# 6 VOORHIS POINT WATERFRONT IMPROVEMENTS

6 VOORHIS POINT SOUTH NYACK, NY

| NO. | DATE | REVISION | BY       | PREPARED BY               | l |
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|     |      |          |          | BUCKMAN                   | ŀ |
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|     |      |          |          | BUCKMAN ENGINEERING, PLLC | ŀ |
|     |      |          |          | 255 DEAN STREET           | l |

B. BUCKMAN

B. BUCKMAN

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PROJECT NO.

FOR APPROVAL
NOT FOR CONSTRUCTION

WARNING -- IT IS A VIOLATION OF SECTION 7229 OF THE STATE
EDUCATION LAW FOR ANY PERSON TO ALTER THIS DEVIATION LAW FOR ANY PERSON TO ALTER THIS DEVIATION AND THE SECTION AND THE STATE
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IDA PARKS 6 VOORHIS POINT SOUTH NYACK, NY 10960 6 VOORHIS POINT
WATERFRONT IMPROVEMENTS

PROJECT LOCATION

DRAWING TITLE

TITLE SHEET

DRAWING NO. T-01

DATE 2/4/2022

REVISION NO. 0

REVISION NO. 0

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| 13          | 09-200159 | TOPOGRAPHIC SURVEY (BY OTHERS) |

### **GENERAL NOTES**

- 1. ELEVATIONS ARE REFERENCED TO THE NORTH AMERICAN VERTICAL DATUM OF 1988
- 2. EXISTING TOPGRAPHIC AND BATHYMETRIC INFORMATION IS BASED ON FIELD SURVEY PERFORMED BY CONTROL POINT ASSOCIATES, INC. DATED OCTOBER 22, 2020 WITH REVISIONS DATED NOVEMBER 17, 2020 AND REPRESENT CONDITIONS OF THE SITE AT THE
- TIDAL DATA ARE COMPUTED FROM VDATUM 3.9 SOFTWARE PUBLISHED BY THE NATIONAL OCEANIC AND ATMOSPHERIC ADMINISTRATION.

| ۸.  | MEAN HIGHER HIGH WATER (MHHW)        | EL. +2.04   |
|-----|--------------------------------------|-------------|
| В.  | BEAN HIGH WATER (MHW)                | EL. +1.76   |
| C.  | MEAN LOW WATER (MLW)                 | EL -1.47    |
| D.  | MEAN LOWER LOW WATER (MLLW)          | EL1.64      |
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- THE OWNER HAS SECURED CERTAIN PERMITS REQUIRED BY FEDERAL AND STATE AUTHORITIES FOR THE PROPOSED ACTIVITIES. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO PERFORM THE WORK IN ACCORDANCE WITH THE TERMS AND CONDITIONS OF THE PERMITS. THE CONTRACTOR SHALL POST COPIES OF THE PERMITS AT THE SITE THROUGHOUT THE COURSE OF THE WORK. THE CONTRACTOR IS RESPONSIBLE TO OBTAIN PERMITS ASSOCIATED WITH THE LEGAL DISPOSAL OF CONSTRUCTION DEBRIS, THE CONTRACTOR SHALL SECURE REQUIRED LOCAL AUTHORIZATIONS AND PERMITS.
- PRIOR TO CONSTRUCTION AND FABRICATION OF CONSTRUCTION MATERIALS, THE CONTRACTOR SHALL VERIFY EXISTING CONDITIONS AND DIMENSIONS.

### DESIGN CRITERIA

- DESIGN CRITERIA ARE IN CONFORMANCE WITH "MINIMUM DESIGN LOADS AND ASSOCIATED CRITERIA FOR BUILDINGS AND OTHER STRUCTURES" PUBLISHED BY THE AMERICAN SOCIETY OF PROFESSIONAL ENGINEERS, 2016 (ASCE 7-16).
- 2. THE FACILITY IS DESIGNED FOR TWO CONDITIONS:
- A. SERVICE CONDITIONS IS THE MAXIMUM ALLOWABLE ENVIRONMENTAL CONDITIONS IN WHICH THE FACILITY MAY REMAIN IN OPERATION. WHEN ENVIRONMENTAL CONDITIONS ARE EXPECTED TO EXCEED SERVICE CONDITIONS, VESSELS SHALL BE MOVED TO A SAFE HARBOR.
- B. EXTREME CONDITIONS DENOTE THE MAXIMUM ALLOWABLE ENVIRONMENTAL CONDITIONS WITHIN WHICH THE STRUCTURE IS DESIGNED TO MAINTAIN ITS STRUCTURAL INTEGRITY USING SAFETY FACTORS FROM THE CODE.
- 3. DESIGN LIVE LOAD SHALL BE A UNIFORMLY DISTRIBUTED LOAD OF 50 PSF UNLESS NOTED OTHERWISE. NO LIVE LOADING IS ALLOWED FOR EXTREME CONDITIONS.
- 4. THE FACILITY IS DESIGNED TO ACCOMMODATE MOORING AND BERTHING LOADS ASSOCIATED WITH A BOAT WITH 40 FOOT OVERALL LENGTH. THE FLOATING DOCK IS DESIGNED TO ACCOMMODATE MOORING OF TWO DESIGN VESSELS, ONE ON EITHER SIDE OF THE DOCK,
- DESIGN WIND LOAD PARAMETERS FOR EXTREME CONDITIONS INCLUDE A BASIC WIND SPEED OF 110 MILES PER HOUR, EXPOSURE CATEGORY C, AND STRUCTURAL OCCUPANCY/RISK CATEGORY I. FOR SERVICE CONDITIONS, THE WIND SPEED IS LIMITED TO 55 MILES PER HOUR.
- 6. EXTREME ENVIRONMENTAL CONDITIONS ARE BASED ON A MAXIMUM WATER LEVEL AT ELEVATION +7.0 PER FEMA FIRM PANELS FOR THE SITE. THE DESIGN WAVE IS CHARACTERIZED BY A SIGNIFICANT WAVE HEIGHT OF 4.2 FEET AND A PERIOD OF 3.1 SECONDS.

### STRUCTURAL STEEL

- 1. STRUCTURAL STEEL SHALL COMPLY WITH THE CURRENT EDITION OF THE "STEEL CONSTRUCTION MANUAL", PUBLISHED BY THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION.
- WELDING SHALL CONFORM TO THE 'STRUCTURAL WELDING CODE STEEL', AS ADOPTED BY THE AMERICAN WELDING SOCIETY (AWS D1.1).
- 3. WELDING ELECTRODES SHALL BE E70XX AND COMPLY WITH AWS A5.1 AND AWS A5.5.
- STRUCTURAL STEEL PLATES AND SHAPES SHALL CONFORM TO ASTM A572, GRADE 50. STEEL TUBES SHALL CONFORM TO ASTM A500, GRADE B.
- HIGH-STRENGTH BOLTS, NUTS AND WASHERS SHALL BE IN ACCORDANCE WITH "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A 325 BOLTS".
- HIGH STRENGTH STRUCTURAL BOLTS SHALL CONFORM TO ASTM A325 WITH HEX HEADS. NUTS SHALL CONFORM TO ASTM A583.
- WASHERS BETWEEN THE BOLT HEAD OR NUT AND STRUCTURAL STEEL SHALL BE HIGH STRENGTH WASHERS CONFORMING TO ASTM F436. WASHERS AGAINST TIMBER SHALL BE COMMON DOCK WASHERS.
- B. ALL STEEL COMPONENTS SHALL BE HOT-DIP GALVANIZED UNLESS NOTED OTHERWISE. GALVANIZING SHALL CONFORM TO ASTM A123 OR ASTM 153 AS APPLICABLE.
- 9. BOLTED CONNECTIONS SHALL USE % DIAMETER A 325 HIGH STRENGTH BOLTS UNLESS
- CONNECTIONS SHALL BE DESIGNED AND DETAILED BY THE STEEL FABRICATOR EXCEPT FOR THOSE SPECIFICALLY DETAILED IN THE CONTRACT DOCUMENTS.

