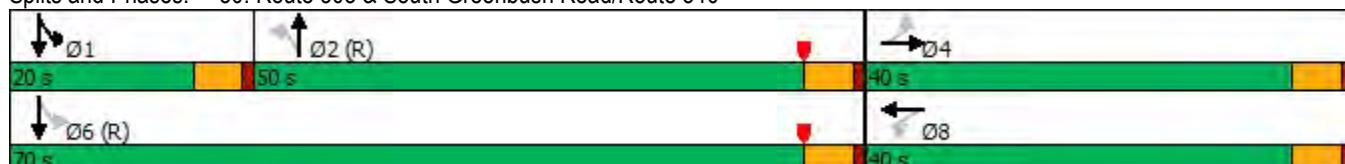
ICU Level of Service G

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 30: Route 303 & South Greenbush Road/Route 340



Lane Group	Ø6
Queue Length 95th (ft)	
Internal Link Dist (ft)	
Turn Bay Length (ft)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

2686-99-013T

Design Horizon Year No Build - PM

30: Route 303 & South Greenbush Road/Route 340

	→	→	→	←	←	↑	↑	↓	↓	←	→	
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	32	37	13	115	65	419	20	731	61	298	952	27
Future Volume (vph)	32	37	13	115	65	419	20	731	61	298	952	27
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	16	16	16	12	12	12	11	11	11	11	11	11
Grade (%)	-2%			-3%			-3%			-1%		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Frt		0.979			0.905			0.989			0.997	
Flt Protected		0.981			0.991			0.999			0.988	
Satd. Flow (prot)	0	2057	0	0	1658	0	0	3373	0	0	3311	0
Flt Permitted		0.617			0.915			0.883			0.522	
Satd. Flow (perm)	0	1294	0	0	1531	0	0	2981	0	0	1749	0
Right Turn on Red			Yes			Yes			Yes			No
Satd. Flow (RTOR)		9			112			9				
Link Speed (mph)		30			40			40			40	
Link Distance (ft)		280			324			945			621	
Travel Time (s)		6.4			5.5			16.1			10.6	
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Heavy Vehicles (%)	4%	0%	0%	1%	6%	5%	0%	4%	2%	4%	4%	20%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	90	0	0	657	0	0	892	0	0	1403	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		custom	NA	
Protected Phases		4			8			2		1	16	
Permitted Phases	4			8			2			6		
Detector Phase	4	4		8	8		2	2		1	16	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		45.0	45.0		5.0		
Minimum Split (s)	10.0	10.0		10.0	10.0		50.0	50.0		10.0		
Total Split (s)	40.0	40.0		40.0	40.0		50.0	50.0		20.0		
Total Split (%)	36.4%	36.4%		36.4%	36.4%		45.5%	45.5%		18.2%		
Yellow Time (s)	4.0	4.0		4.0	4.0		4.0	4.0		4.0		
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0		
Lost Time Adjust (s)		0.0			0.0			0.0				
Total Lost Time (s)		5.0			5.0			5.0				
Lead/Lag							Lag	Lag		Lead		
Lead-Lag Optimize?							Yes	Yes		Yes		
Recall Mode	None	None		None	None		C-Min	C-Min		None		
Act Effct Green (s)		35.0			35.0			45.0			65.0	
Actuated g/C Ratio		0.32			0.32			0.41			0.59	
v/c Ratio		0.22			1.17			0.73			1.13	
Control Delay		26.3			122.8			31.3			87.7	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		26.3			122.8			31.3			87.7	
LOS	C			F			C			F		
Approach Delay		26.3			122.8			31.3			87.7	
Approach LOS		C			F			C			F	
Queue Length 50th (ft)		41			~497			271			~263	
Queue Length 95th (ft)		83			#724			349		m#276		
Internal Link Dist (ft)		200			244			865			541	

Lane Group	Ø6
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Width (ft)	
Grade (%)	
Lane Util. Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Peak Hour Factor	
Heavy Vehicles (%)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Turn Type	
Protected Phases	6
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	45.0
Minimum Split (s)	50.0
Total Split (s)	70.0
Total Split (%)	64%
Yellow Time (s)	4.0
All-Red Time (s)	1.0
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Recall Mode	C-Min
Act Effect Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (ft)	
Queue Length 95th (ft)	
Internal Link Dist (ft)	



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)												
Base Capacity (vph)		417			563			1224			1246	
Starvation Cap Reductn		0			0			0			0	
Spillback Cap Reductn		0			0			0			0	
Storage Cap Reductn		0			0			0			0	
Reduced v/c Ratio		0.22			1.17			0.73			1.13	

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 0 (0%), Referenced to phase 2:NBTL and 6:SBTL, Start of Yellow

Natural Cycle: 100

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.17

Intersection Signal Delay: 76.9

Intersection LOS: E

Intersection Capacity Utilization 124.8%

ICU Level of Service H

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

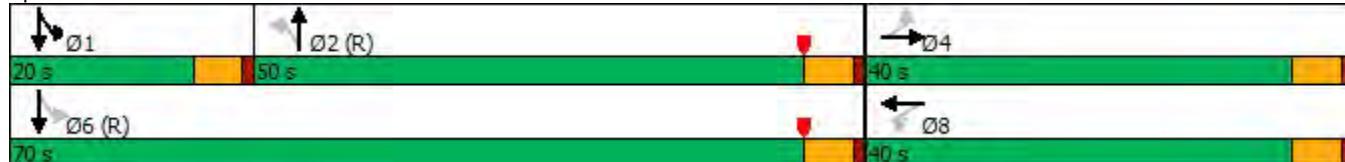
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 30: Route 303 & South Greenbush Road/Route 340



Lane Group	Ø6
Turn Bay Length (ft)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
<hr/> Intersection Summary	

2686-99-013T

 Design Horizon Year Build - AM
 30: Route 303 & South Greenbush Road/Route 340

	↗	→	↘	↙	←	↖	↑	↗	↘	↓	↙	
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	32	37	9	72	38	289	9	731	78	314	672	24
Future Volume (vph)	32	37	9	72	38	289	9	731	78	314	672	24
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	16	16	16	12	12	12	11	11	11	11	11	11
Grade (%)	-2%				-3%			-3%			-1%	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Ped Bike Factor										1.00		1.00
Frt		0.985				0.902			0.986			0.996
Flt Protected		0.980				0.991			0.999			0.985
Satd. Flow (prot)	0	1874	0	0	1614	0	0	3144	0	0	3139	0
Flt Permitted		0.621				0.927			0.941			0.529
Satd. Flow (perm)	0	1187	0	0	1510	0	0	2962	0	0	1686	0
Right Turn on Red			Yes				Yes			Yes		No
Satd. Flow (RTOR)		6				126			12			
Link Speed (mph)		30				40			40			40
Link Distance (ft)		280				324			945			621
Travel Time (s)		6.4				5.5			16.1			10.6
Confl. Peds. (#/hr)								1				1
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	21%	7%	0%	12%	3%	6%	17%	11%	10%	8%	10%	17%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	82	0	0	420	0	0	860	0	0	1063	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		custom	NA	
Protected Phases		4				8			2		1	16
Permitted Phases	4				8			2			6	
Detector Phase	4	4		8	8		2	2			1	16
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		45.0	45.0			5.0	
Minimum Split (s)	10.0	10.0		10.0	10.0		50.0	50.0			10.0	
Total Split (s)	40.0	40.0		40.0	40.0		50.0	50.0			20.0	
Total Split (%)	36.4%	36.4%		36.4%	36.4%		45.5%	45.5%			18.2%	
Yellow Time (s)	4.0	4.0		4.0	4.0		4.0	4.0			4.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0			1.0	
Lost Time Adjust (s)		0.0			0.0			0.0				
Total Lost Time (s)		5.0			5.0			5.0				
Lead/Lag							Lag	Lag		Lead		
Lead-Lag Optimize?							Yes	Yes		Yes		
Recall Mode	None	None		None	None		C-Min	C-Min		None		
Act Effct Green (s)		27.1				27.1			52.9			72.9
Actuated g/C Ratio		0.25				0.25			0.48			0.66
v/c Ratio		0.28				0.90			0.60			0.81
Control Delay		31.1				50.2			24.2			20.3
Queue Delay		0.0				0.0			0.0			0.0
Total Delay		31.1				50.2			24.2			20.3
LOS		C				D			C			C
Approach Delay		31.1				50.2			24.2			20.3
Approach LOS		C				D			C			C
Queue Length 50th (ft)		43				207			227			156

Lane Group	Ø6
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Width (ft)	
Grade (%)	
Lane Util. Factor	
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Confl. Peds. (#/hr)	
Peak Hour Factor	
Heavy Vehicles (%)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Turn Type	
Protected Phases	6
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	45.0
Minimum Split (s)	50.0
Total Split (s)	70.0
Total Split (%)	64%
Yellow Time (s)	4.0
All-Red Time (s)	1.0
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Recall Mode	C-Min
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (ft)	



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Queue Length 95th (ft)		79			314			332			#287	
Internal Link Dist (ft)		200			244			865			541	
Turn Bay Length (ft)												
Base Capacity (vph)		381			566			1430			1315	
Starvation Cap Reductn		0			0			0			0	
Spillback Cap Reductn		0			0			0			0	
Storage Cap Reductn		0			0			0			0	
Reduced v/c Ratio		0.22			0.74			0.60			0.81	

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBTL, Start of Yellow

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.90

Intersection Signal Delay: 27.2

Intersection LOS: C

Intersection Capacity Utilization 104.0%

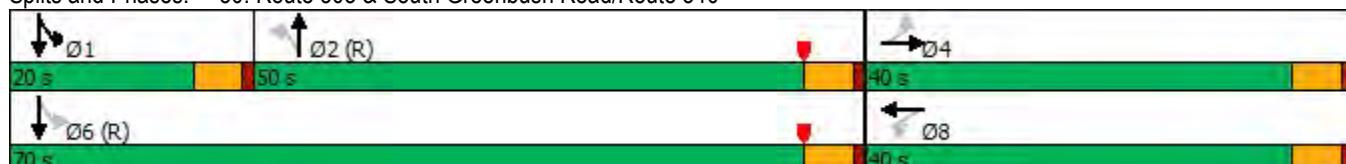
ICU Level of Service G

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 30: Route 303 & South Greenbush Road/Route 340



