PROJECT NAME: 59 TWEED BLVD

PROJECT ARCHITECT: IKE KLIGERMAN BARKLEY
STRUCTURAL ENGINEER: DOMINICK R.PILLA ASSOCIATES PC

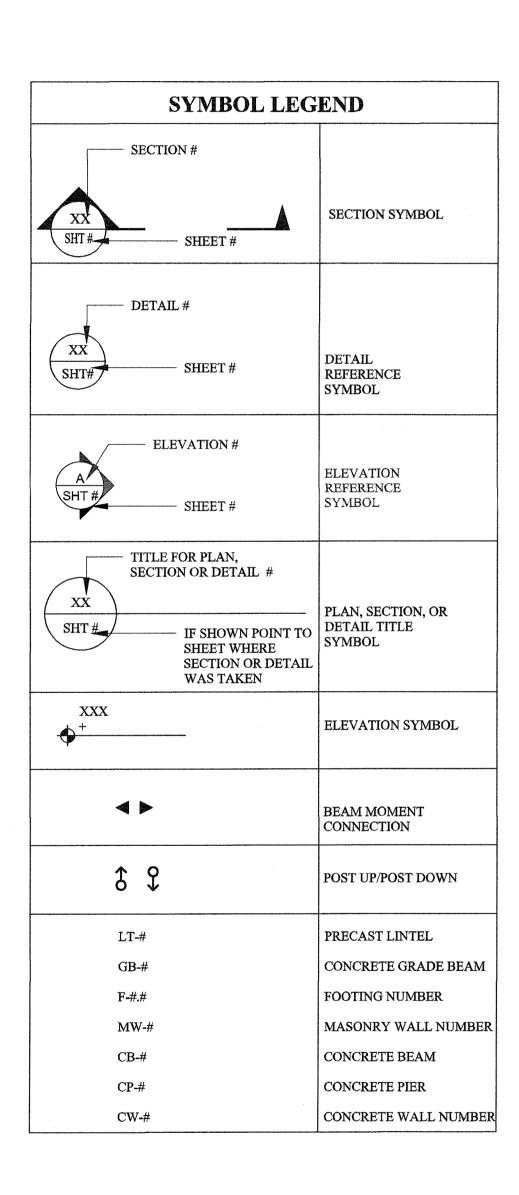
PROJECT ADDRESS: 59 TWEED BLVD, NYACK, NY 10960

ISSUED: FOR PERMIT

STRUCTURAL PLANS

DATE: 08/17/2021

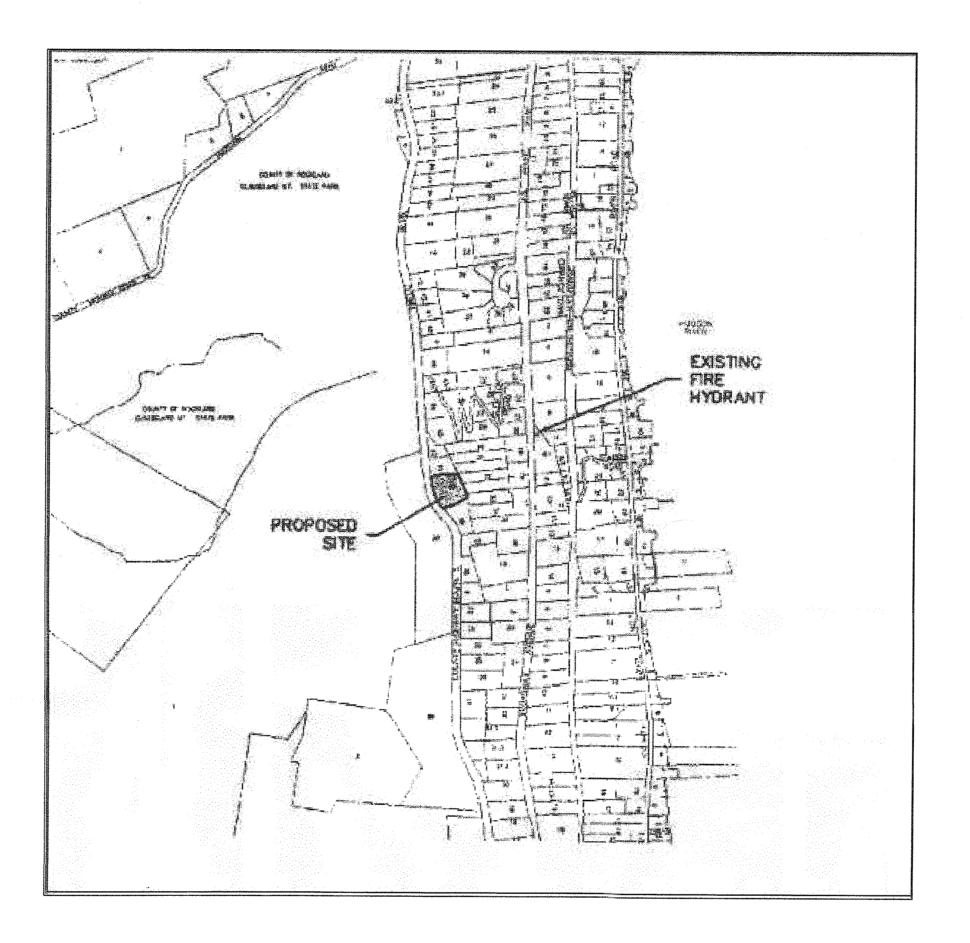
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ABBREVIATION

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WP WWF	WORKPOINT WELDED WIRE FABRIC
VIF	VERIFY IN FIELD
V	VERTICAL BEAM END REACTION IN KIPS
M	MOMENT
TYP UNO,UON	TYPICAL UNLESS OTHERWISE NOTED
TOS	TOP OF STEEL
TOF	TOP OF FOOTING
TOC	TOP OF CONCRETE
TBC	TO BE CONFIRMED
T	TOP REINFORCEMENT
T T	TENSION FORCE IN KIPS THICKNESS
SW	SHEAR WALL
SPW	SOLDIER PILE LAGGING WALL
SIM	SIMILAR
SC SC	SLIP CRITICAL BOLT
SAD S1,S2	SEE ARCHITECTURAL DRAWINGS/DETAILS SLAB ON DECK TYPE
PL SAD	PROPERTY LINE
PP .	PARTIAL PENETRATION WELD
PL	PLATE
PC	PILE CAP
OC	ON CENTER
N NTS	BEARING BOLTS THREADS INCLUDED IN SHEAR PLANE NOT TO SCALE
(N)	NEW CONSTRUCTION
MIN	MINIMUM
MC	MOMENT CONNECTION SHOWN ON DRAWING
M	BENDING MOMENT IN FOOT-KIPS
LW	LÌGHTWEIGHT CONCRETE
LLBB	LONG LEGS BACK-TO-BACK
LD LDC	TENSION DEVELOPMENT LENGTH FOR REINFORCING BAR: COMPRESSION SPLICE LENGTH FOR REINFORCEMENT BAR
LAP	FULL TENSION CAPACITY LAP SPLICE
J1,J2	NEW CODE FORMED STEEL JOISTS
H	HORIZONTAL FORCE IN KIPS
H	HORIZONTAL REINFORCEMENT
GB	GRADE BEAM
F	FINISHED SURFACE
EL EW	ELEVATION EACH WAY
EF	EACH FACE
(E)	EXISTING CONSTRUCTION
DEL	DELTA OR CHANGE IN ELEVATION
DB	DEVELOPMENT LENGTH OF REINFORCEMENT BAR
CP EM	COMPLETE PENETRATION WELD
COMP LAP	COMPRESSION REINF LAP SLICE
COL CONT	COLUMN CONTINUOUS
CG	CENTER OF GRAVITY
CL	CENTER LINE
CANT.	CANTILEVER
C	COMPRESSION FORCE IN KIPS
BW	BEARING WALL
BS BU	BOTH SIDES BUILT UP MEMBER
BM	BEAM
	BOTTOM REINFORCEMENT

LEGEND						
8887 1887 1889 1889	NEW CONCRETE RETAINING WALL					
	NEW FOOTING					
PROPERTY LINE						



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	SPECIAL INSPECTIONS	
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	STRUCTURAL STEEL - DETAILS	BC 1704.3.2
	STRUCTURAL STEEL - HIGH STRENGTH BOLTS	BC 1704.3.3
	STRUCTURAL COLD-FORMED STEEL	BC 1704.3.4
•	CONCRETE - CAST-IN-PLACE	BC 1704.4
	CONCRETE - PRECAST	BC 1704.4
	CONCRETE - PRESTRESSED	BC 1704.4
	MASONRY	BC 1704.5
	WOOD - INSTALLATION OF HIGH-LOAD DIAPHRAGMS	BC 1704.6.1
	WOOD-INSTALLATION OF METAL-PLATE-CONNECTED TRUSSES	BC 1704.6.2
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•	REINFORCING STEEL			
	STEEL DECKING AND STUD LAYOUT			
STRUCTURAL STEEL				
	STEEL STAIRS			
	CONCRETE STAIRS			
	STRUCTURAL COLD FORM STEEL			
	PRECAST CONCRETE			
•	CONCRETE DESIGN MIX			
	ANCHOR ROD LAYOUT			
COMPOSITE DRAWINGS OF ALL SLAB PENETRATIONS				
	CONCRETE MASONRY			
	CURTAIN WALLS			