### IMBER

- VISUALLY GRADED STRUCTURAL LUMBER AND WOOD CONSTRUCTION SHALL CONFORM TO THE REQUIREMENTS OF THE "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION" AND SUPPLEMENT, "DESIGN VALUES FOR WOOD CONSTRUCTION", NDS 2018, PUBLISHED BY THE NATIONAL FOREST AND PAPER ASSOCIATION.
- 2. TIMBER SHALL MEET THE REQUIREMENTS OF THE SOUTHERN PINE INSPECTION BUREAU, "STANDARD GRADING RULES FOR SOUTHERN PINE LUMBER". TIMBER GRADE SHALL BE NO 1 OR BETTER.
- ALL TIMBER SHALL BE TREATED WITH ALKALINE COPPER QUATERNARY (ACQ) TO A PENETRATION OF 0.60 POUNDS PER CUBIC FOOT.
- 4. CUT AND DRILLED EXPOSED TIMBER AND LUMBER SURFACES SHALL BE LIBERALLY RECOATED BY BRUSH WITH A FIELD TREATMENT ACCEPTED BY THE ENGINEER OF RECORD.

### STEEL PILES

- STEEL PIPE PILE MATERIAL SHALL CONFORM TO THE REQUIREMENTS OF ASTM A252, GRADE 3. PILES SHALL BE LONGITUDINALLY WELDED WITH SEAMS SPECIFIED AS COMPLETE PENETRATION WELD.
- 2. STEEL PIPE PILES SHALL BE DRIVEN TO A MINIMUM CAPACITY OR MINIMUM EMBEDMENT LENGTH AS INDICATED ON THE CONTRACT DRAWINGS WHICHEVER IS LARGER.
- DO NOT FIELD SPLICE PILE IN LOWER 40 FEET, BOTH, UPPER AND LOWER SECTIONS OF PILE ENDS, SHALL BE SMOOTH, SQUARE AND FLAT PRIOR TO SPLICING.
- 4. ALL WELDING REQUIRED FOR THE STEEL PIPE PILES SHALL BE FULL PENETRATION WELDS CONFORMING TO AWS D1.1 WELDING CODE AND SHALL BE CAPABLE OF DEVELOPING THE PILE CROSS-SECTION IN TENSION AND BENDING.
- 5. STEEL PIPE PILES SHALL BE SHOP COATED, ON OUTER SURFACES ONLY, TO A MINIMUM OF TEN (10) FEET BELOW THE DESIGN MUDLINE DEPTH ELEVATION OR AS INDICATED ON THE

### PROTECTIVE COATING

- MATERIAL USED FOR FACTORY EPOXY COATING OF ALL SCHEDULED SURFACES SHALL BE BAR-RUST 235 MULTI-PURPOSE EPOXY COATING AS MANUFACTURED BY DEVOE COATINGS OR EQUIVALENT ACCEPTED BY THE ENGINEER OF RECORD.
- 2. FIELD TOUCH—UP COATING SHALL BE IDENTICAL TO FACTORY COATING AND APPLIED IN ACCORDANCE WITH THE MANUFACTURER'S REQUIREMENTS.
- 3. PROTECTIVE COATING TOP COAT SHALL BE BLACK UNLESS OTHERWISE INDICATED ON THE DRAWINGS OR BY THE CHINER,
- SURFACES SHALL BE PREPARED IN STRICT ACCORDANCE WITH THE PROTECTIVE COATING SYSTEM MANUFACTURER'S WRITTEN INSTRUCTIONS.
- 5. THE PROTECTIVE COATING SHALL BE INSTALLED IN STRICT ACCORDANCE WITH MANUFACTURER'S WRITTEN INSTRUCTIONS. COATING IS TO BE APPLIED IN TWO COATS TO ACHIEVE A MINIMUM OVERALL DRY FILM THICKNESS OF 15 MILS.
- ALL HOLIDAYS OR OTHER IMPERFECTIONS IN THE COATING SHALL BE REMOVED OR REPAIRED AT THE CONTRACTORS EXPENSE PRIOR TO FINAL ACCEPTANCE OF THE WORK.

### PILE DRIVING

- INSTALL PILES USING VIBRATORY METHODS TO CAPACITY AND TOE ELEVATIONS WITHOUT DAMAGING THE PILE HEAD. WHERE REQUIRED, CUT OFF HEADS OF PILES ACCURATELY IN ACCORDANCE WITH THE CONTRACT DOCUMENTS AFTER COMPLETION OF DRIVING.
- DRIVE THE PILES STRAIGHT AND TRUE AT INDICATED LOCATIONS, WITH DEVIATION FROM THE LONGITUDINAL AXIS OF NOT MORE THAN 1/4 INCH PER FOOT.
- 3. LOCATE THE PILES WITHIN 3 INCHES OF THE POSITIONS INDICATED ON THE DRAWINGS.
- 4. WITHDRAW PILES THAT ENCOUNTER UNDERGROUND OBSTRUCTIONS SUFFICIENT TO IMPEDE PILE DRIVING. RE-DRIVE AS CLOSE AS POSSIBLE TO ORIGINAL POSITION, SUBJECT TO REVIEW OF THE OWNER. REMOVE PILES WHICH BREAK OR DRIVE OUT OF LINE. DRIVE ANOTHER PILE IN ITS PLACE.
- DRIVE PILES TO THEIR FULL PENETRATION WITHOUT BENDING, RUPTURING, OR MODERATELY DAMAGING THE PILES. IF FAILURE IN THE ABOVE RESPECTS IS ENCOUNTERED, PULL THE PILE AND DRIVE A NEW PILE AT NO ADDITIONAL COST TO THE OWNER.
- 6. JETTING TO ASSIST PENETRATION WILL NOT BE PERMITTED UNLESS ACCEPTED BY THE ENGINEER OF RECORD. WHERE ACCEPTED, PRE-DRILLING TO ASSIST PENETRATION MAY BE USED WHERE EXTREME DRIVING RESISTANCE IS ENCOUNTERED, OR WHERE VIBRATIONS FROM DRIVING MAY BE DETRIMENTAL TO ADJACENT STRUCTURES.
- WHERE PILES ARE PUSHED UP BY PRESSURE FROM DRIVING OF ADJACENT PILES, RE-DRIVE AS REQUIRED AND AT NO ADDITIONAL COST TO THE OWNER.