Lane Group	Ø6
Queue Length 95th (ft)	
Internal Link Dist (ft)	
Turn Bay Length (ft)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

2686-99-013T

 Design Horizon Year Build - PM
 30: Route 303 & South Greenbush Road/Route 340

	→	→	←	←	↑	↑	↓	↓	↗	↙	↖	↙
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	32	37	13	115	65	433	20	739	61	305	954	27
Future Volume (vph)	32	37	13	115	65	433	20	739	61	305	954	27
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	16	16	16	12	12	12	11	11	11	11	11	11
Grade (%)	-2%			-3%			-3%			-1%		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.95
Frt		0.979			0.905			0.989			0.997	
Flt Protected		0.981			0.991			0.999			0.988	
Satd. Flow (prot)	0	2057	0	0	1657	0	0	3373	0	0	3311	0
Flt Permitted		0.609			0.917			0.883			0.519	
Satd. Flow (perm)	0	1277	0	0	1534	0	0	2981	0	0	1739	0
Right Turn on Red			Yes			Yes			Yes			No
Satd. Flow (RTOR)		9			116			9				
Link Speed (mph)		30			40			40			40	
Link Distance (ft)		280			324			945			621	
Travel Time (s)		6.4			5.5			16.1			10.6	
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Heavy Vehicles (%)	4%	0%	0%	1%	6%	5%	0%	4%	2%	4%	4%	20%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	90	0	0	673	0	0	901	0	0	1413	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		custom	NA	
Protected Phases		4			8			2		1	16	
Permitted Phases	4			8			2			6		
Detector Phase	4	4		8	8		2	2		1	16	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		45.0	45.0		5.0		
Minimum Split (s)	10.0	10.0		10.0	10.0		50.0	50.0		10.0		
Total Split (s)	40.0	40.0		40.0	40.0		50.0	50.0		20.0		
Total Split (%)	36.4%	36.4%		36.4%	36.4%		45.5%	45.5%		18.2%		
Yellow Time (s)	4.0	4.0		4.0	4.0		4.0	4.0		4.0		
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0		
Lost Time Adjust (s)		0.0			0.0			0.0				
Total Lost Time (s)		5.0			5.0			5.0				
Lead/Lag							Lag	Lag		Lead		
Lead-Lag Optimize?							Yes	Yes		Yes		
Recall Mode	None	None		None	None		C-Min	C-Min		None		
Act Effct Green (s)		35.0			35.0			45.0			65.0	
Actuated g/C Ratio		0.32			0.32			0.41			0.59	
v/c Ratio		0.22			1.19			0.74			1.14	
Control Delay		26.4			130.2			31.6			92.9	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		26.4			130.2			31.6			92.9	
LOS	C			F			C			F		
Approach Delay		26.4			130.2			31.6			92.9	
Approach LOS		C			F			C			F	
Queue Length 50th (ft)		41			~516			275			~272	
Queue Length 95th (ft)		83			#744			354			m#276	
Internal Link Dist (ft)		200			244			865			541	

Lane Group	Ø6
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Width (ft)	
Grade (%)	
Lane Util. Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Peak Hour Factor	
Heavy Vehicles (%)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Turn Type	
Protected Phases	6
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	45.0
Minimum Split (s)	50.0
Total Split (s)	70.0
Total Split (%)	64%
Yellow Time (s)	4.0
All-Red Time (s)	1.0
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Recall Mode	C-Min
Act Effect Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (ft)	
Queue Length 95th (ft)	
Internal Link Dist (ft)	



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)												
Base Capacity (vph)	412			567			1224			1241		
Starvation Cap Reductn	0			0			0			0		
Spillback Cap Reductn	0			0			0			0		
Storage Cap Reductn	0			0			0			0		
Reduced v/c Ratio	0.22			1.19			0.74			1.14		

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 0 (0%), Referenced to phase 2:NBTL and 6:SBTL, Start of Yellow

Natural Cycle: 120

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.19

Intersection Signal Delay: 81.2

Intersection LOS: F

Intersection Capacity Utilization 125.9%

ICU Level of Service H

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

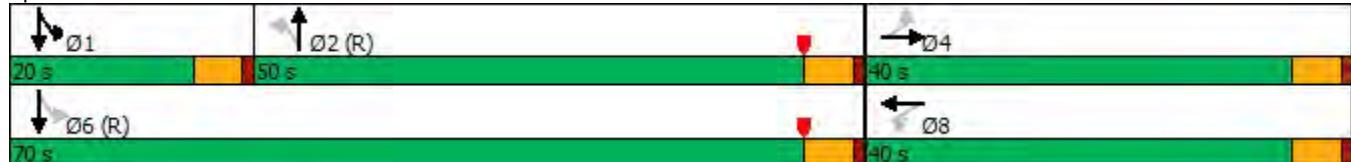
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 30: Route 303 & South Greenbush Road/Route 340



Lane Group	Ø6
Turn Bay Length (ft)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

Intersection

Int Delay, s/veh 1.1

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W		↑↑	↑↑		
Traffic Vol, veh/h	24	21	39	566	580	50
Future Vol, veh/h	24	21	39	566	580	50
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	-6	-	-	-1	-3	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	50	43	13	28	12	10
Mvmt Flow	25	22	41	596	611	53

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	1018	332	664	0	-	0
Stage 1	638	-	-	-	-	-
Stage 2	380	-	-	-	-	-
Critical Hdwy	6.6	7.16	4.36	-	-	-
Critical Hdwy Stg 1	5.6	-	-	-	-	-
Critical Hdwy Stg 2	5.6	-	-	-	-	-
Follow-up Hdwy	4	3.73	2.33	-	-	-
Pot Cap-1 Maneuver	232	589	851	-	-	-
Stage 1	466	-	-	-	-	-
Stage 2	611	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	215	589	851	-	-	-
Mov Cap-2 Maneuver	215	-	-	-	-	-
Stage 1	432	-	-	-	-	-
Stage 2	611	-	-	-	-	-

Approach EB NB SB

HCM Control Delay, s 18.9 0.9 0

HCM LOS C

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	851	-	306	-	-
HCM Lane V/C Ratio	0.048	-	0.155	-	-
HCM Control Delay (s)	9.4	0.3	18.9	-	-
HCM Lane LOS	A	A	C	-	-
HCM 95th %tile Q(veh)	0.2	-	0.5	-	-

Intersection

Int Delay, s/veh 1.5

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W			↑↑	↑↑	
Traffic Vol, veh/h	38	55	10	734	785	19
Future Vol, veh/h	38	55	10	734	785	19
Conflicting Peds, #/hr	0	0	1	0	0	1
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	-6	-	-	-1	-3	-
Peak Hour Factor	93	93	93	93	93	93
Heavy Vehicles, %	24	20	70	15	16	53
Mvmt Flow	41	59	11	789	844	20

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	1272	433	865	0	-	0
Stage 1	855	-	-	-	-	-
Stage 2	417	-	-	-	-	-
Critical Hdwy	6.08	6.7	5.5	-	-	-
Critical Hdwy Stg 1	5.08	-	-	-	-	-
Critical Hdwy Stg 2	5.08	-	-	-	-	-
Follow-up Hdwy	3.74	3.5	2.9	-	-	-
Pot Cap-1 Maneuver	202	563	460	-	-	-
Stage 1	435	-	-	-	-	-
Stage 2	659	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	193	563	460	-	-	-
Mov Cap-2 Maneuver	193	-	-	-	-	-
Stage 1	416	-	-	-	-	-
Stage 2	658	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	21.6	0.5	0
HCM LOS	C		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	460	-	316	-	-
HCM Lane V/C Ratio	0.023	-	0.316	-	-
HCM Control Delay (s)	13	0.3	21.6	-	-
HCM Lane LOS	B	A	C	-	-
HCM 95th %tile Q(veh)	0.1	-	1.3	-	-

Intersection

Int Delay, s/veh 2.2

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W		↑↑	↑↑		
Traffic Vol, veh/h	33	34	90	619	641	83
Future Vol, veh/h	33	34	90	619	641	83
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	-6	-	-	-1	-3	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	50	43	13	28	12	10
Mvmt Flow	35	36	95	652	675	87

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	1235	381	762	0	-	0
Stage 1	719	-	-	-	-	-
Stage 2	516	-	-	-	-	-
Critical Hdwy	6.6	7.16	4.36	-	-	-
Critical Hdwy Stg 1	5.6	-	-	-	-	-
Critical Hdwy Stg 2	5.6	-	-	-	-	-
Follow-up Hdwy	4	3.73	2.33	-	-	-
Pot Cap-1 Maneuver	172	548	778	-	-	-
Stage 1	427	-	-	-	-	-
Stage 2	530	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	139	548	778	-	-	-
Mov Cap-2 Maneuver	139	-	-	-	-	-
Stage 1	345	-	-	-	-	-
Stage 2	530	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	28.3	2	0
HCM LOS	D		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	778	-	224	-	-
HCM Lane V/C Ratio	0.122	-	0.315	-	-
HCM Control Delay (s)	10.3	0.8	28.3	-	-
HCM Lane LOS	B	A	D	-	-
HCM 95th %tile Q(veh)	0.4	-	1.3	-	-

Intersection

Int Delay, s/veh 4

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W			↑↑	↑↑	
Traffic Vol, veh/h	65	94	23	802	835	27
Future Vol, veh/h	65	94	23	802	835	27
Conflicting Peds, #/hr	0	0	1	0	0	1
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	-6	-	-	-1	-3	-
Peak Hour Factor	93	93	93	93	93	93
Heavy Vehicles, %	24	20	70	15	16	53
Mvmt Flow	70	101	25	862	898	29

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	1395	465	928	0	-	0
Stage 1	914	-	-	-	-	-
Stage 2	481	-	-	-	-	-
Critical Hdwy	6.08	6.7	5.5	-	-	-
Critical Hdwy Stg 1	5.08	-	-	-	-	-
Critical Hdwy Stg 2	5.08	-	-	-	-	-
Follow-up Hdwy	3.74	3.5	2.9	-	-	-
Pot Cap-1 Maneuver	173	538	427	-	-	-
Stage 1	410	-	-	-	-	-
Stage 2	620	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	153	538	427	-	-	-
Mov Cap-2 Maneuver	153	-	-	-	-	-
Stage 1	364	-	-	-	-	-
Stage 2	619	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	40.3	1.1	0
HCM LOS	E		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	427	-	265	-	-
HCM Lane V/C Ratio	0.058	-	0.645	-	-
HCM Control Delay (s)	13.9	0.7	40.3	-	-
HCM Lane LOS	B	A	E	-	-
HCM 95th %tile Q(veh)	0.2	-	4.1	-	-