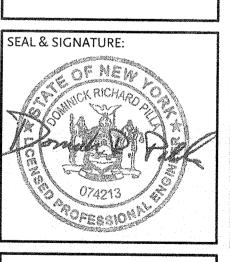
Ike Kligerman Barkley

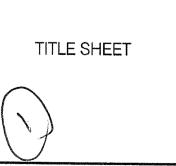
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No.	REMARKS	DATE
1	REVISION 1	02/02/202
2		
3		
-		

PROJECT:

RESIDENCE
59 TWEED BOULEVARD
ORANGEBURG, NY 10960





DATE:	08/17/2021
PROJECT No.:	21-077
DRAWING BY:	AM/JB
CHK BY:	GF
DWG No.:	

S-001.00 |

GENERAL NOTES

- UNLESS OTHERWISE NOTED OR SHOWN ON THE STRUCTURAL DRAWINGS, THE FOLLOWING REQUIREMENTS, TOGETHER WITH THE PROJECT PLANS, SPECIFICATIONS AND GEOTECHNICAL REPORT APPLY TO THE STRUCTURES IN THIS CONTRACT.
- CONSTRUCTION IS TO COMPLY WITH THE REQUIREMENTS OF THE GOVERNING BUILDING CODE AND ALL OTHER APPLICABLE FEDERAL, STATE, AND LOCAL
- 2. THE STRUCTURAL DOCUMENTS SHALL BE USED IN CONJUNCTION WITH AND COORDINATED WITH THE ARCHITECTURAL, CIVIL AND MEP CONTRACT DOCUMENTS AS WELL AS ANY OTHER TRADES. IF A CONFLICT EXISTS, CONTRACTOR SHALL NOTIFY THE ARCHITECT/ENGINEER AND OBTAIN CLARIFICATION PRIOR TO BIDDING AND PROCEEDING WITH WORK

CODES, STANDARDS, REGULATIONS AND LAWS.

- 3. THE GENERAL CONTRACTOR SHALL COORDINATE ALL CONTRACT DOCUMENTS WITH FIELD CONDITIONS, DIMENSIONS, ELEVATIONS AND PROJECT SHOP DRAWINGS PRIOR TO CONSTRUCTION. DO NOT SCALE DRAWINGS; USE ONLY PRINTED DIMENSIONS. REPORT ANY DISCREPANCIES IN WRITING TO THE ARCHITECT/ENGINEER PRIOR TO PROCEEDING WITH WORK. DO NOT CHANGE SIZE OR LOCATION OF STRUCTURAL MEMBERS WITHOUT WRITTEN INSTRUCTIONS FROM THE STRUCTURAL ENGINEER OF RECORD.
- 4. THE DESIGN AT THE EXISTING PART OF THE BUILDING, WHICH WILL REMAIN, IS BASED ON INCOMPLETE INFORMATION ABOUT THE EXISTING STRUCTURE, THE SIZE AND DEPTH OF EXISTING FOUNDATION. AS THE WORK PROGRESS, THE CONTRACTOR SHALL PROVIDE THE ENGINEER WITH FIELD INFORMATION ABOUT THE EXISTING FOUNDATION AND OTHER STRUCTURAL MEMBERS AND FOLLOW ANY CHANGES IN DESIGN THAT WILL BE REQUIRED BY THE ENGINEER DUE TO UNANTICIPATED FIELD CONDITIONS.
- 5. OPENINGS SHOWN ON STRUCTURAL DRAWINGS ARE ONLY PICTORIAL. SEE THE ARCHITECTURAL AND M.E.P. DRAWINGS FOR THE SIZE AND LOCATION OF OPENINGS IN THE STRUCTURE.
- 6. CONTRACTORS WHO DISCOVER DISCREPANCIES, OMISSIONS OR VARIATIONS IN THE CONTRACT DOCUMENTS DURING BIDDING SHALL IMMEDIATELY NOTIFY THE ARCHITECT. THE ARCHITECT WILL RESOLVE THE CONDITION AND ISSUE A WRITTEN CLARIFICATION.
- 7. THE CONTRACTOR SHALL PROTECT ADJACENT PROPERTY, HIS OWN WORK AND THE PUBLIC FROM HARM. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR CONSTRUCTION MEANS AND METHODS, AND JOBSITE SAFETY INCLUDING ALL OSHA REQUIREMENTS.
- 8. SEE PROJECT SPECIFICATIONS FOR TESTING. SEE THE STRUCTURAL SPECIAL INSPECTION NOTES FOR INSPECTION REQUIREMENTS.
- 9. DETAILS LABELED "TYPICAL" APPLY TO ALL SITUATIONS THAT ARE THE SAME OR SIMILAR TO THOSE SPECIFICALLY REFERENCED, WHETHER OR NOT THEY ARE KEYED IN AT EACH LOCATION. QUESTIONS REGARDING THE APPLICABILITY OF TYPICAL DETAILS SHALL BE RESOLVED BY THE ARCHITECT.
- 10. THE STRUCTURE IS DESIGNED TO BE STRUCTURALLY SOUND WHEN COMPLETED, PRIOR TO COMPLETION, THE CONTRACTOR IS RESPONSIBLE FOR STABILITY AND TEMPORARY BRACING, INCLUDING, BUT NOT LIMITED TO, MASONRY WALLS. THE CONTRACTOR SHALL VERIFY THAT CONSTRUCTION LOADS DO NOT EXCEED THE CAPACITY OF THE STRUCTURE AT THE TIME THE LOAD IS APPLIED. WHENEVER THE CONTRACTOR IS UNSURE OF THESE REQUIREMENTS, THE CONTRACTOR SHALL RETAIN A NEW YORK STATE LICENSED ENGINEER TO DESIGN AND INSPECT THE TEMPORARY BRACING AND STABILITY OF THE STRUCTURE.

CODES AND SPECIFICATIONS

- THE DESIGN SHOWN ON THESE DRAWINGS IS BASED ON THE FOLLOWING CODES, SPECIFICATIONS AND STANDARDS:
- "NEW YORK CITY BUILDING CODE," 2014 EDITION.
 ASCE 7-16: "MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES," AWS D1.1: "STRUCTURAL WELDING CODE," 2014.
 "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE," ACI 318-2014.
 "SPECIFICATIONS FOR STRUCTURAL CONCRETE," ACI 301-1999.
 "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES," ACI 530-2013.
 "SPECIFICATIONS FOR STRUCTURAL STEEL BUILDINGS," AISC 360-16,
 "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES." AISC
- "SEISMIC PROVISIONS FOR STRUCTURAL STEEL BUILDINGS," AISC-341-16, "STANDARD FOR COMPOSITE STEEL FLOOR DECK-SLAB," SDI, 2011. "STANDARD FOR STEEL ROOF DECK," SDI 2010.
- "NATIONAL DESIGN SPECIFICATION (NDS) FOR WOOD CONSTRUCTION," AWC NDS 2018.
 ASCE 37-14: "DESIGN LOADS ON STRUCTURES DURING CONSTRUCTION."
- 3. "BUILDING CODE OF NEW YORK STATE," 2020.

SHOP DRAWINGS AND OTHER SUBMITTALS

- 1. SEE TITLE SHEET FOR REQUIRED SUBMITTALS.
- 2. INCOMPLETE SUBMITTALS WILL BE RETURNED WITHOUT REVIEW.
- 3. SUBMIT SPECIFIC COMPONENTS, SUCH AS COLUMNS, FOOTINGS, ETC., IN A SINGLE PACKAGE. SUBMIT SIMILAR FLOORS TOGETHER.
- 4. ON FIRST SUBMITTAL, CLEARLY FLAG AND CLOUD ALL DIFFERENCES FROM THE CONTRACT DOCUMENTS. ON RE-SUBMITTALS, FLAG AND CLOUD ALL CHANGES AND ADDITIONS TO PREVIOUS SUBMITTAL. ONLY CLOUDED ITEMS WILL BE REVIEWED.
- 5. SUBMITTALS FOR SPECIAL STRUCTURAL, LOAD-CARRYING ITEMS THAT ARE REQUIRED BY CODES OR STANDARDS TO RESIST FORCES MUST BE PREPARED BY, OR UNDER THE DIRECT SUPERVISION OF, A DELEGATED ENGINEER. EXAMPLES INCLUDE STRUCTURAL STEEL CONNECTIONS, STRUCTURAL LIGHT GAGE STEEL FRAMING, AND EXTERIOR ENCLOSURE SYSTEMS.
- 6. A DELEGATED ENGINEER IS DEFINED AS A NEW YORK STATE LICENSED ENGINEER WHO SPECIALIZES IN AND UNDERTAKES THE DESIGN OF STRUCTURAL COMPONENTS OR STRUCTURAL SYSTEMS INCLUDED IN A SPECIFIC SUBMITTAL PREPARED FOR THIS PROJECT AND IS AN EMPLOYEE OR OFFICER OF, OR CONSULTANT TO, THE CONTRACTOR OR FABRICATOR RESPONSIBLE FOR THE SUBMITTAL. THE DELEGATED ENGINEER SHALL SIGN, SEAL AND DATE THE SUBMITTAL, INCLUDING CALCULATIONS AND DRAWINGS. SEE SPECIFICATIONS FOR MORE SPECIFIC CRITERIA.
- 7. THE TRADE CONTRACTOR IS RESPONSIBLE FOR CONFIRMING AND CORRELATING DIMENSIONS AT THE JOB SITES, FOR TOLERANCES, CLEARANCES, QUANTITIES, FABRICATION PROCESSES AND TECHNIQUES OF CONSTRUCTION, COORDINATION OF THE WORK WITH OTHER TRADES AND FULL COMPLIANCE WITH THE CONTRACT DOCUMENTS.
- 8. THE GENERAL CONTRACTOR/CONSTRUCTION MANAGER SHALL REVIEW AND APPROVE SUBMITTALS AND SHALL SIGN AND DATE EACH DRAWING PRIOR TO SUBMITTING TO THE ARCHITECT. THIS APPROVAL IS TO CONFIRM THAT THE SUBMITTAL IS COMPLETE, COMPLIES WITH THE SUBMITTAL REQUIREMENTS AND IS COORDINATED WITH FIELD DIMENSIONS, OTHER TRADES, ERECTION SEQUENCING AND CONSTRUCTABILITY.