### ABBREVIATIONS AND SYMBOLS ADDITIONAL

ADD'L

| ALT.     | ALTERNATE              |
|----------|------------------------|
| BOT.     | BOTTOM                 |
| CLR.     | CLEAR                  |
| COL      | COLUMN                 |
| CONC.    | CONCRETE               |
| DWG.     | DRAWING                |
| EA.      | EACH                   |
| EF.      | EACH FACE              |
| EL.      | ELEVATION              |
| EQ.      | EQUAL                  |
| EXIST.   | EXISTING               |
| FT. OR ' | FOOT                   |
| GALV.    | GALVANIZED             |
| H.D.G.   | HOT-DIP GALVANIZED     |
| I.D.     | INSIDE DIAMETER        |
| IN. OR " | INCHES                 |
| KSI.     | KIPS PER SQUARE INCH   |
| LONG.    | LONGITUDINAL           |
| MAX.     | MAXIMUM                |
| N.       | NORTH                  |
| N.I.C.   | NOT IN CONTRACT        |
| O.C.     | ON CENTER              |
| O.D.     | OUTSIDE DIAMETER       |
| OPP.     | OPPOSITE               |
| PL.      | PLATE                  |
| PSI.     | POUNDS PER SQUARE INCH |
| PSF.     | POUNDS PER SQUARE FOOT |
| R        | RADIUS                 |
| REINF.   | REINFORCING            |
| REF.     | REFERENCE              |
|          | REQUIRED               |
| S.       | SOUTH                  |
| SECT.    | SECTION                |
| SIM.     | SIMILAR                |
| SPA.     | SPACING                |
| SS.      | STAINLESS STEEL        |
| TYP.     | TYPICAL                |
| U.O.N.   | UNLESS OTHERWISE NOTED |
| €.       | CENTERLINE             |
| •        | DIAMETER               |
| ±        | PLUS OR MINUS          |
|          | NORTH ARROW            |
|          | NORTH ARROW            |
|          |                        |

### DRAWING CONVENTIONS

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|     |      |          |    | BUCKMAN ENGINEERING BULG                     |
|     |      |          |    | BUCKMAN ENGINEERING, PLLC<br>255 DEAN STREET |

MADE BY B. BUCKMAN DRAWN BY B. BUCKMAN CHECKED BY B. BUCKMAN

20036

PROJECT NO.

FOR APPROVAL NOT FOR CONSTRUCTION

PREPARED FOR

IDA PARKS 6 VOORHIS POINT SOUTH NYACK, NY 10960 TITLE OF PROJECT

6 VOORHIS POINT WATERFRONT IMPROVEMENTS

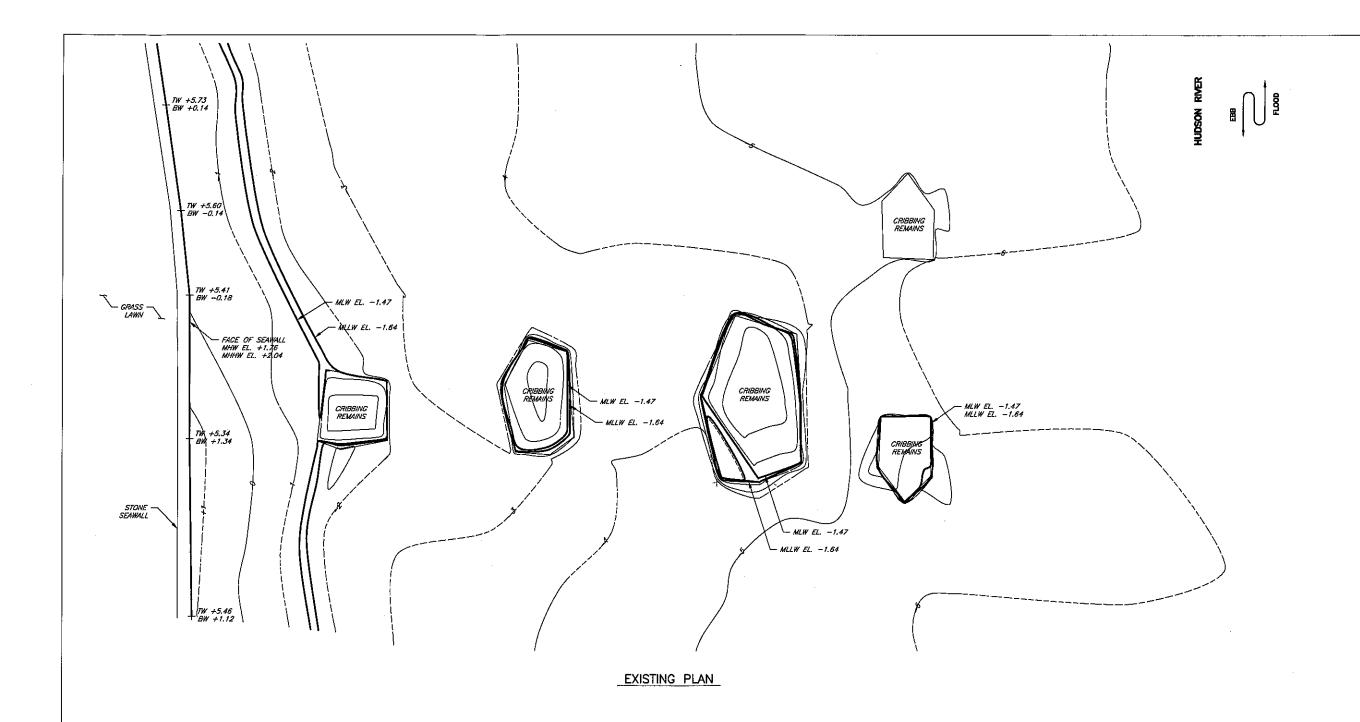
PROJECT LOCATION

6 VOORHIS POINT SOUTH NYACK, NY DRAWING TITLE

**GENERAL NOTES** 

DRAWING NO. G - 01DATE 2/4/2022 REVISION NO.

SHEET NO. 2 of 13



- 1. ELEVATIONS ARE REFERENCED TO THE NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD86).
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| $\vdash$ |      |          |    | BUCKMAN ENGINEERING, PLLC             |
|          |      |          |    | 255 DEAN STREET<br>BROOKLYN, NY 11217 |

|                 | MADE BY     |    |         |
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|                 |             | В. | BUCKMAN |
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| ucation law for any person to alter this drawing in any<br>y unless acting under the drection of a licensed          |
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| or seal and the notation "altered by followed by their<br>nature, the date of alteration, and a specific description |
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| 6 VOORHIS POINT       |
| SOUTH NYACK, NY 10960 |

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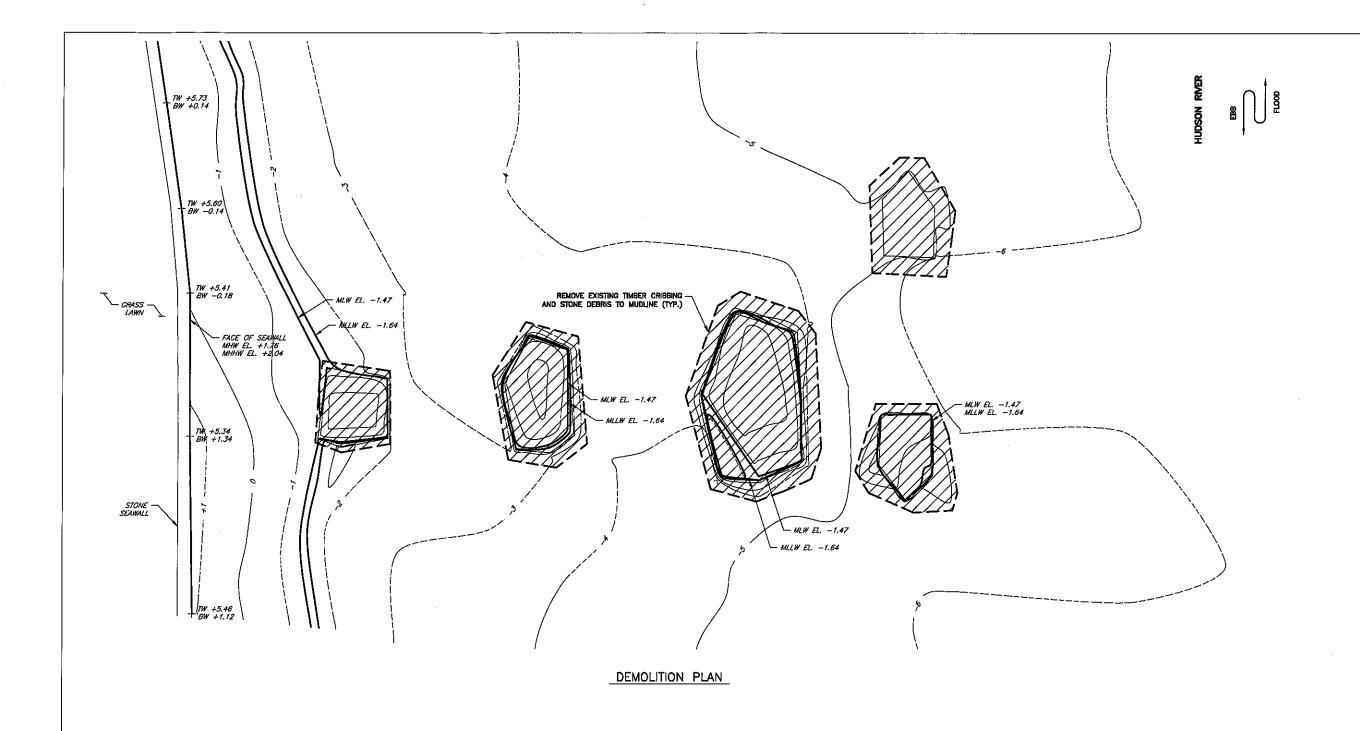
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| 6 VOORHIS POINT<br>SOUTH NYACK, NY |