Intersection

Int Delay, s/veh 2.3

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	33	34	90	639	662	83
Future Vol, veh/h	33	34	90	639	662	83
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	-6	-	-	-1	-3	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	50	43	13	28	12	10
Mvmt Flow	35	36	95	673	697	87

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	1268	392	784	0	-	0
Stage 1	741	-	-	-	-	-
Stage 2	527	-	-	-	-	-
Critical Hdwy	6.6	7.16	4.36	-	-	-
Critical Hdwy Stg 1	5.6	-	-	-	-	-
Critical Hdwy Stg 2	5.6	-	-	-	-	-
Follow-up Hdwy	4	3.73	2.33	-	-	-
Pot Cap-1 Maneuver	164	539	762	-	-	-
Stage 1	417	-	-	-	-	-
Stage 2	524	-	-	-	-	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	131	539	762	-	-	-
Mov Cap-2 Maneuver	131	-	-	-	-	-
Stage 1	334	-	-	-	-	-
Stage 2	524	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	30	2	0
HCM LOS	D		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	762	-	213	-	-
HCM Lane V/C Ratio	0.124	-	0.331	-	-
HCM Control Delay (s)	10.4	0.8	30	-	-
HCM Lane LOS	B	A	D	-	-
HCM 95th %tile Q(veh)	0.4	-	1.4	-	-

Intersection

Int Delay, s/veh 4.3

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W			↑↑	↑↑	
Traffic Vol, veh/h	65	94	23	816	861	27
Future Vol, veh/h	65	94	23	816	861	27
Conflicting Peds, #/hr	0	0	1	0	0	1
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	-6	-	-	-1	-3	-
Peak Hour Factor	93	93	93	93	93	93
Heavy Vehicles, %	24	20	70	15	16	53
Mvmt Flow	70	101	25	877	926	29

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	1431	479	956	0	-	0
Stage 1	942	-	-	-	-	-
Stage 2	489	-	-	-	-	-
Critical Hdwy	6.08	6.7	5.5	-	-	-
Critical Hdwy Stg 1	5.08	-	-	-	-	-
Critical Hdwy Stg 2	5.08	-	-	-	-	-
Follow-up Hdwy	3.74	3.5	2.9	-	-	-
Pot Cap-1 Maneuver	165	528	413	-	-	-
Stage 1	399	-	-	-	-	-
Stage 2	616	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	145	528	413	-	-	-
Mov Cap-2 Maneuver	145	-	-	-	-	-
Stage 1	352	-	-	-	-	-
Stage 2	615	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	44.1	1.2	0
HCM LOS	E		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	413	-	254	-	-
HCM Lane V/C Ratio	0.06	-	0.673	-	-
HCM Control Delay (s)	14.3	0.8	44.1	-	-
HCM Lane LOS	B	A	E	-	-
HCM 95th %tile Q(veh)	0.2	-	4.4	-	-

Intersection

Int Delay, s/veh 4.1

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W			↑↑	↑↑	
Traffic Vol, veh/h	40	41	110	755	783	101
Future Vol, veh/h	40	41	110	755	783	101
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	-6	-	-	-1	-3	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	50	43	13	28	12	10
Mvmt Flow	42	43	116	795	824	106

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	1507	465	930	0	-	0
Stage 1	877	-	-	-	-	-
Stage 2	630	-	-	-	-	-
Critical Hdwy	6.6	7.16	4.36	-	-	-
Critical Hdwy Stg 1	5.6	-	-	-	-	-
Critical Hdwy Stg 2	5.6	-	-	-	-	-
Follow-up Hdwy	4	3.73	2.33	-	-	-
Pot Cap-1 Maneuver	117	482	667	-	-	-
Stage 1	360	-	-	-	-	-
Stage 2	470	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	81	482	667	-	-	-
Mov Cap-2 Maneuver	81	-	-	-	-	-
Stage 1	248	-	-	-	-	-
Stage 2	470	-	-	-	-	-

Approach EB NB SB

HCM Control Delay, s 64.3 2.7 0

HCM LOS F

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	667	-	140	-	-
HCM Lane V/C Ratio	0.174	-	0.609	-	-
HCM Control Delay (s)	11.5	1.4	64.3	-	-
HCM Lane LOS	B	A	F	-	-
HCM 95th %tile Q(veh)	0.6	-	3.2	-	-

Intersection

Int Delay, s/veh 16.9

Movement	EBL	EBR	NBL	NBT	SBT	SBR
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Lane Configurations						
Traffic Vol, veh/h	79	115	28	980	1020	33
Future Vol, veh/h	79	115	28	980	1020	33
Conflicting Peds, #/hr	0	0	1	0	0	1
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	-6	-	-	-1	-3	-
Peak Hour Factor	93	93	93	93	93	93
Heavy Vehicles, %	24	20	70	15	16	53
Mvmt Flow	85	124	30	1054	1097	35

Major/Minor	Minor2	Major1	Major2
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Conflicting Flow All	1703	567	1133	0	-	0
Stage 1	1116	-	-	-	-	-
Stage 2	587	-	-	-	-	-
Critical Hdwy	6.08	6.7	5.5	-	-	-
Critical Hdwy Stg 1	5.08	-	-	-	-	-
Critical Hdwy Stg 2	5.08	-	-	-	-	-
Follow-up Hdwy	3.74	3.5	2.9	-	-	-
Pot Cap-1 Maneuver	116	466	335	-	-	-
Stage 1	337	-	-	-	-	-
Stage 2	562	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	91	466	335	-	-	-
Mov Cap-2 Maneuver	91	-	-	-	-	-
Stage 1	264	-	-	-	-	-
Stage 2	561	-	-	-	-	-

Approach	EB	NB	SB
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HCM Control Delay, s	185.2	2.1	0
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HCM LOS	F
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Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	335	-	174	-	-
HCM Lane V/C Ratio	0.09	-	1.199	-	-
HCM Control Delay (s)	16.8	1.7	185.2	-	-
HCM Lane LOS	C	A	F	-	-
HCM 95th %tile Q(veh)	0.3	-	11.3	-	-

Intersection

Int Delay, s/veh 4.4

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W			↑↑	↑↑	
Traffic Vol, veh/h	40	41	110	775	804	101
Future Vol, veh/h	40	41	110	775	804	101
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	-6	-	-	-1	-3	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	50	43	13	28	12	10
Mvmt Flow	42	43	116	816	846	106

Major/Minor	Minor2	Major1	Major2		
Conflicting Flow All	1539	476	952	0	-
Stage 1	899	-	-	-	-
Stage 2	640	-	-	-	-
Critical Hdwy	6.6	7.16	4.36	-	-
Critical Hdwy Stg 1	5.6	-	-	-	-
Critical Hdwy Stg 2	5.6	-	-	-	-
Follow-up Hdwy	4	3.73	2.33	-	-
Pot Cap-1 Maneuver	112	474	653	-	-
Stage 1	351	-	-	-	-
Stage 2	465	-	-	-	-
Platoon blocked, %				-	-
Mov Cap-1 Maneuver	76	474	653	-	-
Mov Cap-2 Maneuver	76	-	-	-	-
Stage 1	237	-	-	-	-
Stage 2	465	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	72.1	2.8	0
HCM LOS	F		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	653	-	132	-	-
HCM Lane V/C Ratio	0.177	-	0.646	-	-
HCM Control Delay (s)	11.7	1.5	72.1	-	-
HCM Lane LOS	B	A	F	-	-
HCM 95th %tile Q(veh)	0.6	-	3.4	-	-

Intersection

Int Delay, s/veh 19.3

Movement	EBL	EBR	NBL	NBT	SBT	SBR
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Lane Configurations



Traffic Vol, veh/h 79 115 28 994 1046 33

Future Vol, veh/h 79 115 28 994 1046 33

Conflicting Peds, #/hr 0 0 1 0 0 1

Sign Control Stop Stop Free Free Free Free

RT Channelized - None - None - None

Storage Length 0 - - - - -

Veh in Median Storage, # 0 - - 0 0 -

Grade, % -6 - - -1 -3 -

Peak Hour Factor 93 93 93 93 93 93

Heavy Vehicles, % 24 20 70 15 16 53

Mvmt Flow 85 124 30 1069 1125 35

Major/Minor	Minor2	Major1	Major2
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Conflicting Flow All 1739 581 1161 0 - 0

Stage 1 1144 - - - - -

Stage 2 595 - - - - -

Critical Hdwy 6.08 6.7 5.5 - - -

Critical Hdwy Stg 1 5.08 - - - - -

Critical Hdwy Stg 2 5.08 - - - - -

Follow-up Hdwy 3.74 3.5 2.9 - - -

Pot Cap-1 Maneuver 110 457 324 - - -

Stage 1 327 - - - - -

Stage 2 557 - - - - -

Platoon blocked, % - - - - -

Mov Cap-1 Maneuver 85 457 324 - - -

Mov Cap-2 Maneuver 85 - - - - -

Stage 1 252 - - - - -

Stage 2 556 - - - - -

Approach	EB	NB	SB
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HCM Control Delay, s 215.9 2.3 0

HCM LOS F

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
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Capacity (veh/h) 324 - 164 - -

HCM Lane V/C Ratio 0.093 - 1.272 - -

HCM Control Delay (s) 17.2 1.9 215.9 - -

HCM Lane LOS C A F - -

HCM 95th %tile Q(veh) 0.3 - 12.1 - -

Intersection

Int Delay, s/veh 0.2

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	0	23	0	729	675	21
Future Vol, veh/h	0	23	0	729	675	21
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	4	2	-
Peak Hour Factor	94	94	94	94	94	94
Heavy Vehicles, %	0	0	0	17	14	14
Mvmt Flow	0	24	0	776	718	22

Major/Minor	Minor2	Major1	Major2
Conflicting Flow All	-	370	-
Stage 1	-	-	-
Stage 2	-	-	-
Critical Hdwy	-	6.9	-
Critical Hdwy Stg 1	-	-	-
Critical Hdwy Stg 2	-	-	-
Follow-up Hdwy	-	3.3	-
Pot Cap-1 Maneuver	0	633	0
Stage 1	0	-	0
Stage 2	0	-	0
Platoon blocked, %	-	-	-
Mov Cap-1 Maneuver	-	633	-
Mov Cap-2 Maneuver	-	-	-
Stage 1	-	-	-
Stage 2	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	10.9	0	0
HCM LOS	B		