- 9. THE STRUCTURAL ENGINEER REVIEWS SUBMITTALS TO CONFIRM THAT THE SUBMITTAL IS IN GENERAL CONFORMANCE WITH THE DESIGN CONCEPT PRESENTED IN THE CONTRACT DOCUMENTS. QUANTITIES AND DIMENSIONS ARE NOT CHECKED. NOTATIONS ON SUBMITTALS DO NOT AUTHORIZE CHANGES TO THE CONTRACT SUM. CHECKING OF THE SUBMITTAL BY THE STRUCTURAL ENGINEER SHALL NOT RELIEVE THE CONTRACTOR OF RESPONSIBILITY FOR DEVIATIONS FROM THE CONTRACT DOCUMENTS AND FROM ERRORS OR OMISSIONS IN THE SUBMITTAL.
- 10. IN ADDITION TO THE ABOVE, THE STRUCTURAL ENGINEER'S REVIEW OF DELEGATED ENGINEER SUBMITTALS IS LIMITED TO VERIFYING THAT THE SPECIFIED STRUCTURAL SUBMITTAL HAS BEEN FURNISHED, SIGNED AND SEALED BY THE DELEGATED ENGINEER AND THAT THE DELEGATED ENGINEER HAS UNDERSTOOD THE DESIGN INTENT AND USED THE SPECIFIED STRUCTURAL CRITERIA. NO DETAILED CHECK OF CALCULATIONS WILL BE MADE. THE DELEGATED ENGINEER IS SOLELY RESPONSIBLE FOR HIS/HER DESIGN, INCLUDING BUT NOT LIMITED TO THE ACCURACY OF HIS/HER CALCULATIONS AND COMPLIANCE WITH THE APPLICABLE CODES AND STANDARDS.
- 11. CAD FILES OF STRUCTURAL DRAWINGS MAY BE USED AS AN AID IN PREPARING SHOP DRAWINGS ONLY UPON THE CONTRACTOR SIGNING AN AGREEMENT. WHEN CAD FILES OR COPIES OF THE STRUCTURAL DRAWINGS ARE MADE AVAILABLE, IT IS UNDER THE FOLLOWING CONDITIONS:
- a. ALL INFORMATION CONTAINED IN THE CAD FILES OR COPIES OF THE STRUCTURAL DRAWINGS ARE INSTRUMENTS OF SERVICE OF THE ARCHITECT/ENGINEER AND SHALL NOT BE USED FOR OTHER PROJECTS, ADDITIONS TO THE PROJECT OR THE COMPLETION OF THE PROJECT BY OTHERS. CAD FILES AND COPIES OF THE STRUCTURAL DRAWINGS REMAIN THE PROPERTY OF DOMINICK R. PILLA ASSOCIATES AND IN NO CASE SHALL THEIR TRANSFER BE CONSIDERED A SALE.
- b. CAD FILES OR COPIES OF THE STRUCTURAL DRAWINGS ARE NOT CONTRACT DOCUMENTS. IN THE EVENT OF A CONFLICT, THE STRUCTURAL DRAWINGS SHALL GOVERN.
- c. THE USE OF CAD FILES OR COPIES OF THE STRUCTURAL DRAWINGS SHALL NOT IN ANY WAY RELIEVE THE CONTRACTOR'S RESPONSIBILITY FOR PROPER CHECKING AND COORDINATION OF DIMENSIONS, DETAILS, SIZES AND QUANTITIES OF MATERIALS AS REQUIRED FOR THE PREPARATION OF COMPLETE AND ACCURATE SHOP DRAWINGS.
- d. THE CONTRACTOR SHALL REVISE ALL REFERENCES TO CONTRACT DOCUMENT SHEET NUMBERS AND SECTION MARKS AND SHALL REMOVE INFORMATION THAT IS NOT REQUIRED FOR THEIR WORK FROM THE CAD FILES OR COPIES OF THE STRUCTURAL DRAWINGS, INCLUDING THE TITLE BLOCK; AND
- e. DIMENSIONS IN THE CAD FILES MAY NOT BE PRECISE AND, IN SOME CASES, HAVE BEEN INTENTIONALLY ALTERED FOR PRESENTATION PURPOSES. DO NOT SCALE DIMENSIONS ELECTRONICALLY OR OTHERWISE.

SHALLOW FOUNDATIONS

- 1. FOUNDATION DESIGN IS BASED ON THE "GEOTECHNICAL ENGINEERING REPORT," PREPARED BY ROBERT DYKSTRA OF GTA ENGINEERING SERVICES OF NEW YORK, P.C. AND DATED 7/16/2021 SEE THAT REPORT FOR ADDITIONAL REQUIREMENTS.
- 2. FOUNDATIONS PLACED ON UNDISTURBED SOIL AT ELEVATIONS INDICATED ARE DESIGNED FOR AN ALLOWABLE NET SOIL BEARING PRESSURE OF 2,000 PSF.

AND

FOUNDATIONS PLACED ON COMPACTED STRUCTURAL FILL HAVE BEEN DESIGNED FOR AN ALLOWABLE NET SOIL BEARING PRESSURE OF 2,000 PSF.

- 3. THE CONTRACTOR SHALL NOTIFY THE ENGINEER WHERE BOTTOM OF FOOTING ELEVATION IS CHANGED AND OBTAIN REVISED DESIGN OF THE FOUNDATION AND RETAINING WALLS AS REQUIRED.
- 4. ALL FILL REQUIRED BELOW ANY PORTION OF THE STRUCTURE SHALL BE COMPACTED IN 9" LIFTS TO AT LEAST 98% OF THE MAXIMUM DRY DENSITY PER ASTM D-1557. REMOVE UNSUITABLE FILL AND REPLACE WITH CONTROLLED FILL AS REQUIRED FOR SOUND PLACEMENT OF FOUNDATIONS.
- 5. SOIL SUPPORTED FOOTING SHALL BE FOUNDED UPON UNDISTURBED NATURAL SUBGRADE (OR CONTROLLED COMPACTED FILL) WITH A MINIMUM BEARING CAPACITY AS NOTED AND AS FIELD VERIFIED AND APPROVED BY A REGISTERED SOIL ENGINEER. THE BOTTOM OF THE FOOTING ELEVATIONS AND BEARING CAPACITIES AS SHOWN ON THE DRAWINGS ARE ESTIMATED AND WILL REQUIRE VERIFICATION. FINAL, EXACT ELEVATIONS AND BEARING CAPACITIES SHALL BE FIELD DETERMINED.
- 6. WHEN NECESSARY, FOOTINGS STEPS SHALL BE CONSTRUCTED AT MAXIMUM
- SLOPE OF ONE VERTICAL TO TWO HORIZONTALS.

 7. CENTER ALL FOOTINGS UNDER THEIR RESPECTIVE COLUMNS OR WALLS, U.O.N.

EXCAVATION, BACKFILL AND DEWATERING

- 1. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR ALL EXCAVATION PROCEDURES INCLUDING LAGGING, SHORING, AND PROTECTION OF ADJACENT PROPERTY, STRUCTURES, STREETS AND UTILITIES IN ACCORDANCE WITH THE REQUIREMENTS OF THE LOCAL BUILDING DEPARTMENT AND OSHA REGULATIONS. DO NOT EXCAVATE WITHIN ONE FOOT OF THE ANGLE OF REPOSE OF ANY SOIL BEARING FOUNDATION UNLESS THE FOUNDATION IS PROPERLY PROTECTED AGAINST SETTLEMENT.
- 2. DO NOT BACKFILL AGAINST WALLS UNTIL 7 DAYS AFTER THE WALLS ARE BRACED BY THE STRUCTURE OR ARE TEMPORARILY BRACED. DO NOT BACKFILL CANTILEVERED RETAINING WALLS UNTIL CONCRETE IS 7 DAYS OLD. DO NOT BACKFILL UNTIL AFTER COMPLETION AND INSPECTION OF ANY WATERPROOFING.
- 3. THE CONTRACTOR IS RESPONSIBLE FOR THE DISPOSAL OF ALL ACCUMULATED WATER IN A MANNER THAT DOES NOT INCONVENIENCE OR DAMAGE THE WORK.