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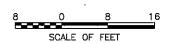
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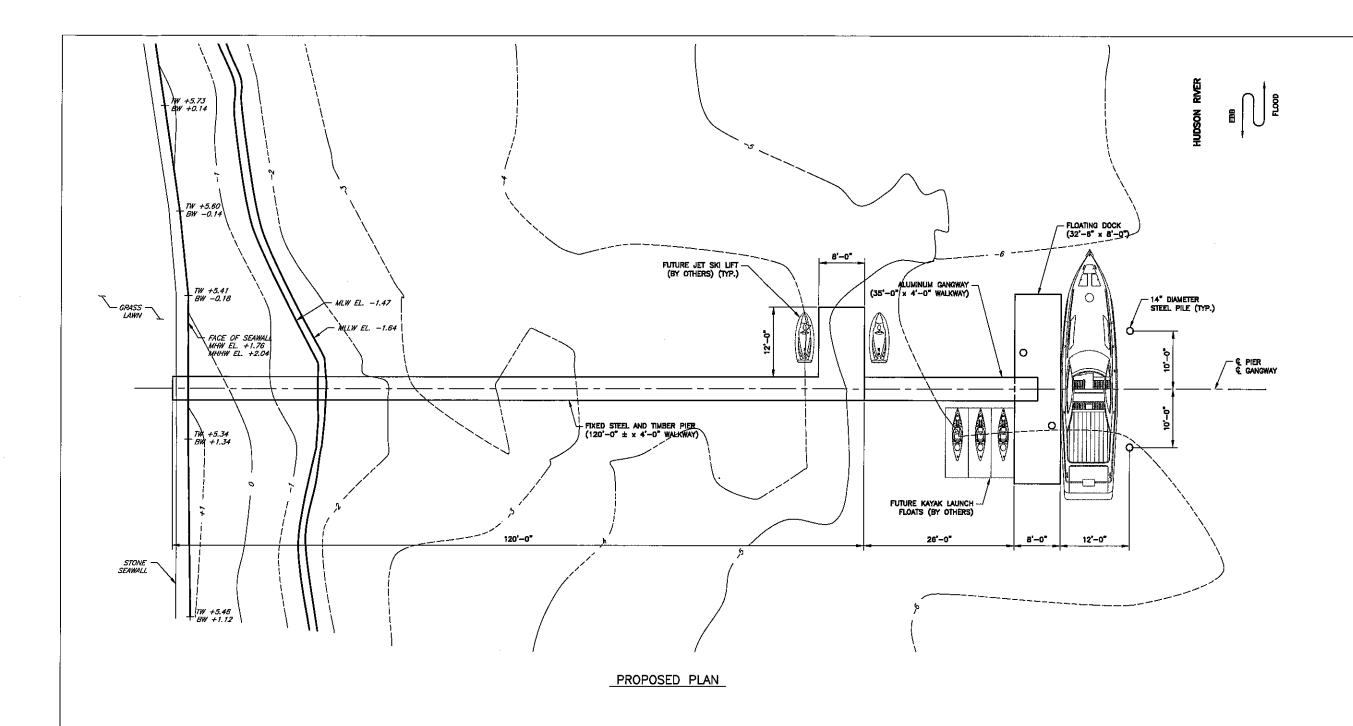
### NOTE

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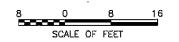


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|   |         |             | BUCKMAN ENGINEERING, PLLC             | B. BUCKMAN        | MARNING - IT IS A VIOLATION OF SECTION 7208 OF THE STATE EDUCATION LAW FOR ANY PERSON TO ALTER THIS REMINING IN A MARKET THE DEPOSITION OF THE STATE OF THE SECTION OF THE SECTIO | 1 200 // 11/AOK, 11/ 10000               | 6 VOORHIS POINT         |                 | 0             |
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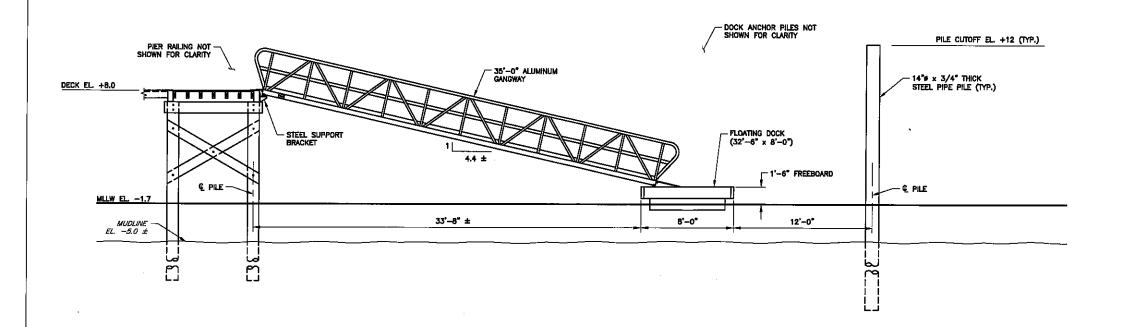
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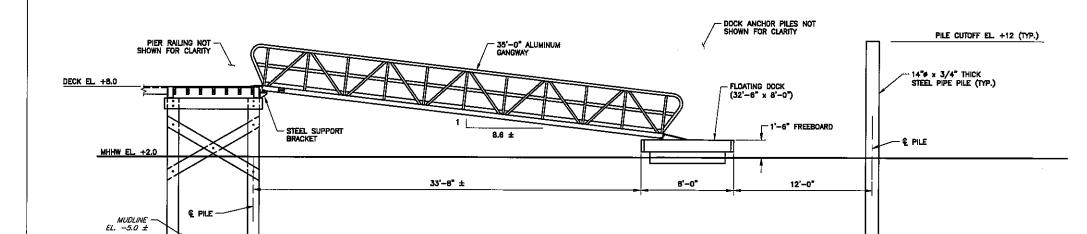
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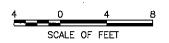
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| . |        |               | BUCKMAN ENGINEERING, PLLC<br>255 DEAN STREET | PROJECT NO. | POFESSIONAL | WAY UNLESS ACTING UNDER THE DIRECTION OF A LICENSEE PROFESSIONAL ENGINEER. THE ALTERONG ENCINEER SHALL AFFOR    |  | 6 VOORHIS POINT<br>SOUTH NYACK, NY |               | SHEET NO.     |
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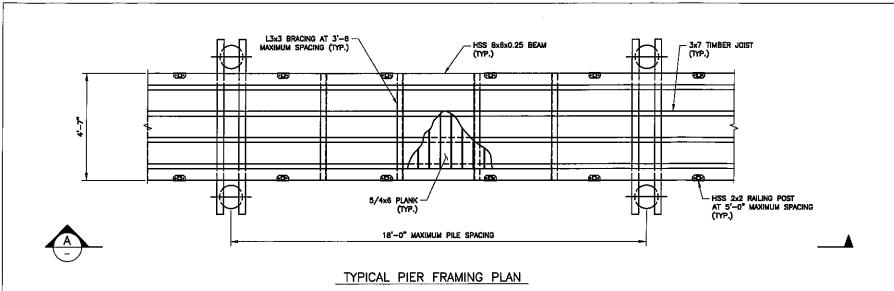