Minor Lane/Major Mvmt	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	-	633	-	-
HCM Lane V/C Ratio	-	0.039	-	-
HCM Control Delay (s)	-	10.9	-	-
HCM Lane LOS	-	B	-	-
HCM 95th %tile Q(veh)	-	0.1	-	-

Intersection

Int Delay, s/veh 0.1

Movement	EBL	EBR	NBL	NBT	SBT	SBR
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Lane Configurations						
Traffic Vol, veh/h	0	13	0	839	929	26
Future Vol, veh/h	0	13	0	839	929	26
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	4	2	-
Peak Hour Factor	96	96	96	96	96	96
Heavy Vehicles, %	0	0	0	10	6	19
Mvmt Flow	0	14	0	874	968	27

Major/Minor	Minor2	Major1	Major2
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Conflicting Flow All	-	498	-	0	-	0
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.9	-	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.3	-	-	-	-
Pot Cap-1 Maneuver	0	523	0	-	-	-
Stage 1	0	-	0	-	-	-
Stage 2	0	-	0	-	-	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	-	523	-	-	-	-
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-

Approach	EB	NB	SB
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HCM Control Delay, s	12.1	0	0
HCM LOS	B		

Minor Lane/Major Mvmt	NBT	EBLn1	SBT	SBR
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Capacity (veh/h)	-	523	-	-
HCM Lane V/C Ratio	-	0.026	-	-
HCM Control Delay (s)	-	12.1	-	-
HCM Lane LOS	-	B	-	-
HCM 95th %tile Q(veh)	-	0.1	-	-

Intersection

Int Delay, s/veh 0.2

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	0	23	0	885	824	21
Future Vol, veh/h	0	23	0	885	824	21
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	4	2	-
Peak Hour Factor	94	94	94	94	94	94
Heavy Vehicles, %	0	0	0	17	14	14
Mvmt Flow	0	24	0	941	877	22

Major/Minor	Minor2	Major1	Major2
Conflicting Flow All	-	450	-
Stage 1	-	-	-
Stage 2	-	-	-
Critical Hdwy	-	6.9	-
Critical Hdwy Stg 1	-	-	-
Critical Hdwy Stg 2	-	-	-
Follow-up Hdwy	-	3.3	-
Pot Cap-1 Maneuver	0	562	0
Stage 1	0	-	0
Stage 2	0	-	0
Platoon blocked, %	-	-	-
Mov Cap-1 Maneuver	-	562	-
Mov Cap-2 Maneuver	-	-	-
Stage 1	-	-	-
Stage 2	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	11.7	0	0
HCM LOS	B		

Minor Lane/Major Mvmt	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	-	562	-	-
HCM Lane V/C Ratio	-	0.044	-	-
HCM Control Delay (s)	-	11.7	-	-
HCM Lane LOS	-	B	-	-
HCM 95th %tile Q(veh)	-	0.1	-	-

Intersection

Int Delay, s/veh 0.1

Movement	EBL	EBC	NBL	NBT	SBT	SBR
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Lane Configurations



Traffic Vol, veh/h 0 13 0 1022 1135 26

Future Vol, veh/h 0 13 0 1022 1135 26

Conflicting Peds, #/hr 0 0 0 0 0 0

Sign Control Stop Stop Free Free Free Free

RT Channelized - None - None - None

Storage Length - 0 - - - -

Veh in Median Storage, # 0 - - 0 0 -

Grade, % 0 - - 4 2 -

Peak Hour Factor 96 96 96 96 96 96

Heavy Vehicles, % 0 0 0 10 6 19

Mvmt Flow 0 14 0 1065 1182 27

Major/Minor	Minor2	Major1	Major2
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Conflicting Flow All - 605 - 0 - 0

Stage 1 - - - - - -

Stage 2 - - - - - -

Critical Hdwy - 6.9 - - - -

Critical Hdwy Stg 1 - - - - - -

Critical Hdwy Stg 2 - - - - - -

Follow-up Hdwy - 3.3 - - - -

Pot Cap-1 Maneuver 0 446 0 - - -

Stage 1 0 - 0 - - -

Stage 2 0 - 0 - - -

Platoon blocked, % - - - - - -

Mov Cap-1 Maneuver - 446 - - - -

Mov Cap-2 Maneuver - - - - - -

Stage 1 - - - - - -

Stage 2 - - - - - -

Approach	EB	NB	SB
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HCM Control Delay, s 13.3 0 0

HCM LOS B

Minor Lane/Major Mvmt	NBT	EBLn1	SBT	SBR
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Capacity (veh/h) - 446 - -

HCM Lane V/C Ratio - 0.03 - -

HCM Control Delay (s) - 13.3 - -

HCM Lane LOS - B - -

HCM 95th %tile Q(veh) - 0.1 - -

Intersection

Int Delay, s/veh 1

Movement	EBL	EBT	WBT	WBR	SBL	SBR
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Lane Configurations						
Traffic Vol, veh/h	6	223	236	35	32	6
Future Vol, veh/h	6	223	236	35	32	6
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	-8	6	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	16	12	0	13	0
Mvmt Flow	7	242	257	38	35	7

Major/Minor	Major1	Major2	Minor2			
Conflicting Flow All	295	0	-	0	532	276
Stage 1	-	-	-	-	276	-
Stage 2	-	-	-	-	256	-
Critical Hdwy	4.1	-	-	-	6.53	6.2
Critical Hdwy Stg 1	-	-	-	-	5.53	-
Critical Hdwy Stg 2	-	-	-	-	5.53	-
Follow-up Hdwy	2.2	-	-	-	3.617	3.3
Pot Cap-1 Maneuver	1278	-	-	-	490	768
Stage 1	-	-	-	-	746	-
Stage 2	-	-	-	-	762	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	1278	-	-	-	487	768
Mov Cap-2 Maneuver	-	-	-	-	487	-
Stage 1	-	-	-	-	742	-
Stage 2	-	-	-	-	762	-

Approach	EB	WB	SB
----------	----	----	----

HCM Control Delay, s	0.2	0	12.6
----------------------	-----	---	------

HCM LOS	B
---------	---

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1	
Capacity (veh/h)	1278	-	-	-	517	
HCM Lane V/C Ratio	0.005	-	-	-	0.08	
HCM Control Delay (s)	7.8	0	-	-	12.6	
HCM Lane LOS	A	A	-	-	B	
HCM 95th %tile Q(veh)	0	-	-	-	0.3	

Intersection

Int Delay, s/veh 0.7

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	7	182	249	43	20	3
Future Vol, veh/h	7	182	249	43	20	3
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	-8	6	-	0	-
Peak Hour Factor	86	86	86	86	86	86
Heavy Vehicles, %	0	11	3	0	15	0
Mvmt Flow	8	212	290	50	23	3

Major/Minor	Major1	Major2	Minor2			
Conflicting Flow All	340	0	-	0	543	315
Stage 1	-	-	-	-	315	-
Stage 2	-	-	-	-	228	-
Critical Hdwy	4.1	-	-	-	6.55	6.2
Critical Hdwy Stg 1	-	-	-	-	5.55	-
Critical Hdwy Stg 2	-	-	-	-	5.55	-
Follow-up Hdwy	2.2	-	-	-	3.635	3.3
Pot Cap-1 Maneuver	1230	-	-	-	479	730
Stage 1	-	-	-	-	711	-
Stage 2	-	-	-	-	780	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	1230	-	-	-	476	730
Mov Cap-2 Maneuver	-	-	-	-	476	-
Stage 1	-	-	-	-	706	-
Stage 2	-	-	-	-	780	-

Approach	EB	WB	SB			
HCM Control Delay, s	0.3	0	12.6			
HCM LOS			B			

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1	
Capacity (veh/h)	1230	-	-	-	499	
HCM Lane V/C Ratio	0.007	-	-	-	0.054	
HCM Control Delay (s)	7.9	0	-	-	12.6	
HCM Lane LOS	A	A	-	-	B	
HCM 95th %tile Q(veh)	0	-	-	-	0.2	

Intersection

Int Delay, s/veh 0.9

Movement	EBL	EBT	WBT	WBR	SBL	SBR
----------	-----	-----	-----	-----	-----	-----

Lane Configurations						
Traffic Vol, veh/h	6	272	288	35	32	6
Future Vol, veh/h	6	272	288	35	32	6
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	-8	6	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	16	12	0	13	0
Mvmt Flow	7	296	313	38	35	7

Major/Minor	Major1	Major2	Minor2			
Conflicting Flow All	351	0	-	0	642	332
Stage 1	-	-	-	-	332	-
Stage 2	-	-	-	-	310	-
Critical Hdwy	4.1	-	-	-	6.53	6.2
Critical Hdwy Stg 1	-	-	-	-	5.53	-
Critical Hdwy Stg 2	-	-	-	-	5.53	-
Follow-up Hdwy	2.2	-	-	-	3.617	3.3
Pot Cap-1 Maneuver	1219	-	-	-	421	714
Stage 1	-	-	-	-	703	-
Stage 2	-	-	-	-	719	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	1219	-	-	-	418	714
Mov Cap-2 Maneuver	-	-	-	-	418	-
Stage 1	-	-	-	-	698	-
Stage 2	-	-	-	-	719	-

Approach	EB	WB	SB
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HCM Control Delay, s	0.2	0	13.9
----------------------	-----	---	------

HCM LOS		B	
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Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1	
Capacity (veh/h)	1219	-	-	-	447	
HCM Lane V/C Ratio	0.005	-	-	-	0.092	
HCM Control Delay (s)	8	0	-	-	13.9	
HCM Lane LOS	A	A	-	-	B	
HCM 95th %tile Q(veh)	0	-	-	-	0.3	

Intersection

Int Delay, s/veh 0.6

Movement	EBL	EBT	WBT	WBR	SBL	SBR
----------	-----	-----	-----	-----	-----	-----

Lane Configurations						
Traffic Vol, veh/h	7	222	304	43	20	3
Future Vol, veh/h	7	222	304	43	20	3
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	-8	6	-	0	-
Peak Hour Factor	86	86	86	86	86	86
Heavy Vehicles, %	0	11	3	0	15	0
Mvmt Flow	8	258	353	50	23	3