SLABS ON GRADE

- 1. SLABS-ON-GRADE SHALL BE 5" THICK FIBER REINFORCED CONCRETE SLAB. (PROVIDE 3 LBS PER CU. FT. OF MACRO SYNTHETIC FIBER.) SHALL BE FINISHED IN ACCORDANCE WITH ACI STANDARD 302.1R FOR CLASS 2 FLOORS. TYPE II OR I/II CEMENT AND 3/4" COARSE AGGREGATE (SIZE NO. 57) SHALL BE USED.
- 2 SUBGRADE SOILS BELOW SLAB SHALL BE PROOF ROLLED AND CERTIFIED BY A SOILS ENGINEER AS ACCEPTABLE BEFORE PLACEMENT OF GRAVEL OR CONCRETE.
- 3. PROVIDE CONTROL JOINTS SPACING OF 36 TIMES THE SLAB THICKNESS MAXIMUM.
- 4. FOLLOW RECOMMENDATIONS OF ACI 302.1R.
- 5. SIDEWALKS AND WALKWAYS, LOCATE ISOLATION JOINTS AT 20 FT. O.C. MAXIMUM SCORE AND TOOL BETWEEN ISOLATION JOINTS IN EQUAL BAYS OF 5 FT. OR LESS.
- 6. SEE THE ARCHITECTURAL DRAWINGS FOR SLAB ON GRADE DEPRESSIONS AND OTHER REQUIREMENTS.

REINFORCED CONCRETE

- 1. COMPLY WITH ACI 301 AND 318.
- 2. ALL CAST-IN-PLACE CONCRETE SHALL BE CONTROLLED CONCRETE AND SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH (Fc) AT 28 DAYS AS FOLLOWS:

FOOTINGS 4,000 PSI
POURED WALLS 4,000 PSI
SLABS-ON-GRADE 4,000 PSI

- SLAB ON METAL DECK 4,000 PSI (LT. WT. CONC.)
- 4. CONCRETE REINFORCEMENT SHALL BE ASTM A615, GRADE 60 DEFORMED REINFORCING STEEL. LAP BOTTOM STEEL OVER SUPPORTS AND TOP STEEL AT MIDSPAN (U.O.N.). HOOK DISCONTINUOUS ENDS OF ALL TOP BARS AND ALL BARS IN WALLS, U.O.N.

3. USE NORMAL WEIGHT CONCRETE FOR ALL STRUCTURAL MEMBERS. U.O.N.

5. USE EPOXY COATED REINFORCEMENT CONFORMING TO ASTM A775 FOR CONCRETE SUBJECT TO WATER AND CHLORITE PENETRATION.

A.LOADING DOCK SLABS AND WALLS.

- 6. WHERE SPECIFIED, PROVIDE PLAIN, COLD-DRAWN ELECTRONICALLY WELDED WIRE REINFORCEMENT (WWF) CONFORMING TO ASTM A185. SUPPLY IN FLAT SHEETS ONLY. LAP SPLICE SHALL BE ONE CROSS WIRE SPACING PLUS TWO
- 7. FOLLOW ACI 117-10 "SPECIFICATION FOR TOLERANCES OF CONCRETE CONSTRUCTION AND MATERIALS" FOR REQUIRED TOLERANCES.
- 8. UTILITIES SHALL NOT PENETRATE BEAMS OR COLUMNS BUT MAY PASS THROUGH SLABS AND WALLS INDIVIDUALLY, UON. SEE TYPICAL DETAILS.
- 9. PROVIDE CONSTRUCTION JOINTS IN ACCORDANCE WITH ACI 318, SECTION 6.4. PROVIDE KEYWAYS AND ADEQUATE DOWELS. SUBMIT DRAWINGS SHOWING LOCATION OF CONSTRUCTION JOINTS AND DIRECTION OF POUR FOR REVIEW.
- 10. PROVIDE 3/4" CHAMFER FOR ALL EXPOSED CORNERS. SEE ARCHITECTURAL DRAWINGS FOR ADDITIONAL CONCRETE FINISH REQUIREMENTS.

CONCRETE FIELD TESTING:

- 1. TESTING: OWNER WILL ENGAGE A QUALIFIED TESTING AGENCY TO PERFORM FIELD TESTS AND PREPARE TEST REPORTS.
- 2. CONCRETE TESTS: TESTING OF COMPOSITE SAMPLES OF FRESH CONCRETE OBTAINED ACCORDING TO ASTM C172 AND SECTION BC 1905.6.5 OF NYC BUILDING CODE SHALL BE PERFORMED ACCORDING TO THE FOLLOWING REQUIREMENTS:
- a. TESTING FREQUENCY: OBTAIN ONE COMPOSITE SAMPLE FOR EACH DAY'S POUR OF EACH CONCRETE MIXTURE LESS THAN 25 CU. YD., PLUS ONE SET FOR EACH ADDITIONAL 50 CU. YD. OR FRACTION THEREOF.
- b. WHEN FREQUENCY OF TESTING WILL PROVIDE FEWER THAN FIVE COMPRESSIVE STRENGTH TESTS OF EACH CONCRETE MIXTURE, TESTING SHALL BE CONDUCTED FROM AT LEAST FIVE RANDOMLY SELECTED BATCHES OR FROM EACH BATCH IF FEWER THAN FIVE ARE USED.
- c. WATER CONTENT AND SLUMP: VERIFY WATER CONTENT IN ACCORDANCE WITH AASHTO t-318 "STANDARD METHOD OF TESTS FOR WATER CONTENT USING MICROWAVE OVEN DRYING." TEST SLUMP IN ACCORDANCE WITH ASTM C143; ONE TEST AT POINT OF PLACEMENT FOR EACH COMPOSITE SAMPLE, BUT NOT LESS THAN ONE TEST FOR EACH DAY'S POUR OF EACH CONCRETE MIXTURE. PERFORM ADDITIONAL TESTS WHEN CONCRETE CONSISTENCY APPEARS TO CHANGE.
- d. AIR CONTENT: ASTM C231, PRESSURE METHOD, FOR NORMAL-WEIGHT CONCRETE; ASTM C173, VOLUMETRIC METHOD, FOR LIGHTWEIGHT CONCRETE; ONE TEST FOR EACH COMPOSITE SAMPLE, BUT NOT LESS THAN ONE TEST FOR EACH DAY'S POUR OF EACH CONCRETE MIXTURE.
- e. CONCRETE TEMPERATURE: ASTM C1064; ONE TEST HOURLY WHEN AIR TEMPERATURE IS 40 DEG F AND BELOW AND WHEN 80 DEG F AND ABOVE, AND ONE TEST FOR EACH COMPOSITE SAMPLE.
- f. COMPRESSION TEST SPECIMENS: ASTM C31.i. CAST AND LABORATORY CURE ALL TEST CYLINDER SPECIMENS.
- ii. WHEN REQUIRED, CAST AND FIELD CURE TWO SETS OF TWO STANDARD CYLINDER SPECIMENS FOR EACH COMPOSITE SAMPLE.
- g. COMPRESSIVE-STRENGTH TESTS: ASTM C39 AND SECTION BC 1905.6.2 OF THE NYC BUILDING CODE; TEST FIRST SET OF TWO LABORATORY-CURED SPECIMENS AT 7 DAYS FOR INFORMATION, SECOND SET OF TWO LABORATORY-CURED SPECIMENS AT 28 DAYS FOR ACCEPTANCE AND THIRD SET OF TWO SPECIMENS AT 56 DAYS IF NECESSARY.
- i. TEST ONE SET OF FIELD-CURED SPECIMENS AT 7 DAYS AND ONE SET OF TWO SPECIMENS AT 28 DAYS.
- ii. A COMPRESSIVE-STRENGTH TEST SHALL BE THE AVERAGE COMPRESSIVE STRENGTH FROM A SET OF TWO SPECIMENS OBTAINED FROM SAME COMPOSITE SAMPLE AND TESTED AT AGE INDICATED.

STRUCTURAL STEEL

- 1. FABRICATION AND ERECTION OF STRUCTURAL STEEL SHALL CONFORM TO AISC "SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS", WITH COMMENTARY, AND ALL OSHA REQUIREMENTS.
- 2. STRUCTURAL STEEL MATERIALS SHALL CONFORM TO THE FOLLOWING MINIMUM REQUIREMENTS, UNLESS OTHERWISE NOTED ON THE CONTRACT DOCUMENTS:
- ROLLED W SHAPES: ASTM A992, GRADE 50.
 ROLLED M, S, C, MC, AND L SHAPES: ASTM A36, FY=36 KSI.
 PLATES AND BARS: ASTM A36, FY=36 KSI, UON.
- PLATES FOR MOMENT CONNECTIONS: ASTM A572, GR. 50. STEEL PIPE: ASTM A53, TYPE E OR S, GRADE B, FY=35 KSI.