GANGWAY POSITION AT MLLW

GANGWAY POSITION AT MHHW

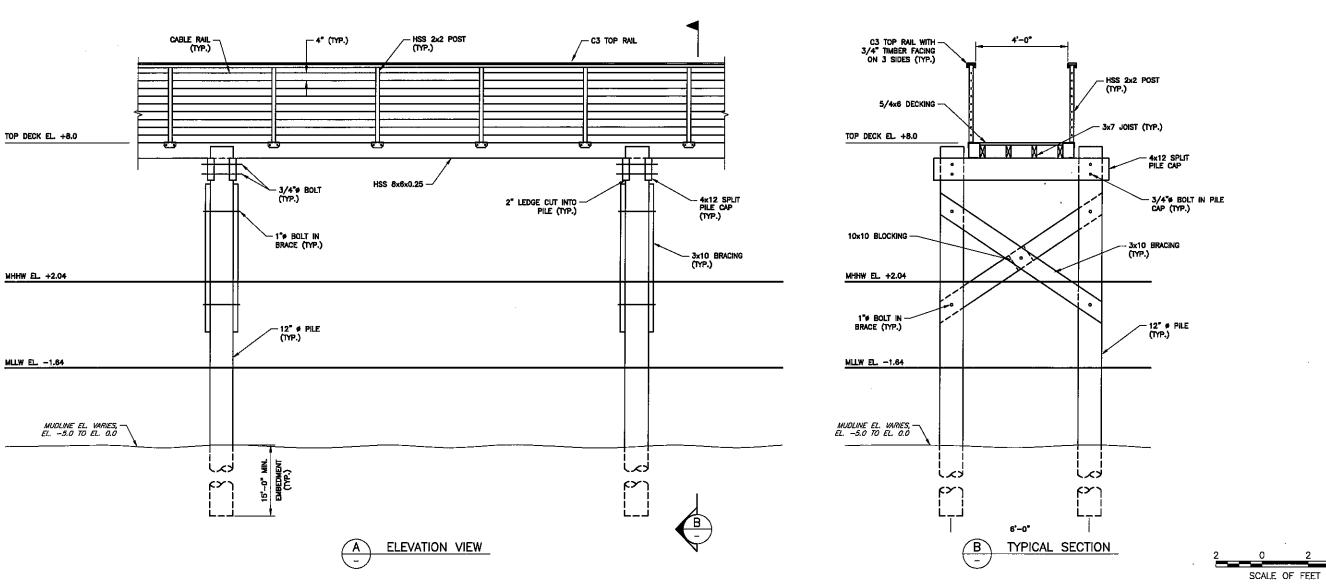


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|   |                    |                |     | BUCKMAN                   |           | B. BUCKMAN | VV L                                   | ▼   NOT FOR CONSTRUCTION   |                       | WATERFRONT IMPROVEMENTS |                         | DATE 2/4/202    | 22       |
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|   |                    |                |     |                           |           | B. BUCKMAN |  | WARNING - IT IS A VIOLATION OF SECTION 7209 OF THE STATE   | SOUTH NYACK, NY 10960 |                         |                         | REVISION NO.    | 1        |
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### NOTES

- UNLESS NOTED OTHERWISE, PIER FRAMING AND DECKING ARE BUILT FROM PRESSURE TREATED TIMBER.
- 2. FOR DECK FRAMING DETAILS, REFER TO DRAWING "PIER DETAILS 2".
- Cable Railing Railing shall be 3/16 inch minimum diameter Stainless Steel Cable. The Cable Railing Shall be provided as Manufactured Kit including anchorage Hardware to post and tensioning Hardware.
- 4. CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR REVIEW AND APPROVAL SHOP DRAWINGS OF THE POSTS, TOP RAILING, AND CABLE RAILS TO DEMONSTRATE SUPPORTING MEMBERS ARE PROPERLY COORDINATED WITH CABLE RAILING. SHOP DRAWINGS SHALL SHOW THE DETAILS OF THE ASSEMBLY AND CONNECTION DETAILS AS WELL AS THE PROCEDURE FOR INSTALLING THE CABLE RAILS.
- S. CONTRACTOR MAY SUBMIT AN ALTERNATE POST AND RAIL SYSTEM TO THE ENGINEER AND OWNER FOR REVIEW AND APPROVAL PROVIDED THE SYSTEM HAS SIMILAR TOP RAIL SIZE, POST SIZE AND POST SPACING DEPICTED ON THESE PLANS. THE ALTERNATION STEM SHALL BE SIMILAR IN APPEARANCE TO THE CURRENTLY PROPOSED RAILING TO BE DETERMINED SOLELY BY THE ENGINEER AND OWNER. THE CONTRACTOR MUST DEMONSTRATE THAT THE ALTERNATIVE SYSTEM CAN SAFELY SUPPORT THE RAILING LOADS PRESCRIBED IN ASCE 7–18.



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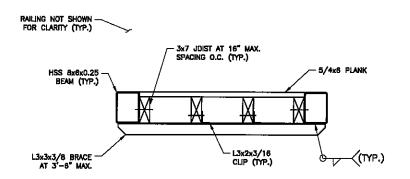
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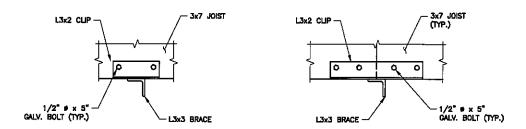
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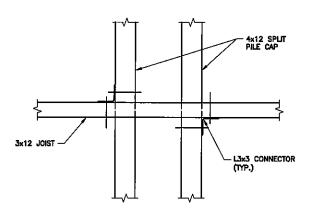
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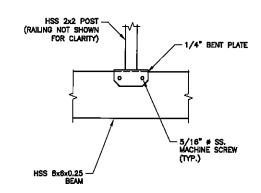
SECTION AT L3x3 BRACE
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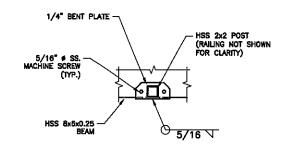
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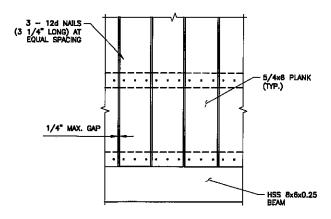
JOIST-CAP CONNECTION DETAIL SCALE (A)



## RAILING POST MOUNTING DETAIL — ELEVATION SCALE (A)



## RAILING POST MOUNTING DETAIL - PLAN SCALE (A)

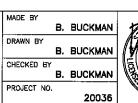


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FOR APPROVAL NOT FOR CONSTRUCTION

WARNING — IT IS A VIGLATION OF SECTION 7209 OF THE STAT DEUCATION LAW FOR ANY PERSON TO ALTER THES DRAWING IN AN WAY LALESS ACTION LARGER THE DREATION OF A LICEOUS PROFESSIONED DEARNER. THE ALTERNON DEWLERS SALE ART HER IDA PARKS 6 VOORHIS POINT SOUTH NYACK, NY 10960