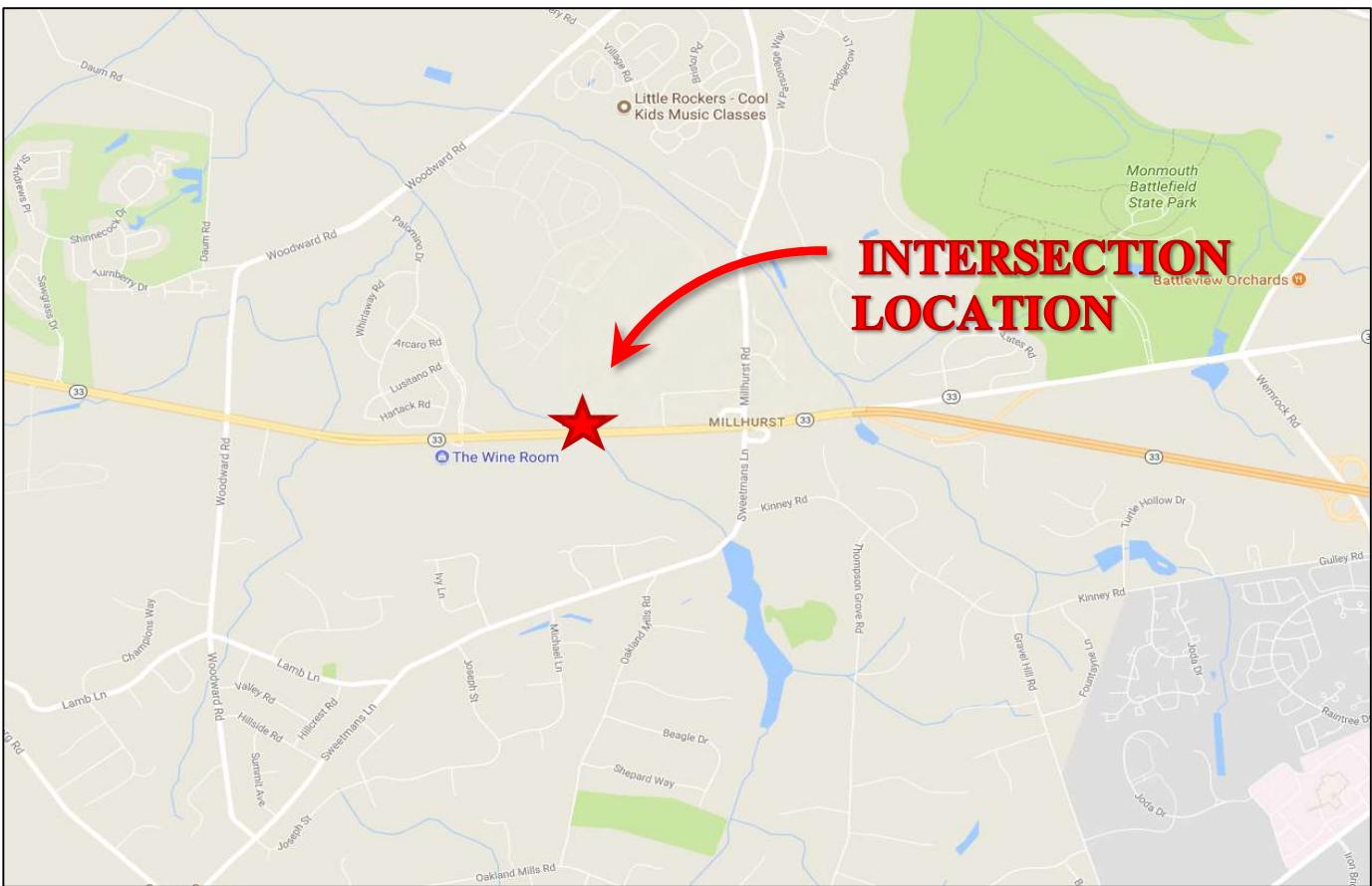
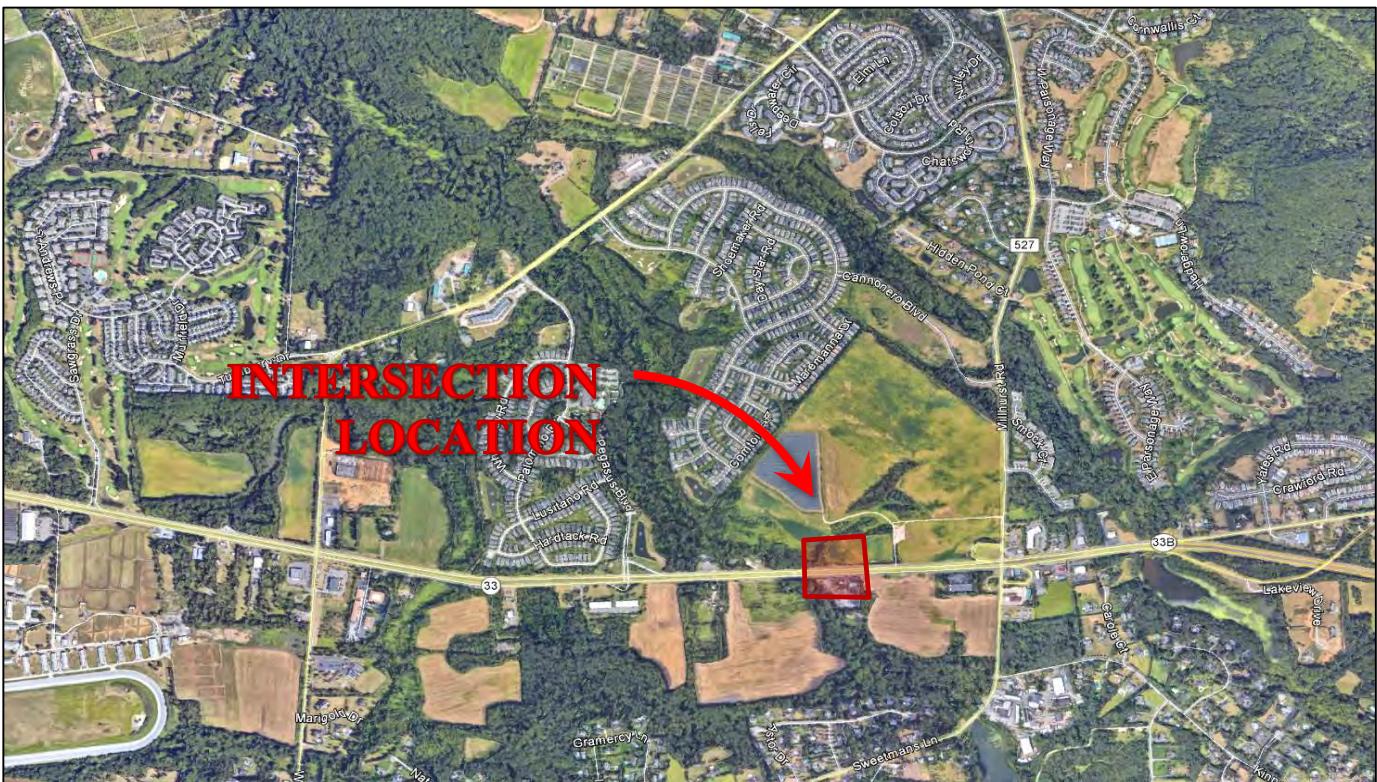
Major/Minor	Major1	Major2	Minor2		
Conflicting Flow All	403	0	-	0	652 378
Stage 1	-	-	-	-	378 -
Stage 2	-	-	-	-	274 -
Critical Hdwy	4.1	-	-	-	6.55 6.2
Critical Hdwy Stg 1	-	-	-	-	5.55 -
Critical Hdwy Stg 2	-	-	-	-	5.55 -
Follow-up Hdwy	2.2	-	-	-	3.635 3.3
Pot Cap-1 Maneuver	1167	-	-	-	413 673
Stage 1	-	-	-	-	665 -
Stage 2	-	-	-	-	743 -
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	1167	-	-	-	410 673
Mov Cap-2 Maneuver	-	-	-	-	410 -
Stage 1	-	-	-	-	660 -
Stage 2	-	-	-	-	743 -

Approach	EB	WB	SB
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HCM Control Delay, s	0.2	0	13.9
HCM LOS			B

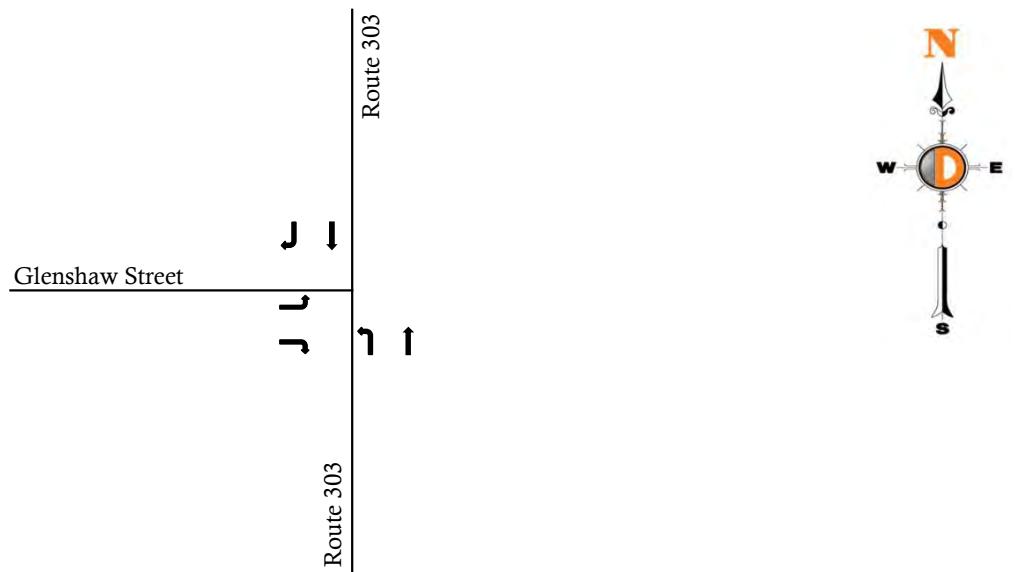
Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	1167	-	-	-	432
HCM Lane V/C Ratio	0.007	-	-	-	0.062
HCM Control Delay (s)	8.1	0	-	-	13.9
HCM Lane LOS	A	A	-	-	B
HCM 95th %tile Q(veh)	0	-	-	-	0.2