HOLLOW STRUCTURAL SECTIONS:

- ROUND SECTIONS: ASTM A500, GRADE C, FY=46 KSI SQUARE AND RECTANGULAR SECTIONS: ASTM A500, GRADE C, FY=50 KSI.
- 3. ALL STRUCTURAL STEEL CONNECTIONS BOLTS SHALL BE ASTM A325 OR ASTM A490, TYPE 1, UNLESS OTHERWISE NOTED, AND SHALL COMPLY WITH "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS," INCLUDING COMMENTARY. ALL JOINT TYPE SHALL BE 'PT' (PRETENSIONED).
- 4. BOLT SIZE SHALL BE 3/4" DIAMETER MINIMUM, UNLESS OTHERWISE NOTED.
- 5. A MINIMUM OF TWO (2) 3/4" DIAMETER A325 BOLTS SHALL BE PROVIDED AT EACH CONNECTIONS.

- 6. SHOP DRAWINGS SHALL BE COORDINATED WITH STAIR DETAILS. IF HANGER RODS ARE USED, PROVIDE STIFFENER PLATE, 3/8" THICK MINIMUM, ALONGSIDE HANGER LOCATION.
- 7. SHEAR AND BRACING CONNECTIONS SHALL BE DESIGNED AND DETAILED BY THE FABRICATOR IN ACCORDANCE WITH AISC FOR THE FORCES AND/OR REACTIONS SHOWN, THE FABRICATOR SHALL SUBMIT CALCULATIONS DEMONSTRATING THAT THE SELECTED SHEAR AND BRACING CONNECTIONS WILL ACHIEVE THE FORCES AND/OR REACTIONS INDICATED, OR AS REQUIRED BY THE CODES.
- 8. WHERE DETAILS ARE NOT GIVEN, CONNECTIONS FOR BEAMS AND GIRDERS ARE TO BE DESIGNED FOR THE GIVEN FORCES AND REACTIONS.
- 9. ANCHOR RODS SHALL BE ASTM F1554 GRADE 55 WITH WELDABILITY SUPPLEMENTARY REQUIREMENT S1, HOOKED OR ANCHOR RODS SHALL BE A449, TYPE 1, THREADED WITH NUTS AND WASHERS EACH END.
- 10. WHERE CAMBER IS INDICATED, FABRICATE BEAMS SO THAT ANY NATURAL CAMBER IS UPWARD AFTER ERECTION.
- 11. PROVIDE HOLES IN ALL STEEL TO PREVENT ANY ACCUMULATION OF WATER. HOLES SHALL NOT EXCEED 1" DIAMETER.
- 12. CUT, DRILL, OR PUNCH HOLES PERPENDICULAR TO METAL SURFACES. REAM HOLES THAT MUST BE ENLARGED TO ADMIT BOLTS AS PERMITTED BY ARCHITECT. DO NOT ENLARGE UNFAIR HOLES BY BURNING OR USING DRIFT PINS.
- 13. DO NOT SPLICE STRUCTURAL STEEL MEMBERS EXCEPT WHERE INDICATED ON THE DRAWINGS.
- 14. UNLESS NOTED OTHERWISE, PROVIDE A 1/4" CAP PLATE CONTINUOUSLY WELDED AT THE ENDS OF EXTERIOR EXPOSED HOLLOW STRUCTURAL SHAPES.
- 15. SEE ARCHITECTURAL AND MECHANICAL DRAWINGS FOR MISCELLANEOUS STEEL NOT SHOWN ON THE STRUCTURAL DRAWINGS.
- 16. ALL STEEL MEMBERS SHALL BE SHOP PAINTED TNEMEC 10-99 PRIMER OR APPROVED EQUAL 2.0 MILS IN THICKNESS. ALL WELDS AND BARE SPOTS SHALL RECEIVE TOUCH-UP PAINT. ALL STEEL WITH EXTERIOR EXPOSURE SHALL RECEIVE A SHOP PAINTED TNEMEC 66 OR 161 PRIMER AND A FIELD APPLIED FINISH COAT AFTER PRIMER TOUCH-UP. FINISH COAT SHALL BE EPOXY BASED WITH THICKNESS OF 2.0 MILS.
- 17. REFER TO ARCHITECTURAL DRAWINGS AND PROJECT SPECIFICATIONS FOR PAINTING AND FIREPROOFING OF STRUCTURAL STEEL. DO NOT PAINT STEEL SURFACES IN CONTACT WITH CONCRETE OR FIREPROOFING.
- 18. STEEL USING COMPLETE JOINT PENETRATION GROOVE WELDS THAT FUSE THROUGH THE THICKNESS OF THE FLANGE OR WEB SHALL HAVE A MINIMUM CHARPY V-NOTCH IMPACT TESTING VALUE AS FOLLOWS:
- a. ASTM A6, HOT-ROLLED SHAPES WITH A FLANGE THICKNESS EXCEEDING (2) INCHES AND BUILT-UP HEAVY SHAPES WITH PLATES EXCEEDING (2) INCHES IN THICKNESS: 20 FT-LB @ 70 DEG. F.
- (INCLUDING COLUMNS, WIND GIRDERS, BRACES, ETC.): 20 FT-LB @ 70 DEG. F

 c. STEEL EXPOSED TO TEMPERATURES IN SERVICE BELOW 50 DEG. F: 20 FT-LB @

b. REGARDLESS OF THICKNESS, ALL TRUSSES, LATERAL SYSTEM MEMBERS

(LAST + 70 DEG. F, 40 DEG. F MAX)

d. REGARDLESS OF THICKNESS, ALL TRUSSES, LATERAL SYSTEM MEMBERS (INCLUDING COLUMNS, WIND GIRDERS, BRACES, ETC) EXPOSED TO

TEMPERATURES IN SERVICE BELOW 50 DEG. F: 30 FT-LB @ (LOWEST

ANTICIPATED SERVICE TEMPERATURE + 70 DEG. F, 40 DEG. F MAX)

- e. WELD METAL: 20 FT-LB @ MINUS 20 DEG. F AND 40 FT-LB @ 70 DEG. F
- f. WELD METAL EXPOSED TO TEMPERATURES IN SERVICE BELOW 50 DEG. F: 20 FT-LB @ MINUS 20 DEG. F AND 40 FT-LB @ (LAST + 20 DEG. F, 40 DEG. F MAX)
- g. TESTING IS TO BE IN ACCORDANCE WITH ASTM A6, SUPPLEMENTARY REQUIREMENT S30, CHARPY V-NOTCH IMPACT TEST FOR STRUCTURAL SHAPES ALTERNATE CORE LOCATION, AT ROLLED SHAPES AND ASTM A673 FOR PLATES AT ANY PERMITTED LOCATIONS.

WELDING

- 1. ALL SHOP AND FIELD WELDING SHALL CONFORM TO THE AWS D1.1. STRUCTURAL WELDING CODE.
- 2. WELDING ELECTRODES SHALL CONFORM TO E70XX.
- 3. WHERE NECESSARY, REMOVE GALVANIZING OR PRIMER PRIOR TO WELDING.
- 4. ALL WELDERS SHALL BE LICENSED AND CERTIFIED TO AWS STANDARDS OR THOSE REQUIRED BY APPLICABLE BUILDING CODES.
- 5. ALL WELDS SHALL BE VISUALLY INSPECTED. ALL GROOVE WELDS SHALL RECEIVE RADIOGRAPHIC OR ULTRASONIC TESTING. MAGNETIC PARTICLE TEST 20 PERCENT OF ALL FILLET WELDS.
- 6. WELDING SHALL PROGRESS IN A MANNER THAT BALANCES THE STRESSES IN THE MEMBER, IN ACCORDANCE WITH AWS.

7. FOLLOW PREHEAT REQUIREMENTS FOR BASE METAL PER AWS GUIDELINES.

METAL DECK AND SHEAR STUDS

- 1. DECK UNITS SHALL BE MANUFACTURED AND INSTALLED IN ACCORDANCE WITH CURRENT "STANDARDS FOR COMPOSITE STEEL FLOOR DECKS -SLABS," AND "STANDARDS FOR STEEL ROOF DECKS" PUBLISHED BY THE STEEL DECK INSTITUTE.
- 2. ALL FLOOR DECK SECTIONS SHALL BE 3" DEEP, 18 GAGE MINIMUM, GALVANIZED COMPOSITE METAL DECK WITH A MINIMUM I = 0.993 IN4/FT, UNLESS OTHERWISE NOTED.
- 3. ALL ROOF DECK SHALL BE 1 1/2" TYPE 'B' WIDE RIB ROOF DECK, 20 GAGE MINIMUM, GALVANIZED DECK WITH MINIMUM I = 0.22 IN4/FT, UNLESS OTHERWISE NOTED.
- 4. ALL DECKS SHALL BE CONTINUOUS OVER A MINIMUM OF TWO SPANS.