PREPARED FOR

TITLE OF PROJECT

6 VOORHIS POINT
WATERFRONT IMPROVEMENTS

PROJECT LOCATION

6 VOORHIS POINT SOUTH NYACK, NY

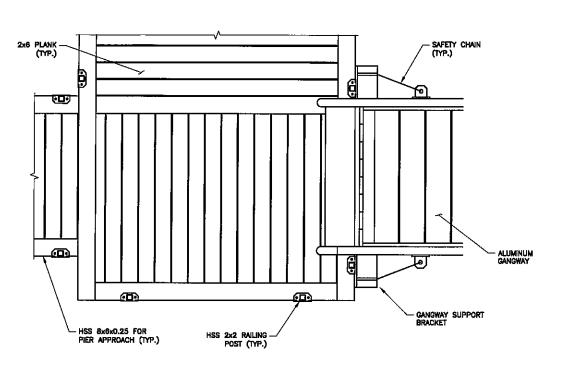
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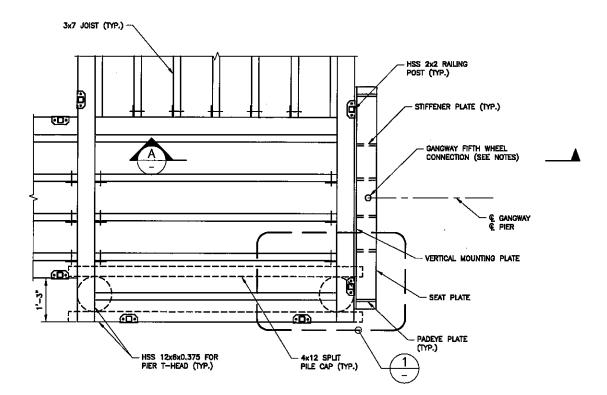
1. FOR TYPICAL PIER FRAMING PLAN, REFER TO DRAWING "PIER DETAILS 1".

2. L3x3x3/16 CONNECTION ANGLE SHALL CONFORM TO ASTM 368 AND SHALL BE HOT-DIP GALYANIZED PER ASTM A123. FASTEN USING TWO 3/4" BOLTS INTO BOTH THE JOIST AND SPLIT CAP BEAM. PROVIDE STANDARD WASHER BETWEEN BOLT HEAD AND ANGLE. PROVIDE OVERSIZE PLATE WASHER OR OGEE WASHER BETWEEN NUT AND TIMBER BEAM. BOLTS, NUTS, AND WASHERS SHALL BE HOT-DIP GALYANIZED PER ASTM A153.

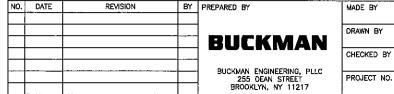
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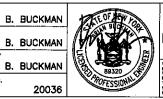


PIER PLAN AT GANGWAY BRACKET SCALE (A)



PIER FRAMING AT GANGWAY BRACKET SCALE (A)





### FOR APPROVAL NOT FOR CONSTRUCTION

### IDA PARKS 6 VOORHIS POINT SOUTH NYACK, NY 10960

PREPARED FOR

### TITLE OF PROJECT 6 VOORHIS POINT WATERFRONT IMPROVEMENTS

6 VOORHIS POINT SOUTH NYACK, NY

SECTION THROUGH BRACKET

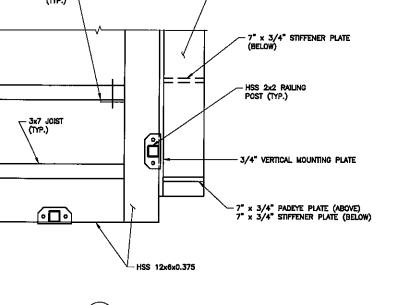
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## SCALE OF INCHES SCALE OF FEET

### **NOTES**

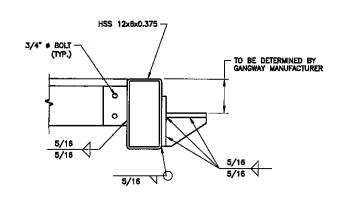
- CONTRACTOR SHALL PROVIDE TO THE ENGINEER FOR REVIEW AND APPROVAL SHOP
  DRAWINGS OF THE GANGWAY SUPPORT BRACKET WITH NECESSARY MODIFICATIONS INDICATED
  BY THE GANGWAY MANUFACTURE TO ACCOMMODATE THEIR SYSTEM INCLUDING, BUT NOT
  LIMITED TO, FIFTH WHEEL CONNECTION, SAFETY CHAIN CONNECTION, AND HEIGHT OF SEAT
  PLATE BELOW TOP OF DECK.
- THE GANGWAY SUPPORT BRACKET SHOP DRAWINGS SHALL ALSO SHOW THE CONNECTIONS FOR THE RAILING POST.
- 3. THE GANGWAY SUPPORT BRACKET AND ITS CONNECTION HARDWARE SHALL BE HOT-DIP GALVANIZED.

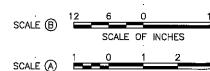


-7" x 3/4" SEAT PLATE

4" x 3/8" STRINGER CONNECTION PLATE

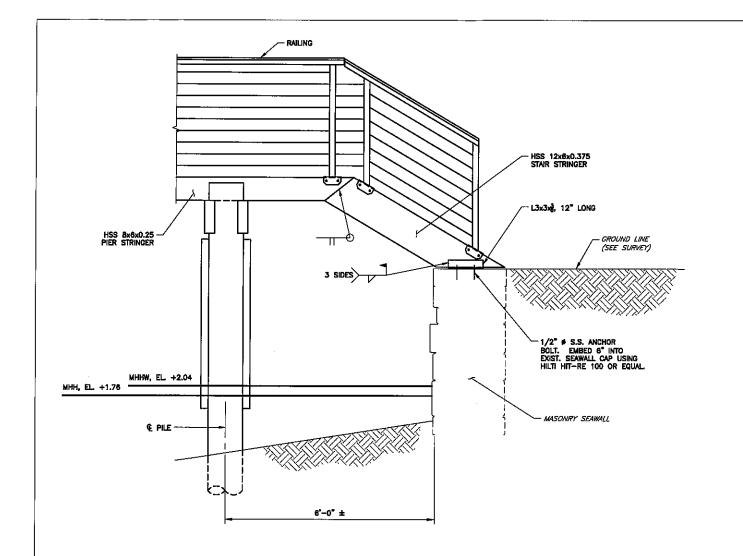


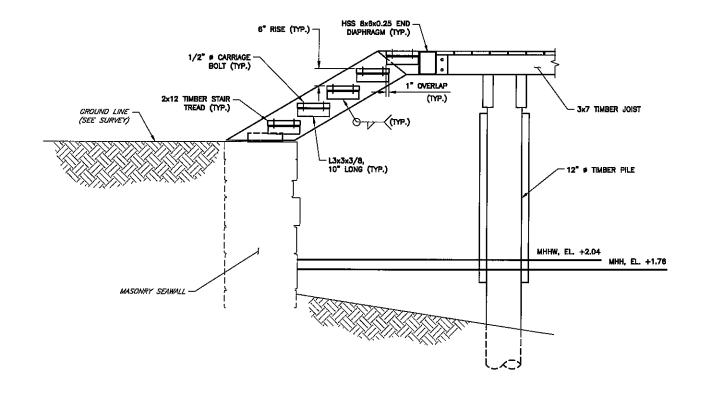




PIER DETAILS 3

DRAWING NO. MR-07 2/4/2022 REVISION NO. SHEET NO. 9 of 13





STAIR ELEVATION VIEW

SECTON THROUGH STAIR TREADS

### NOTE

DRAWING TITLE

CONTRACTOR MAY SUBMIT AN ALTERNATE STAIR DESIGN TO THE ENGINEER AND OWNER FOR REVIEW AND APPROVAL. THE ALTERNATE SYSTEM SHALL BE SIMILAR IN APPEARANCE TO THE CURRENTLY PROPOSED STAIR DESIGN, TO BE DETERMINED SOLELY BY THE ENGINEER AND OWNER. THE CONTRACTOR MUST DEMONSTRATE THAT THE ALTERNATIVE SYSTEM CAN SAFELY SUPPORT THE RAILING LOADS PRESCRIBED IN ASCE 7—16 AND SATISFIES THE REQUIREMENTS OF IBC.