Appendix G
Alternate Traffic Signal Warrant Analysis



Route 303 & Glenshaw Street
Signal Warrant Analysis
2686-99-013T

Intersection Location Map



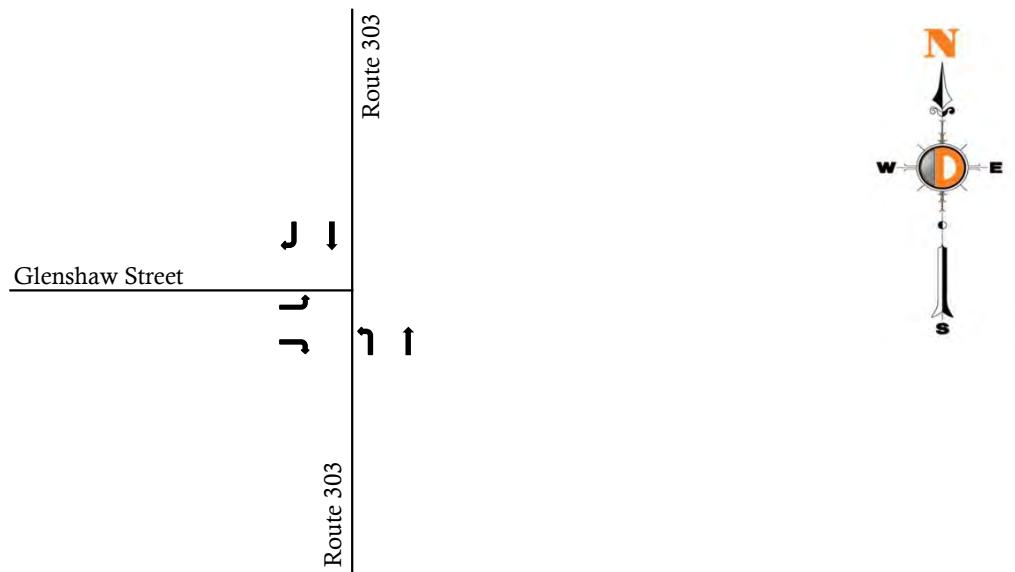
Weekday	Glenshaw Street			Route 303					
	Eastbound			Northbound			Southbound		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
	8	0	8	37	478	0	0	465	38
8:00 AM	8	0	8	37	478	0	0	465	38
9:00 AM	23	0	22	43	479	0	0	561	43
5:00 PM	30	0	32	9	696	0	0	652	18
6:00 PM	32	0	74	9	573	0	0	714	6



Route 303 & Glenshaw Street
Signal Warrant Analysis
2686-99-013T

Figure 1

Existing Traffic Volumes



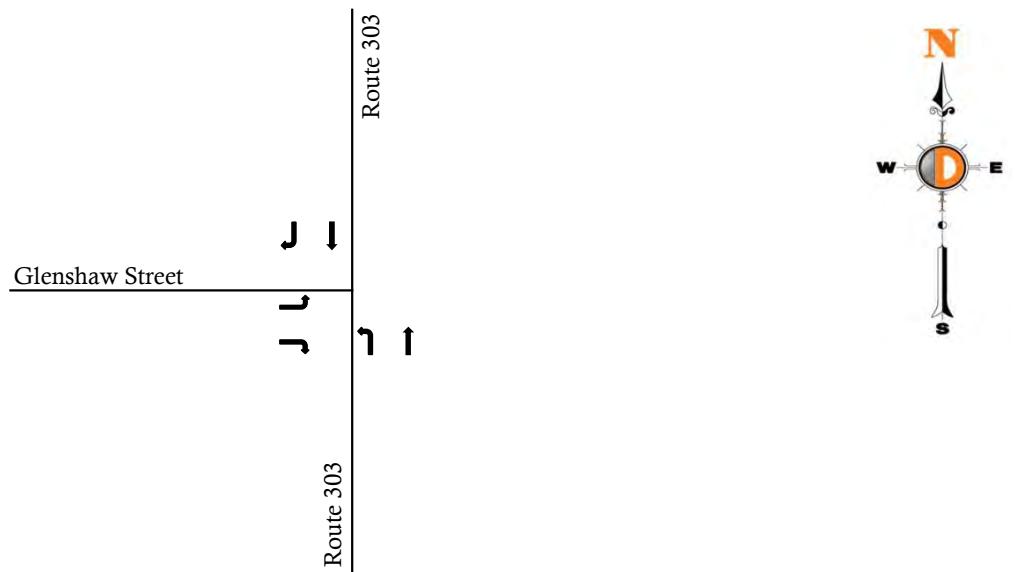
Weekday	Glenshaw Street			Route 303					
	Eastbound			Northbound			Southbound		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
	8	0	8	38	488	0	0	474	39
8:00 AM	8	0	8	38	488	0	0	474	39
9:00 AM	23	0	22	44	489	0	0	572	44
5:00 PM	31	0	33	9	710	0	0	665	18
6:00 PM	33	0	75	9	584	0	0	728	6



Route 303 & Glenshaw Street
Signal Warrant Analysis
2686-99-013T

Figure 3

No Build Traffic Volumes



Weekday	Glenshaw Street			Route 303					
	Eastbound			Northbound			Southbound		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
	10	0	10	46	595	0	0	578	48
8:00 AM	10	0	10	46	595	0	0	578	48
9:00 AM	28	0	27	54	597	0	0	698	54
5:00 PM	38	0	40	11	866	0	0	811	22
6:00 PM	40	0	92	11	713	0	0	888	7



Route 303 & Glenshaw Street
Signal Warrant Analysis
2686-99-013T

Figure 4

No Build Traffic Volumes

Trip Generation					
Land Use	Trip Type	Trips			
		In	Out	Total	
175,760 SF High-Cube Parcel Hub Warehouse	Cars	371	370	741	

Weekday	Temp. Dist	Total Trips	In-Out %		Car Trips	
			In	Out	In	Out
8:00 AM	6.9%	51	50.0%	50.0%	26	25
9:00 AM	11.3%	84	46.6%	53.4%	39	45
5:00 PM	6.9%	51	65.7%	34.3%	34	17
6:00 PM	10.2%	76	66.5%	33.5%	50	26



Route 303 & Glenshaw Street
 Signal Warrant Analysis
 2686-99-013T

Figure 5

Temporal Distribution (Cars)

Trip Distribution

Glenshaw Street			Route 303						
Eastbound			Northbound			Southbound			
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Cars	IN	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	30.0%	0.0%
	OUT	0.0%	0.0%	0.0%	0.0%	30.0%	0.0%	0.0%	0.0%

Weekday	Glenshaw Street			Route 303					
	Eastbound			Northbound			Southbound		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
8:00 AM	0	0	0	0	8	0	0	8	0
9:00 AM	0	0	0	0	13	0	0	12	0
5:00 PM	0	0	0	0	5	0	0	10	0
6:00 PM	0	0	0	0	8	0	0	15	0



Trip Generation				
Land Use	Trip Type	Trips		
		In	Out	Total
175,760 SF High-Cube Parcel Hub Warehouse	Trucks	37	36	73

Weekday	Temp. Dist	Total Trips	In-Out %		Truck Trips	
			In	Out	In	Out
8:00 AM	5.5%	4	34.4%	65.6%	1	3
9:00 AM	9.9%	7	49.2%	50.8%	4	3
5:00 PM	3.9%	3	36.6%	63.4%	1	2
6:00 PM	0.0%	0	50.0%	50.0%	0	0



Route 303 & Glenshaw Street
 Signal Warrant Analysis
 2686-99-013T

Figure 7

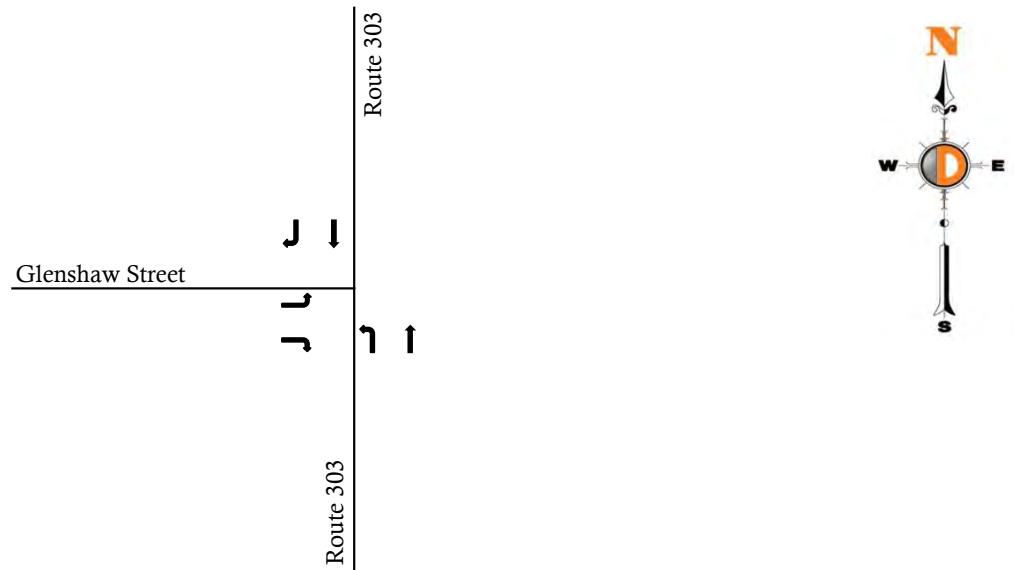
Temporal Distribution (Trucks)

Trip Distribution

		Glenshaw Street			Route 303					
		Eastbound			Northbound			Southbound		
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Trucks	IN	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	100.0%	0.0%
	OUT	0.0%	0.0%	0.0%	0.0%	100.0%	0.0%	0.0%	0.0%	0.0%

Weekday	Glenshaw Street			Route 303					
	Eastbound			Northbound			Southbound		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
8:00 AM	0	0	0	0	3	0	0	1	0
9:00 AM	0	0	0	0	3	0	0	4	0
5:00 PM	0	0	0	0	2	0	0	1	0
6:00 PM	0	0	0	0	0	0	0	0	0