- 5. SHEAR CONNECTORS SHALL BE HEADED STUDS CONFORMING TO AWS D1.1 CHAPTER 7 (SECTION 7.2.6 AND TABLE 7.1) TYPE B, ASTM A108, Fu = 65 KSI, SIZE AND SPACING AS SPECIFIED ON THE DRAWINGS.
- 6. NO ELECTRICAL OR ANY OTHER ACCESS HOLES SHALL BE ALLOWED IN THE FLOOR SLAB.
- 7. PROVIDE CELLULAR, FLUTED AND FORM UNITS, OPENINGS, SLEEVES, ETC. AS INDICATED ON THE STRUCTURAL, ARCHITECTURAL AND MECHANICAL DRAWINGS AND SPECIFICATIONS.
- 8. PROVIDE METAL DECK REINFORCEMENT AT ALL UNFRAMED OPENINGS, SLEEVES AND COLUMN CUTOUTS AS SHOWN ON PLANS OR AS INDICATED IN THE SPECIFICATIONS.
- 9. ALL METAL DECK UNITS SHALL BE FASTENED TO THE STEEL FRAME BY 5/8" DIAMETER PUDDLE WELD WITH AN AVERAGE WELD SPACING OF AT LEAST 12" ON CENTER. SIDE LAPS ARE TO BE WELDED AT A MAXIMUM SPACING OF 36" ON CENTER.
- 10. FURNISH, INSTALL AND WELD IN POSITION, CLOSURES AND OTHER MISCELLANEOUS ITEMS AS REQUIRED TO CLOSE OPENINGS BETWEEN FLOOR UNITS AND COLUMNS, BEAMS AND GIRDERS AND AREAS WHERE DECK CHANGES SPAN DIRECTION. CLOSURE SHALL NOT BE LESS THAN 16 GAGE SHEET METAL.
- 11. CONTRACTOR SHALL SUBMIT SHOP DRAWINGS SHOWING LAYOUT OF DECK PANELS INCLUDING DETAILS OF SPECIAL CONDITIONS.

SPECIAL STRUCTURAL INSPECTIONS

- 1. ALL WORKS SHALL BE SUBJECT TO SPECIAL INSPECTIONS IN ACCORDANCE WITH CHAPTER 17 OF NEW YORK STATE BUILDING CODE. SPECIAL INSPECTORS SHALL MEET THE QUALIFICATIONS OUTLINED IN THE RULES OF THE STATE OF NEW YORK SECTION 105.3.
- 2. SEE COVER SHEET FOR REQUIRED SPECIAL INSPECTIONS.

| Ike | Kligerman | Barkley

330 West Forty-Second Stre New York, NY 10036 212 268 0128

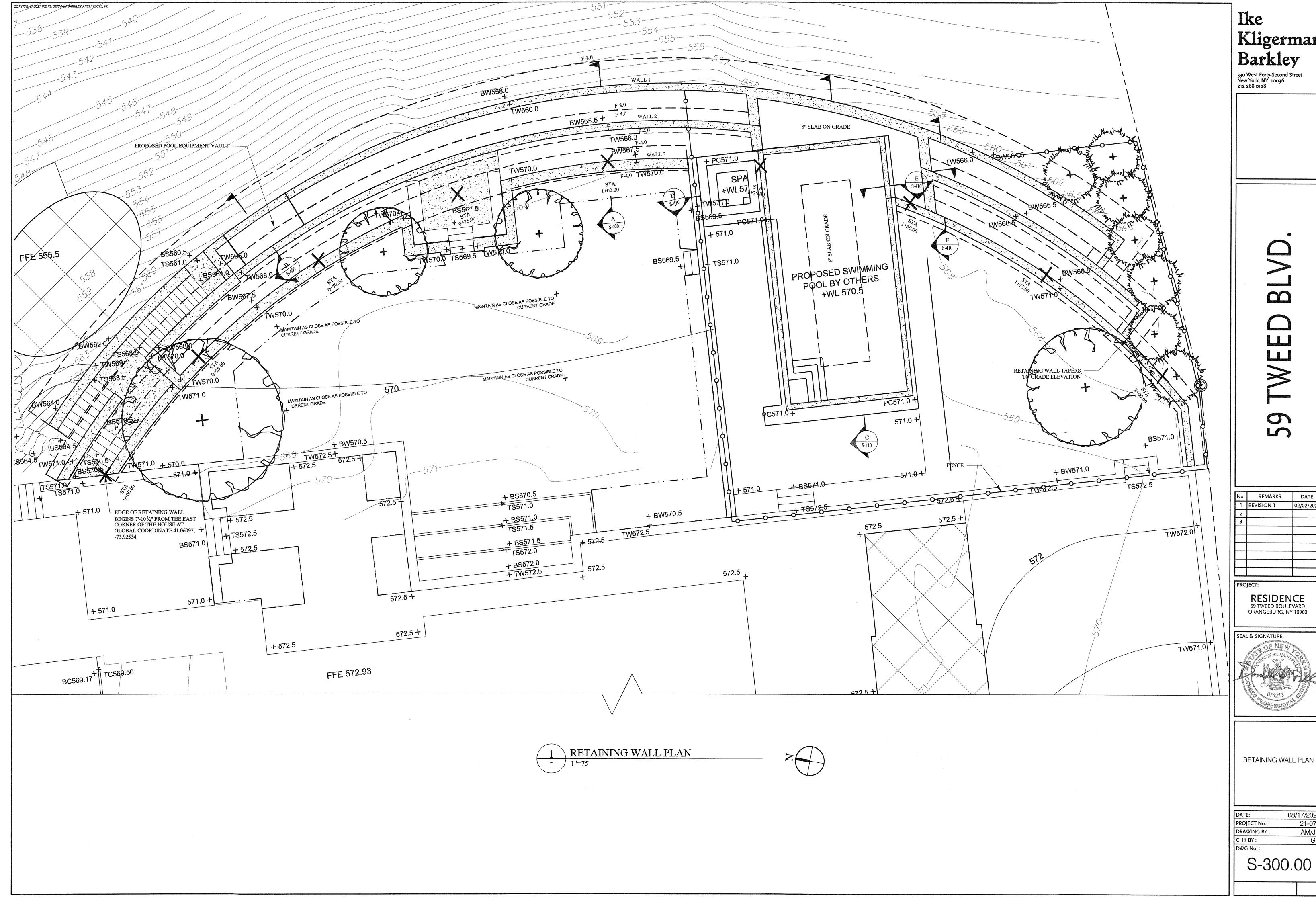
RESIDENCE 59 TWEED BOULEVARD ORANGEBURG, NY 10960

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GENERAL NOTES

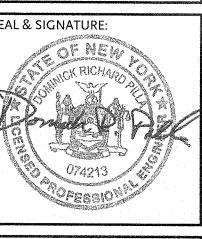
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PROJECT No.: 21-077
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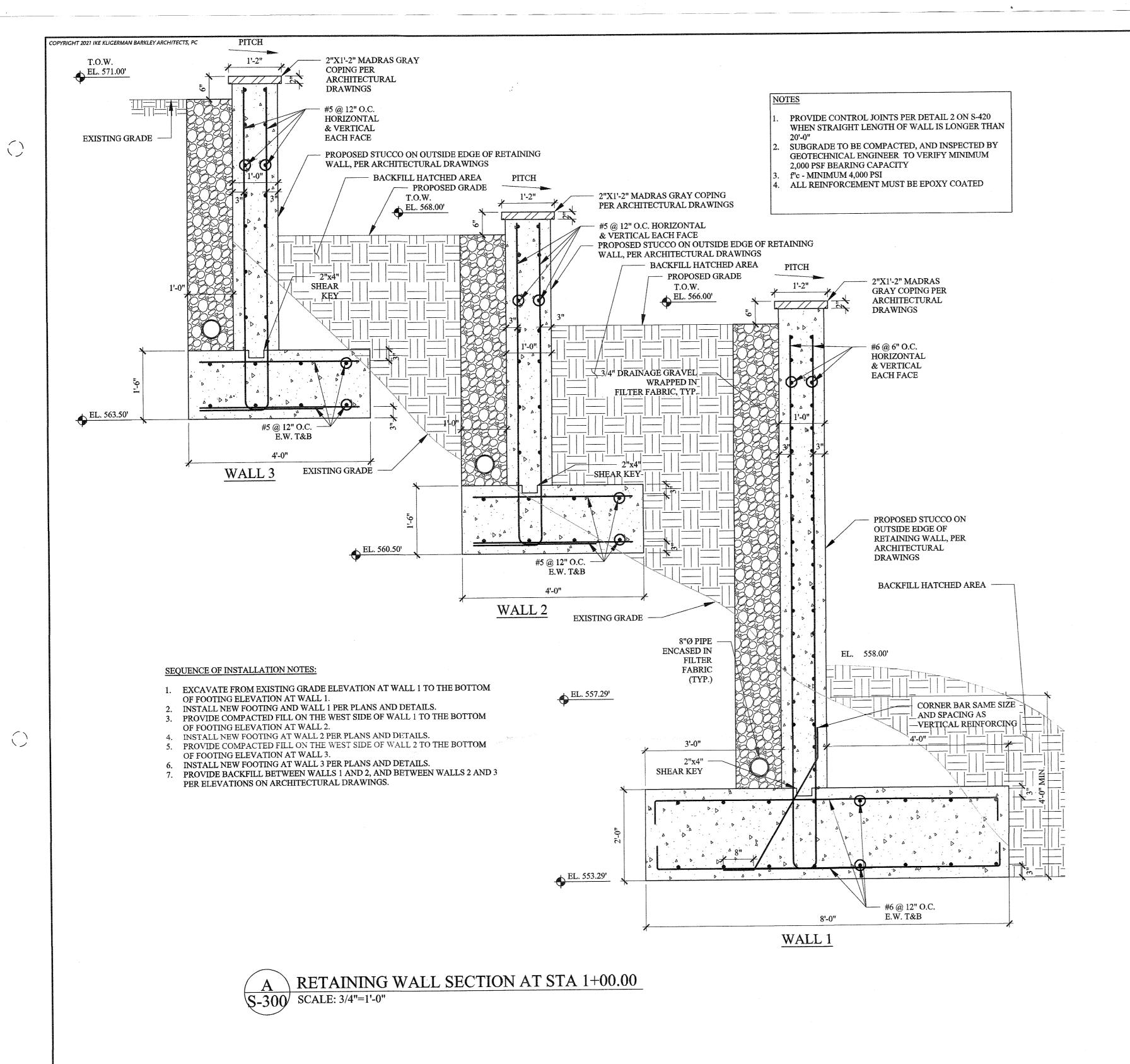