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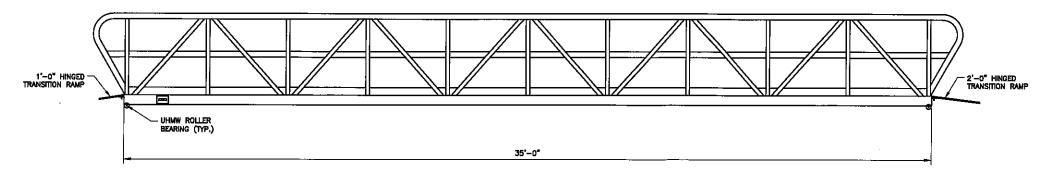
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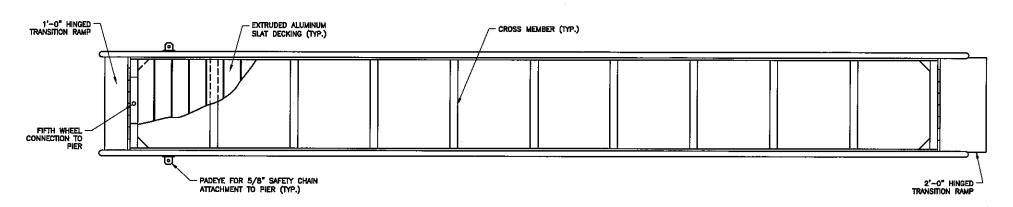
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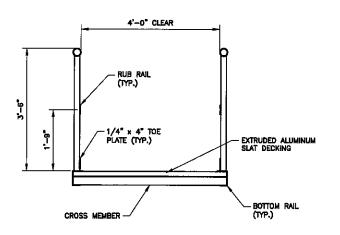
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### GANGWAY ELEVATION

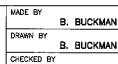


### GANGWAY PLAN



GANGWAY TYPICAL SECTION

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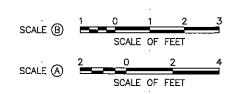
## 6 VOORHIS POINT WATERFRONT IMPROVEMENTS

PROJECT LOCATION

6 VOORHIS POINT SOUTH NYACK, NY

### GANGWAY NOTES

- THE WORK COVERED UNDER THIS SECTION SHALL CONSIST OF MANUFACTURING AND SUPPLYING OF A PREFABRICATED ALLIMINUM GANGWAY AS SHOWN IN THE PLANS. ALL MATERIALS SHALL BE MANUFACTURED OR DISTRIBUTED BY RAVENS MARINE OR APPROVED EQUAL.
- 2. THE GANGWAY SHALL BE DESIGNED WITH MINIMUM SAFETY FACTORS ON WORKING STRESS WHICH CONFORM TO THOSE SET FORTH IN THE LATEST ISSUE OF THE ALUMINUM ASSOCIATION "SPECIFICATIONS FOR ALLUMINUM STRUCTURES" FOR BUILDINGS AND SIMILAR TYPE STRUCTURES. TO ENSURE SPECIFIED CRITERIA HAVE BEEN MET, THE CONTRACTOR SHALL SUBMIT THE FOLLOWING:
- A. SHOP DRAWINGS SHOWING DIMENSIONAL LAYOUT OF THE GANGWAY.
- B. SHOP DRAWINGS SHOWING CONNECTION DETAILS INCLUDING, BUT NOT LIMITED TO, ROLLER BERINGS, FIFTH WHEEL CONNECTION, RAILINGS, TRANSITION RAMPS, AND PLANKS.
- C. ENGINEERING CALCULATIONS SHOWING COMPLIANCE WITH THE DESIGN CRITERIA SPECIFIED HEREIN. CALCULATIONS MUST BE SIGNED AND SEALED BY AN ENGINEER LICENSED TO PRACTICE IN THE STATE OF NEW YORK.
- 3. ALUMINUM EXPRESIONS FOR GANGWAY STRUCTURES SHALL BE ALUMINUM ALLOY 6061—T6 EXTRUDED IN ACCORDANCE WITH THE REQUIREMENTS OF FEDERAL SPECIFICATION QQ—A—200.
- 4. STAINLESS STEEL BOLTS, NUTS, WASHERS, AND SCREWS SHALL BE TYPE 304.
- ROLLER BEARINGS SHALL BE UHMW POLYURETHANE WITH BLACK ULTRAVIOLET LIGHT INHIBITOR ADDED.
- 6. EXTRUDED RIBBED DECKING SHALL BE DESIGNED TO WITHSTAND A COMBINED DEAD LOAD AND LIVE LOAD OF 100 POUNDS PER SQUARE FOOT PER INDIMIDUAL SLAT WITH AN ALLOWABLE DEFLECTION OF L/180 WHERE "L" IS THE SPAN OF THE DECKING.
- 7. HANDRAILS SHALL BE DESIGNED TO WITHSTAND A CONCENTRATED LOAD OF 200 POUNDS OR A LINEAR LOAD OF 50 POUNDS PER LINEAR FOOT, WHICHEVER PRODUCES MAXIMUM EFFECT. INTERMEDIATE RAILS SHALL BE DESIGNED FOR A CONCENTRATED LOAD OF OPOUNDS. THE RAILS SHALL BE DESIGNED TO WITHSTAND THE LOADS ACTING IN ANY DIRECTION.
- 8. GANGWAYS SHALL BE DESIGNED TO WITHSTAND A UNIFORM LIVE LOAD OF 50 POUNDS PER SQUARE FOOT APPLIED VERTICALLY. ALLOWABLE DEFLECTION SHALL BE A MAXIMUM OF L/180 WHERE "L" IS THE LENGTH OF THE GANGWAY.
- 9. DECKING SHALL BE EXTRUDED ALUMINUM SLATS TO PROVIDE A NON-SKID SURFACE AND SHALL NOT EXCEED 9 INCHES IN WIDTH AND NOT MORE THAN 3/8 INCH AIR SPACE BETWEEN THE SLATS. THE LEGS OF EACH SLAT SHALL BE WELDED TO THE SIDE MEMBERS WITH A MINIMUM OF 1-1/4 INCHES OF WELD PER LEG. DECKING SLATS SHALL BE PLACED TRANSVERSELY ON THE GANGWAY.
- 10. HINGED TRANSITION PLATES SHALL HAVE A NON-SKID SURFACE MADE OF DIAMOND PLATE, RIBBED PLATE, SLIP-NOT COATING, OR APPROVED EQUAL.
- 11. HINGE MOUNT EXTRUSIONS SHALL BE WELDED TO THE FRAME OF THE DOCK WITH A CONTINUOUS FILLET WELD UNLESS OTHERWISE SHOWN ON THE PLANS.
- 12. ANY POTENTIALLY CORROSIVE INSTALLATION OF DISSIMILAR METALS SHALL BE PROPERLY INSULATED TO MINIMIZE OR ELIMINATE CORROSION IN A MARINE ENVIRONMENT.
- 13. CANGWAYS SHALL BE SECURELY FASTENED TO THE DOCK. MANUFACTURER SHALL DESIGN
  THE FIFTH WHEEL CONNECTION AND INDICATE ANY MODIFICATIONS TO THE STEEL SUPPORT
  BRACKET NECESSARY TO ACCOMMODATE THE GANGWAY CONNECTION.