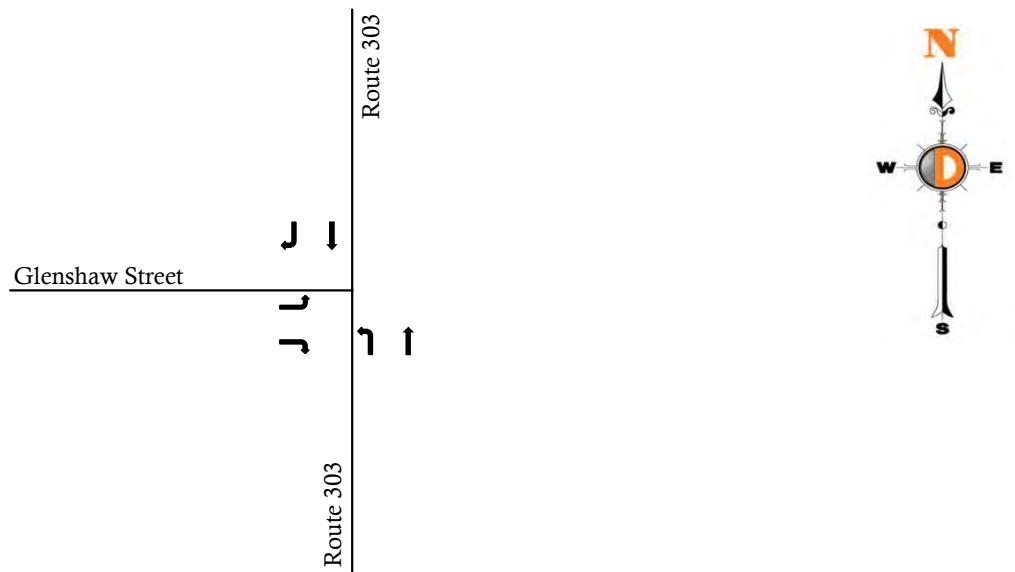
Weekday	Glenshaw Street			Route 303					
	Eastbound			Northbound			Southbound		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
	0	0	0	0	11	0	0	9	0
8:00 AM	0	0	0	0	16	0	0	16	0
9:00 AM	0	0	0	0	7	0	0	11	0
5:00 PM	0	0	0	0	8	0	0	15	0
6:00 PM	0	0	0	0	8	0	0	15	0



Route 303 & Glenshaw Street
Signal Warrant Analysis
2686-99-013T

Figure 9

Total Site Generated Volumes



Weekday	Glenshaw Street			Route 303					
	Eastbound			Northbound			Southbound		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
	10	0	10	46	606	0	0	587	48
8:00 AM	10	0	10	46	606	0	0	587	48
9:00 AM	28	0	27	54	613	0	0	714	54
5:00 PM	38	0	40	11	873	0	0	822	22
6:00 PM	40	0	92	11	721	0	0	903	7



Route 303 & Glenshaw Street
Signal Warrant Analysis
2686-99-013T

Figure 10

Design Horizon Year Build Traffic Volumes

Traffic Signal Warrant Analysis

Intersection: Route 303 & Glenshaw Street
 Location: Orangetown Town, Rockland County, NJ
 Prepared By: A. Ferrante
 Date: 10/4/2022
 Job #: 2686-99-013T



Time	Weekday Volumes			
	Major Road		Minor Road	
	NB	SB	EB	O
12:00 AM	0	0	0	0
1:00 AM	0	0	0	0
2:00 AM	0	0	0	0
3:00 AM	0	0	0	0
4:00 AM	0	0	0	0
5:00 AM	0	0	0	0
6:00 AM	0	0	0	0
7:00 AM	0	0	0	0
8:00 AM	652	635	20	0
9:00 AM	667	768	55	0
10:00 AM	0	0	0	0
11:00 AM	0	0	0	0
12:00 PM	0	0	0	0
1:00 PM	0	0	0	0
2:00 PM	0	0	0	0
3:00 PM	0	0	0	0
4:00 PM	0	0	0	0
5:00 PM	884	844	78	0
6:00 PM	732	910	132	0
7:00 PM	0	0	0	0
8:00 PM	0	0	0	0
9:00 PM	0	0	0	0
10:00 PM	0	0	0	0
11:00 PM	0	0	0	0
Lanes	2	2	1	0
Speed	40	40	30	0

Warrant % Criteria	Warrant Summary			
	1A 100	1B 100	2 100	3 100
	NO	NO	NO	NO
	NO	NO	NO	NO
	NO	NO	NO	NO
	NO	NO	NO	NO
	NO	NO	NO	NO
	NO	NO	NO	NO
	NO	NO	NO	NO
	NO	NO	NO	NO
	NO	NO	NO	NO
	NO	NO	NO	NO
	NO	NO	NO	NO
	NO	NO	NO	NO
	NO	NO	NO	NO
	NO	NO	NO	NO
	NO	NO	NO	NO
	NO	NO	NO	NO
	NO	NO	NO	NO
	NO	NO	NO	NO
	NO	YES	NO	NO
	NO	YES	YES	YES
	NO	NO	NO	NO
	NO	NO	NO	NO
	NO	NO	NO	NO
	NO	NO	NO	NO
	NO	NO	NO	NO
	NO	NO	NO	NO
	Hours Met	0	2	1
	Satisfied?	NO	NO	NO
				YES

Based upon the Traffic Signal Warrants described in Chapter 4C of the Manual on Uniform Traffic Control Devices (MUTCD), 2009 Edition, published by the Federal Highway Administration (FHWA).

Traffic Signal Warrant Analysis

Intersection: Route 303 & Glenshaw Street
 Location: Orangetown Town, Rockland County, NJ
 Prepared By: J. Pesce
 Date: 10/4/2022
 Job #: 2686-99-013T



Warrant 1, Eight-Hour Vehicular Volume (Conditions A and B)

Condition A—Minimum Vehicular Volume

Number of lanes for moving traffic on each approach		Vehicles per hour on major street (total of both approaches)				Vehicles per hour on higher-volume minor-street approach (one direction only)			
Major Street	Minor Street	100% ^a	80% ^b	70% ^c	56% ^d	100% ^a	80% ^b	70% ^c	56% ^d
1	1	500	400	350	280	150	120	105	84
2 or more	1	600	480	420	336	150	120	105	84
2 or more	2 or more	600	480	420	336	200	160	140	112
1	2 or more	500	400	350	280	200	160	140	112

Condition B—Interruption of Continuous Traffic

Number of lanes for moving traffic on each approach		Vehicles per hour on major street (total of both approaches)				Vehicles per hour on higher-volume minor-street approach (one direction only)			
Major Street	Minor Street	100% ^a	80% ^b	70% ^c	56% ^d	100% ^a	80% ^b	70% ^c	56% ^d
1	1	750	600	525	420	75	60	53	42
2 or more	1	900	720	630	504	75	60	53	42
2 or more	2 or more	900	720	630	504	100	80	70	56
1	2 or more	750	600	525	420	100	80	70	56

^a Basic minimum hourly volume

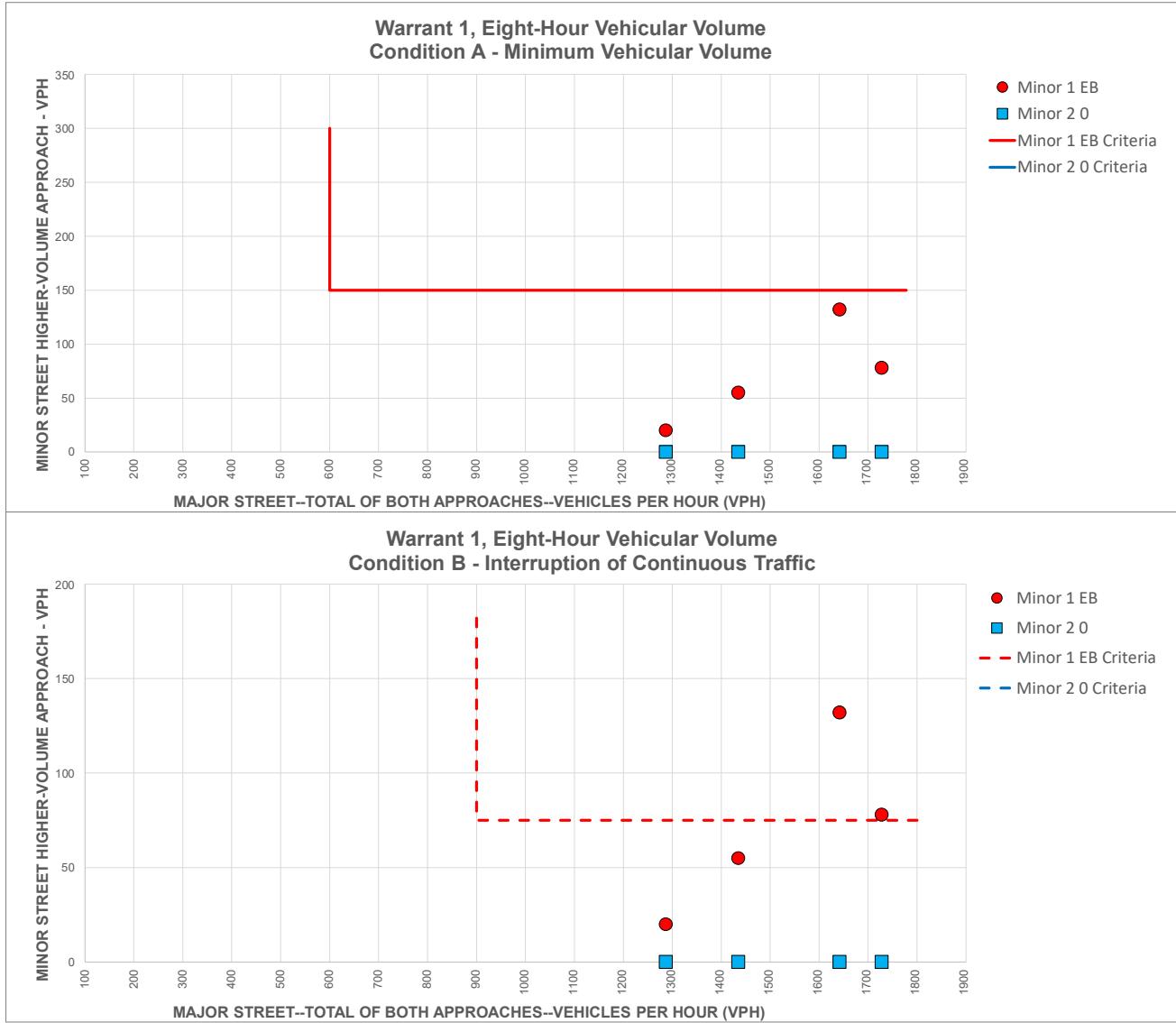
^b Used for combination of Conditions A and B after adequate trial of other remedial measures