Kligerman

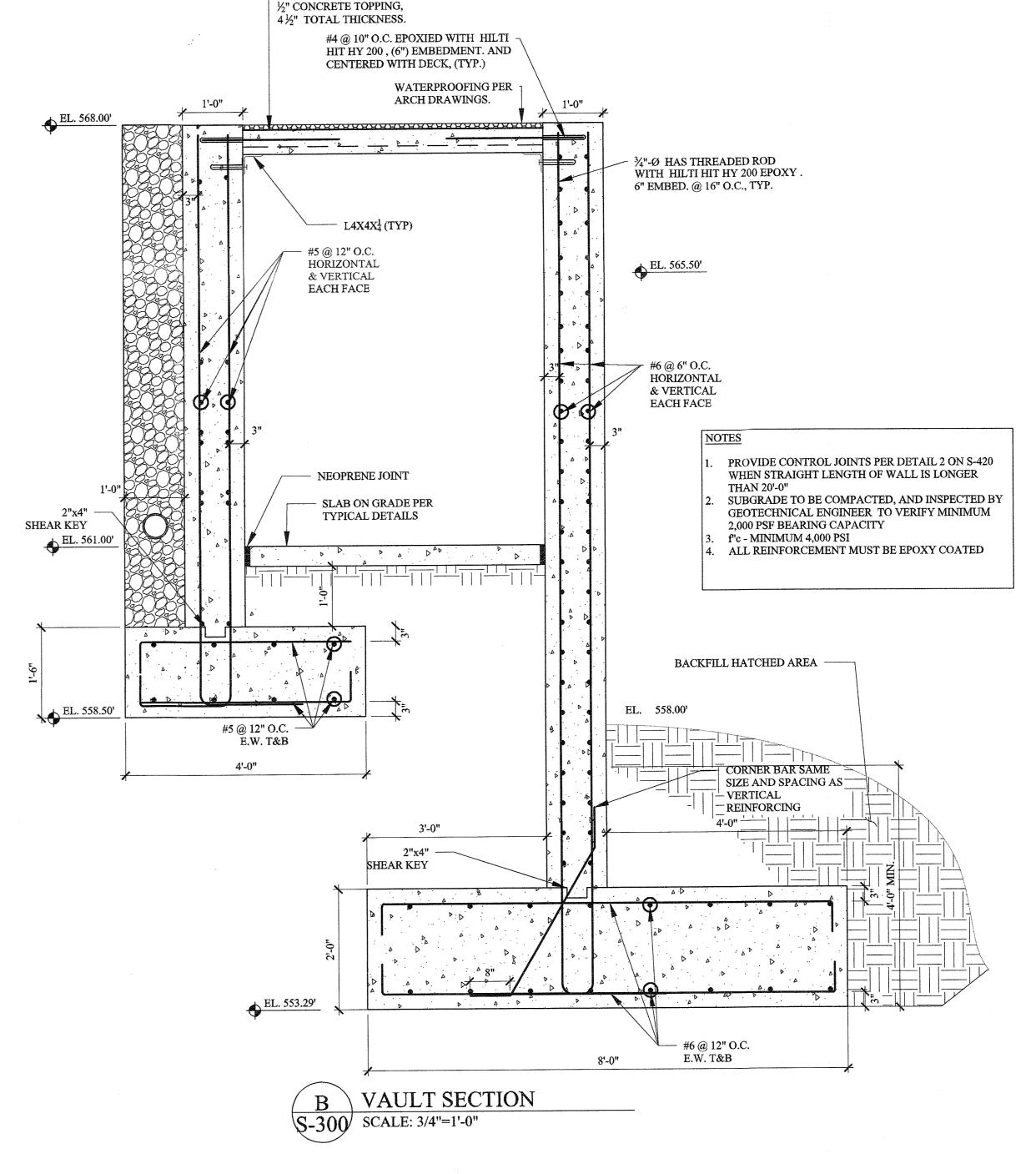
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DATE:	08/17/2021
PROJECT No.:	21-077
DRAWING BY:	AM/JB
CHK BY:	GF
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				RETA	INING WALL S	CHEDULE				
		FROM STA 0+00.00 TO STA 0+16.75	FROM STA 0+16.75 TO STA 0+25.00	FROM STA 0+25.00 TO STA 0+50.00	FROM STA 0+50.00 TO STA 0+75.00	FROM STA 0+75.00 TO STA 1+00.00	FROM STA 1+00.00 TO STA 1+25.00	FROM STA 1+25.00 TO STA 1+50.00	FROM STA 1+50.00 TO STA 1+75.00	FROM STA 1+75.00 TO STA 2+00.00
WALL 1	T.O.W. ELEVATION (FT)	569.0	569.0	566.0	566.0	566.0	566.0	566.0	566.0	566.0
	B.O.W. ELEVATION (FT)	564.0	562.0	560.5	558.0	558.0	558.0	558.0	561.5	561.5
	B.O.F. ELEVATION (FT)	560.0	558.0	556.5	554.0	554.0	554.0	554.0	557.5	557.5
WALL 2	T.O.W. ELEVATION (FT)	571.0	570.0	568.0	568.0	568.0	568.0	NO WALL	568.5	568.5
	B.O.W. ELEVATION (FT)	564.5	564.5	561.0	565.5	565.5	565.5	NO WALL	656.5	656.5
	B.O.F. ELEVATION (FT)	561.0	561.0	557.5	562.0	562.0	562.0	NO WALL	653.0	653.0
WALL 3	T.O.W. ELEVATION (FT)	571.0	571.0	570.0	570.0	570.0	570.0	NO WALL	571.0	571.0
	B.O.W. ELEVATION (FT)	570.5	570.5	567.5	567.5	567.5	567.5	NO WALL	568.5	568.5
	B.O.F. ELEVATION (FT)	567.0	567.0	564.0	564.0	564.0	564.0	NO WALL	565.0	565.0



VULCRAFT 2VLI18 DECK WITH 2

Ike Kligerman Barkley

330 West Forty-Second Street New York, NY 10036 212 268 0128

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No.	REMARKS	DATE
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RESIDENCE
59 TWEED BOULEVARD
ORANGEBURG, NY 10960



RETAINING WALL SECTIONS

DATE:	08/17/202 ⁻
PROJECT No.:	21-077
DRAWING BY:	AM/JE
CHK BY:	GF
DWG No.:	

S-400.00

- FIBER REINFORCED CONCRETE. USE 4 LB/YD^3

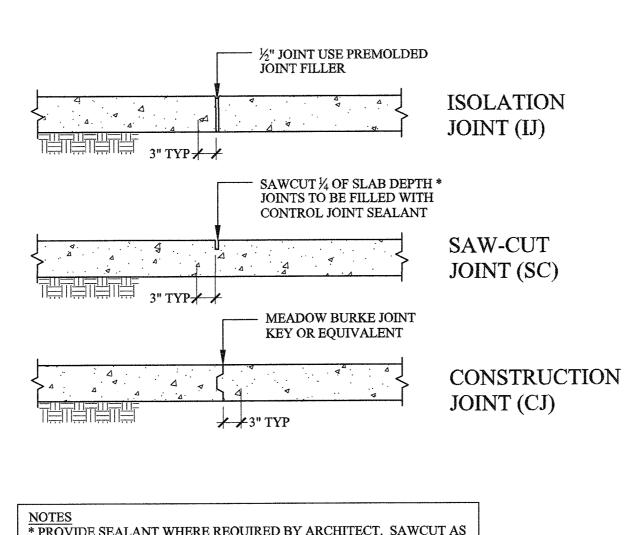
TUF-STRAND SF 1 MACRO SYNTHETIC FIBER

BY EUCLID CHEMICAL OR APPROVED EQUAL.