GANGWAY DETAILS

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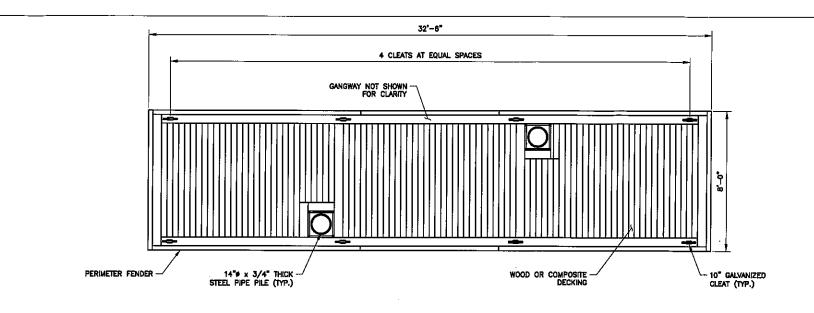
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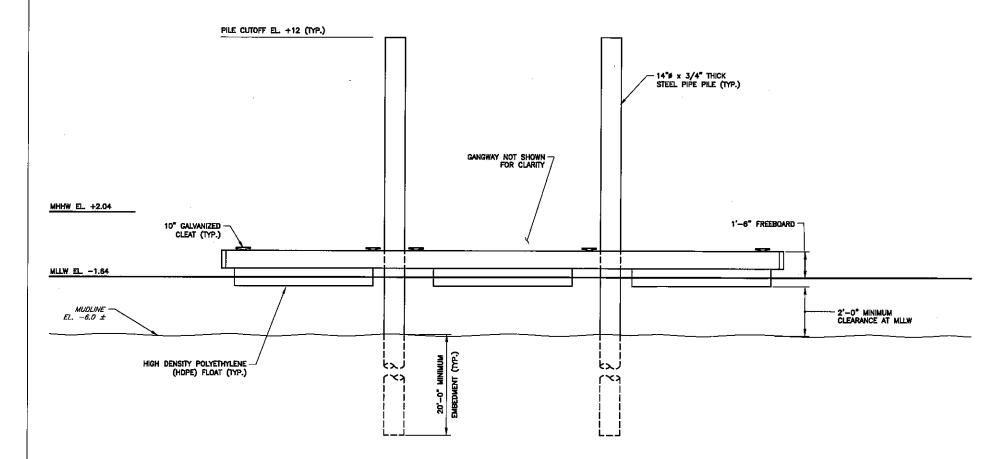
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FLOATING DOCK PLAN





### FLOATING DOCK ELEVATION

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PREPARED FOR

6 VOORHIS POINT WATERFRONT IMPROVEMENTS

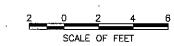
TITLE OF PROJECT

6 VOORHIS POINT SOUTH NYACK, NY

### FLOATING DOCK NOTES

- THE WORK COVERED UNDER THIS SECTION SHALL CONSIST OF MANUFACTURING AND SUPPLYING OF A FLOATING DOCK SYSTEM AS SHOWN IN THE PLANS.
- THE FLOATING DOCK SYSTEM SHALL BE COMPLETELY PREFABRICATED BY THE CONTRACTOR IN THEIR PLANT AND DELIVERED READY FOR ASSEMBLY AT THE SITE.
- 3. THE FLOATING DOCK SHALL BE DESIGNED WITH LOADS AND MINIMUM SAFETY FACTORS ON WORKING STRESS WHICH CONFORM TO THOSE SET FORTH IN THE LATEST ISSUE OF ASCE MANUALS AND REPORTS ON ENGINEERING PRACTICE NO. 50 PLANNING AND GUIDELINES FOR SMALL CART HARBORS. TO ENSURE SPECIFIED CRITERIA HAVE BEEN MET, THE CONTRACTOR SHALL SUBMIT THE FOLLOWING:
  - A. SHOP DRAWINGS SHOWING DIMENSIONAL LAYOUT OF THE FLOATING DOCK.
- A. SHOP DRAWINGS SHOWING CONNECTION DETAILS INCLUDING, BUT NOT LIMITED TO,
  MOORING HARDWARE, FENDERING, DECKING, INTERNAL PILE WELLS, AND PILE GUIDES.

  C. ENGINEERING CALCULATIONS SHOWING COMPLIANCE WITH THE DESIGN CRITERIA SPECIFIED
  HEREIN. CALCULATIONS MUST BE SIGNED AND SEALED BY AN ENGINEER LICENSED TO
  PRACTICE IN THE STATE OF NEW YORK.
- LIVE LOAD FOR THE FLOATING DOCK SHALL BE 50 POUNDS PER SQUARE FOOT ACTING ON THE DOCK AND THE GANGWAY. DEAD LOAD FOR THE FLOATING DOCK SHALL INCLUDE ALL DOCK COMPONENTS PLUS THE REACTION FROM THE GANGWAY.
- 4. THE FLOATING DOCK SHALL BE CONNECTED AND DESIGNED SO AS TO SAFELY WITHSTAND A MINIMUM WIND PRESSURE OF 20 POUNDS PER SQUARE FOOT AGAINST THE AVERAGE PROFILE OF THE BERTHED VESSELS AND EXPOSED PORTIONS OF THE FLOATING DOCK SYSTEM SUPERIMPOSED WITH A 1.0 FOOT WAVE HEIGHT WITH WAVE PERIODS LESS THAN 2 SECONDS. DOCK DESIGN SHALL BE VERIFIED TO BE ADEQUATE TO RESIST LOADS PRODUCED BY THE 100-YEAR STORM EVENT WITH NO VESSELS BERTHED.
- 5. THE FLOTATION UNITS SHALL BE DESIGNED TO MAINTAIN THEIR DESIRED BUCYANCY AND FREEBOARD EVEN IF STRUCTURALLY DAMAGED. CONNECTIONS OF THE FLOTATION SYSTEM WILL BE SO DESIGNED THAT THE SYSTEM WILL EFFECTIVELY ACT AS A SINGLE UNIT. A RIGID SYSTEM IS REQUIRED WHICH WILL NEITHER RACK NOR TWIST IN TORSION UNDER EXTREME DESIGN CONDITIONS.
- 6. The contractor shall demonstrate by in water—tests or written calculations that the system will not tilt, list, or pitch more than six degrees from horizontal when a concentrated load of 500 pounds is placed anywhere on the
- 7. MOORING CLEATS SHALL BE GALVANIZED CAST STEEL, THRU BOLTED INTO THE DOCK STRUCTURE.
- B. ALL BOLTS, NUTS, AND WASHERS SHALL BE STAINLESS STEEL OR HOT-DIP GALVANIZED.
- ANY POTENTIALLY CORROSIVE INSTALLATION OF DISSIMILAR MATERIALS SHALL BE PROPERLY INSULATED TO MINIMIZE OR ELIMINATE CORROSION IN A MARINE ENVIRONMENT.
- 10. PROVIDE FENDERING ALL AROUND THE PERIMETER OF THE FLOATING DOCK TO PROTECT BOTH THE BOATS AND DOCKS.



DRAWING TITLE

FLOATING DOCK DETAILS

DRAWING NO. MR-10 DATE 2/4/2022 REVISION NO.

> SHEET NO. 12 of 13