^c May be used when the major-street speed exceeds 40 mph or in an isolated community with a population of less than 10,000

^d May be used for combination of Conditions A and B after adequate trial of other remedial measures when the major-street speed exceeds 40 mph or in an isolated community with a population of less than 10,000

Volumes			Condition A Criteria						WARRANT SATISFIED ?	Condition B Criteria						WARRANT SATISFIED ?		
Time	Major TOTAL	Minor EB	Minor 0	Volume %			100				Major	Minor 1	Satisfied?	Major	Minor 2	Satisfied?		
				Major	Minor 1	Satisfied?	Major	Minor 2	Satisfied?									
12:00 AM	0	0	0	600	150	No	-	-	No	NO	900	75	No	-	-	No	NO	
1:00 AM	0	0	0	600	150	No	-	-	No	NO	900	75	No	-	-	No	NO	
2:00 AM	0	0	0	600	150	No	-	-	No	NO	900	75	No	-	-	No	NO	
3:00 AM	0	0	0	600	150	No	-	-	No	NO	900	75	No	-	-	No	NO	
4:00 AM	0	0	0	600	150	No	-	-	No	NO	900	75	No	-	-	No	NO	
5:00 AM	0	0	0	600	150	No	-	-	No	NO	900	75	No	-	-	No	NO	
6:00 AM	0	0	0	600	150	No	-	-	No	NO	900	75	No	-	-	No	NO	
7:00 AM	0	0	0	600	150	No	-	-	No	NO	900	75	No	-	-	No	NO	
8:00 AM	1287	20	0	600	150	No	-	-	No	NO	900	75	No	-	-	No	NO	
9:00 AM	1435	55	0	600	150	No	-	-	No	NO	900	75	No	-	-	No	NO	
10:00 AM	0	0	0	600	150	No	-	-	No	NO	900	75	No	-	-	No	NO	
11:00 AM	0	0	0	600	150	No	-	-	No	NO	900	75	No	-	-	No	NO	
12:00 PM	0	0	0	600	150	No	-	-	No	NO	900	75	No	-	-	No	NO	
1:00 PM	0	0	0	600	150	No	-	-	No	NO	900	75	No	-	-	No	NO	
2:00 PM	0	0	0	600	150	No	-	-	No	NO	900	75	No	-	-	No	NO	
3:00 PM	0	0	0	600	150	No	-	-	No	NO	900	75	No	-	-	No	NO	
4:00 PM	0	0	0	600	150	No	-	-	No	NO	900	75	No	-	-	No	NO	
5:00 PM	1728	78	0	600	150	No	-	-	No	NO	900	75	Yes	-	-	No	YES	
6:00 PM	1642	132	0	600	150	No	-	-	No	NO	900	75	Yes	-	-	No	YES	
7:00 PM	0	0	0	600	150	No	-	-	No	NO	900	75	No	-	-	No	NO	
8:00 PM	0	0	0	600	150	No	-	-	No	NO	900	75	No	-	-	No	NO	
9:00 PM	0	0	0	600	150	No	-	-	No	NO	900	75	No	-	-	No	NO	
10:00 PM	0	0	0	600	150	No	-	-	No	NO	900	75	No	-	-	No	NO	
11:00 PM	0	0	0	600	150	No	-	-	No	NO	900	75	No	-	-	No	NO	
Lanes	2	1	0							0						2		
Speed	40	30	0							Satisfied?	NO					Satisfied?	NO	

Warrant 1, Eight-Hour Vehicular Volume
 (Conditions A and B)

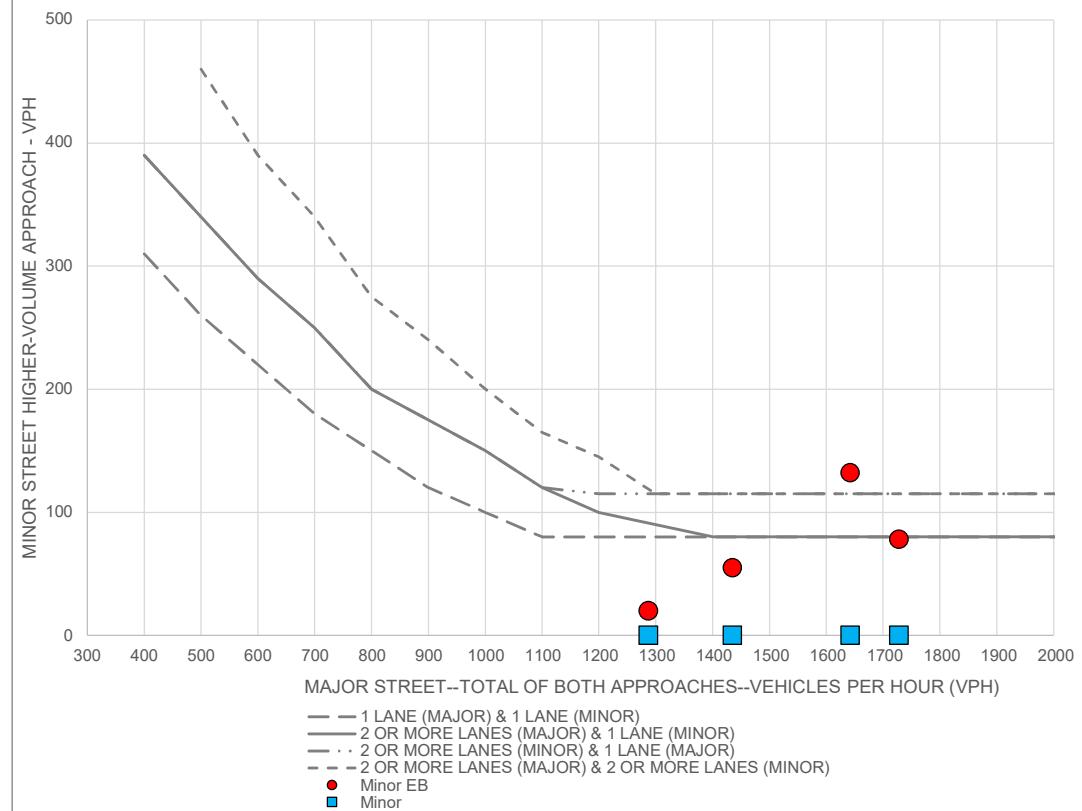


Warrant 2 - Four-Hour Vehicular Volume

(100% Thresholds)

Time	Volumes							WARRANT SATISFIED?
	Major (Total)	Minor EB	Threshold	Satisfied?	Minor	Threshold	Satisfied?	
12:00 AM	0	0	-	No	0	-	No	NO
1:00 AM	0	0	-	No	0	-	No	NO
2:00 AM	0	0	-	No	0	-	No	NO
3:00 AM	0	0	-	No	0	-	No	NO
4:00 AM	0	0	-	No	0	-	No	NO
5:00 AM	0	0	-	No	0	-	No	NO
6:00 AM	0	0	-	No	0	-	No	NO
7:00 AM	0	0	-	No	0	-	No	NO
8:00 AM	1287	20	91	No	0	91	No	NO
9:00 AM	1435	55	80	No	0	80	No	NO
10:00 AM	0	0	-	No	0	-	No	NO
11:00 AM	0	0	-	No	0	-	No	NO
12:00 PM	0	0	-	No	0	-	No	NO
1:00 PM	0	0	-	No	0	-	No	NO
2:00 PM	0	0	-	No	0	-	No	NO
3:00 PM	0	0	-	No	0	-	No	NO
4:00 PM	0	0	-	No	0	-	No	NO
5:00 PM	1728	78	80	No	0	80	No	NO
6:00 PM	1642	132	80	Yes	0	80	No	YES
7:00 PM	0	0	-	No	0	-	No	NO
8:00 PM	0	0	-	No	0	-	No	NO
9:00 PM	0	0	-	No	0	-	No	NO
10:00 PM	0	0	-	No	0	-	No	NO
11:00 PM	0	0	-	No	0	-	No	NO
Lanes	2	1			0			1
Speed	40	30			0		Satisfied?	No

Figure 4C-1. Warrant 2, Four-Hour Vehicular Volume



Warrant 3 - Peak Hour Vehicular Volume

(100% Thresholds)

Time	Volumes							WARRANT SATISFIED ?
	Major (Total)	Minor EB	Threshold	Satisfied?	Minor	Threshold	Satisfied?	
12:00 AM	0	0	-	No	0	-	No	NO
1:00 AM	0	0	-	No	0	-	No	NO
2:00 AM	0	0	-	No	0	-	No	NO
3:00 AM	0	0	-	No	0	-	No	NO
4:00 AM	0	0	-	No	0	-	No	NO
5:00 AM	0	0	-	No	0	-	No	NO
6:00 AM	0	0	-	No	0	-	No	NO
7:00 AM	0	0	-	No	0	-	No	NO
8:00 AM	1287	20	194	No	0	194	No	NO
9:00 AM	1435	55	152	No	0	152	No	NO
10:00 AM	0	0	-	No	0	-	No	NO
11:00 AM	0	0	-	No	0	-	No	NO
12:00 PM	0	0	-	No	0	-	No	NO
1:00 PM	0	0	-	No	0	-	No	NO
2:00 PM	0	0	-	No	0	-	No	NO
3:00 PM	0	0	-	No	0	-	No	NO
4:00 PM	0	0	-	No	0	-	No	NO
5:00 PM	1728	78	100	No	0	100	No	NO
6:00 PM	1642	132	112	Yes	0	112	No	YES
7:00 PM	0	0	-	No	0	-	No	NO
8:00 PM	0	0	-	No	0	-	No	NO
9:00 PM	0	0	-	No	0	-	No	NO
10:00 PM	0	0	-	No	0	-	No	NO
11:00 PM	0	0	-	No	0	-	No	NO
Lanes	2	1			0			1
Speed	40	30			0		Satisfied?	Yes

Figure 4C-3. Warrant 3, Peak Hour