- NON-ORGANIC VAPOR BARRIER

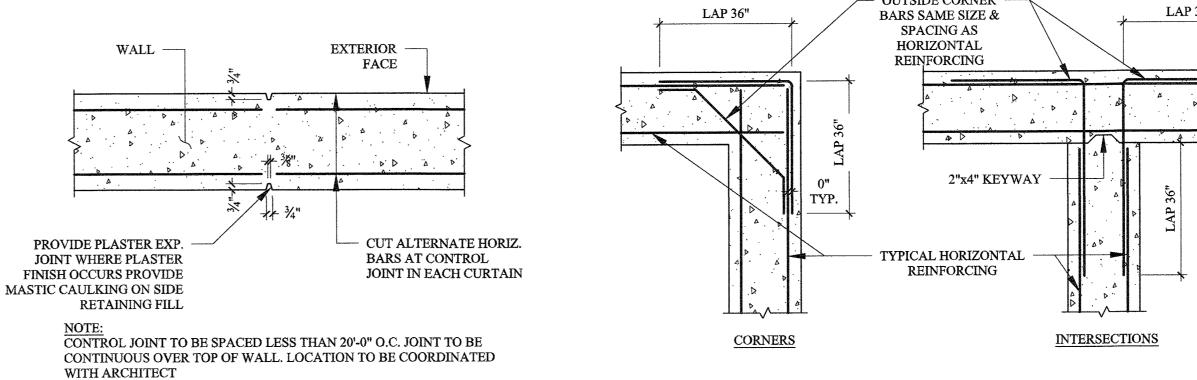
COMPACTED SUBGRADE **EXTERIOR SLAB**

— 15 MIL POLYETHYLENE

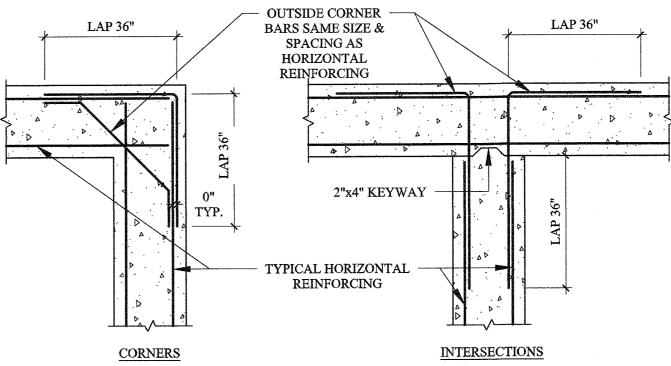


NOTES
* PROVIDE SEALANT WHERE REQUIRED BY ARCHITECT. SAWCUT AS SOON AS POSSIBLE AFTER CONCRETE HARDENS. SAW CUTTING MUST BE COMPLETE WITHIN 8 HRS AFTER CASTING.

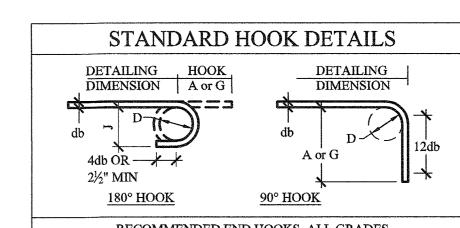
SLAB ON GRADE CONTROL JOINTS SCALE: 3/4" = 1'-0"



CONTROL JOINT SCALE: 3/4"=1'-0"



TYP. HORIZONTAL REINFORCING CONCRETE TIE BEAMS, WALLS, AND FOOTINGS SCALE: 3/4"=1'-0"



BAR SIZE RECOMMENDED END HOOKS, ALL GRADES 90° HOOKS 180° HOOKS A or G A or G 21/2" 3" 6" 8" 33/4" 10" 41/2" 8" 6"

#3 #4 #6 1'-0" 1'-2" 5½" 10" 1'-4" 11" 1'-7" 1'-3" 11¾" 1'-5" 1'-11/4" 1'-10" 1'-23/4" #11 12" 1'-7" 2'-0"

MIN. LAP SPLICE LENGTH SCHEDULE

	17111 17 1				
	fc=4	,000 psi	fy=60,00	0 psi	
THAN DB,	PACING OF B CLEAR COVI N TIES THRO	ER NOT LESS	THAN db, AN	ND BEAM STI	RRUPS OR
		0	R		
CLEAR SI	PACING OF B. THAN 2DB A				
Bar size	dc	Id	Id (top bar)	1.3*ld	1.3*ld(top)
#4	0.500	19	25	25	32
#5	0.625	24	31	31	40
#6	0.750	28	37	37	48
#7	0.875	42	54	54	70
#8	1.000	47	62	62	80
#9	1.128	54	70	70	90
#10	1.270	60	78	78	102
#11	1.410	67	87	87	113
		Other	cases		
Bar size	dc	Id	Id (top bar)	1.3*ld	1.3*ld(top)
#4	0.500	28	37	37	48
#5	0.625	36	46	46	60
#6	0.750	43	55	55	72
#7	0.875	62	81	81	105
#8	1.000	71	92	92	120
	1				l

90

100

104

117

130

117

130

153

170

1.128

1.270

1.410

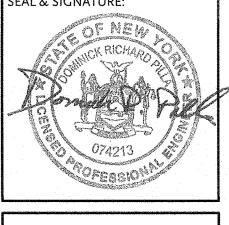
330 West Forty-Second Street New York, NY 10036 212 268 0128

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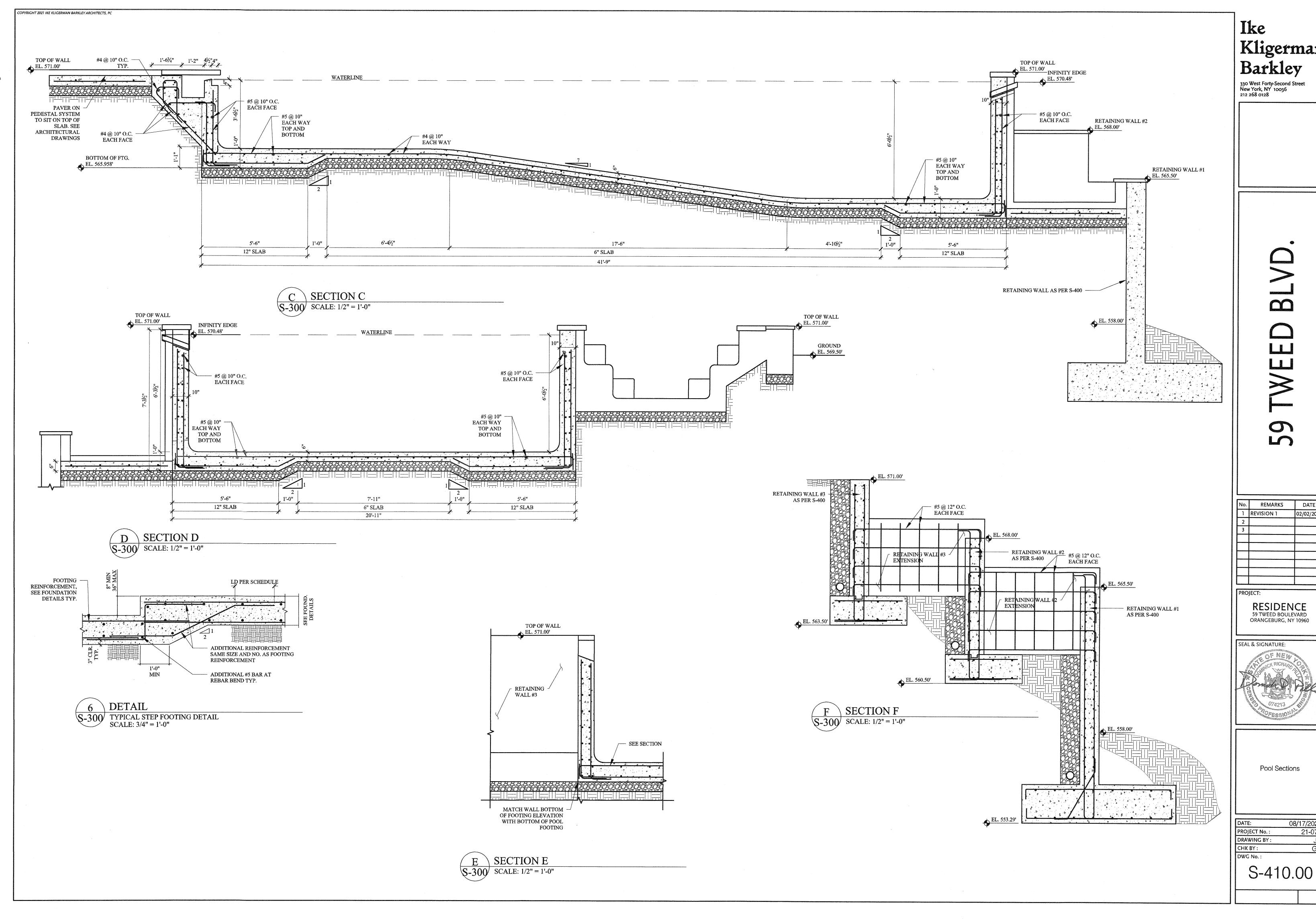
RESIDENCE 59 TWEED BOULEVARD ORANGEBURG, NY 10960

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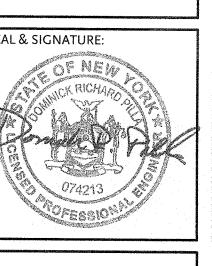


RETAINING WALL DETAILS

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ROJECT No. :	21-077
RAWING BY:	AM/JE
HK BY:	GF
WG No.:	



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DATE:	08/17/202
PROJECT No.:	21-07
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CHK BY:	G
DWG No.:	