COPYRIGHT 2021 IKE KLIGERMAN BARKLEY ARCHITECTS, PC VERIFY ALL DIMENSIONS IN FIELD

FOUNDATION CONSTRUCTION SPECIFICATIONS:

- ALL WORK SHALL COMPLY WITH THE REQUIREMENTS OF THE BUILDING CODE 2015 OF NEW YORK STATE AND THE APPLICABLE CONCRETE, MASONRY, AND REINFORCEMENT SPECIFICATIONS.
- 2. THE FOUNDATION DRAWINGS SHALL BE USED IN CONJUNCTION WITH THE SPECIFICATIONS, ARCHITECTURAL, MECHANICAL, PLUMBING AND ELECTRICAL DRAWINGS, AND ALL SUBCONTRACTOR SHOP DRAWINGS.
- 3. THE CONTRACTOR SHALL VERIFY ALL EXISTING FIELD CONDITIONS BEFORE EXCAVATION, AND NOTIFY THE ARCHITECT OF ANY DISCREPANCIES.
- 4. THE CONTRACTOR SHALL LOCATE ANY UNDERGROUND UTILITY LINES AND REPORT FINDINGS TO ENGINEER PRIOR TO ANY EXCAVATION.
- 5. THE CONTRACTOR SHALL COORDINATE FOUNDATION WORK WITH DIMENSIONS, OPENINGS, SLEEVE BLOCK OUTS, AND EMBEDDED ITEMS REQUIRED BY OTHER TRADES AND DRAWINGS, AND SUBMIT FOR APPROVAL COMPOSITE DRAWINGS SHOWIN THE SIZE AND LOCATION OF THESE ELEMENTS.
- 6. THE CONTRACTOR SHALL ENGAGE A GEOTECHNICAL ENGINEER WITH APPROPRIATE FIELD AND LABORATORY TESTING SUPPORT FOR INSPECTION OF THE FOLLOWING WORK:
 - A. EXCAVATION AND PREPARATION OF FOOTING SUBGRADE.

 B. SELECTION, PLACEMENT, AND COMPACTION OF CONTROLLED FILL.

 C. PREPARATION OF SLAB SUBGRADE.
- 7. ALL FOOTINGS ARE TO BEAR ON UNDISTURBED SOIL WITH AN ALLOWABLE BEARING PRESSURE OF 4,000 PSF. IF PREDOMINATELY CLAYEY AND/OR SILTY SOILS OR FILL ARE ENCOUNTERED BENEATH FOOTINGS, THE MATERIAL SHALL BE REMOVED AND REPLACED WITH COMPACTED STRUCTURAL FILL. DO NOT PLACE FOOTINGS ON LOOSE FILL, FROZEN GROUND, MUD, INORGANIC MATERIAL, OR ANY OTHER UNSUITABLE OR UNAPPROVED MATERIAL.
- 8. STRUCTURAL FILL SHALL CONSIST OF INORGANIC SOIL, FREE OF CLAY, LOAM, ICE, SNOW, TREE SUMPS, ROOTS, AND OTHER ORGANIC MATERIALS, GRADED WITHIN THE FOLLOWING LIMITS:

SIEVE SIZE	PERCENT BY WEIGHT
4 INCH	100
2 NCH	65-100
NO. 4	45-95
NO. 16	25-80
NO. 50	10-65
NO. 200	0-10

- 9. STRUCTURAL FILL SHALL BE PLACED IN MAXIMUM LIFTS OF 6 INCHES BEFORE COMPACTION. EACH LIFT SHALL BE COMPACTED AT OR NEAR OPTIMUM MOISTURE WITH APPROPRIATE EQUIPMENT TO ≥ 95% OF MAXIMUM DENSITY.
- 10. BACKFILLING AGAINST FOUNDATION OR RETAINING WALLS SHALL NOT BE UNDERTAKEN UNTIL CONCRETE HAS REACHED 85 % OF DESIGN STRENGTH. PROVIDE TEMPORARY BRACING FOR ALL FOUNDATION WALLS UNTIL FLOOR SLABS ON GRADE AND BRACING WALLS HAVE REACHED 75% OF DESIGN STRENGTH.
- 11. BOTTOM ELEVATIONS OF FOOTINGS SHOWN ON PLANS ARE FOR ESTIMATING PURPOSES ONLY. THE ACTUAL FOOTING ELEVATIONS SHALL BE BASED ON THE REQUIRED BEARING STRATA, AS DETERMINED BY THE GEOTECHNICAL ENGINEER, FOLLOWING EXCAVATION.
- 12. VERTICAL CONSTRUCTION JOINTS IN FOUNDATION WALLS SHALL BE PLACED:
 A. 40 FT ON CENTER (MAX.)
 B. 4'-6" FROM ANY OPENING
 C. 10'-0" FROM ANY CORNER
- 13. ALL ELECTRICAL SLOTS, PLUMBING SLOTS, SLEEVES, AND DEPRESSIONS FOR LEAD BENDS SHALL BE FILLED WITH CONCRETE TO THE DEPTH OF SLABS AFTER PIPING IS INSTALLED.
- 14. SLABS ON GRADE SHALL BE PLACED ON 15 MIL. POLYETHYLENE VAPOR BARRIER ON A COMPACTED 6 IN. THICK BASE OF CRUSHED STONE OR GRAVEL, UNLESS OTHERWISE SPECIFIED.

CONCRETE SPECIFICATIONS:

1. THE LATEST EDITION OF THE FOLLOWING STANDARDS APPLY:

ACI 318 (CODE)	ACI 304 (PLACING)
ACI 306 (WINTER CONCRETING)	ACI 315 (DETAILING)
ACI 305 (HOT WEATHER CONCRETING)	ACI 347 (FORMWORK)
ACI 211.1 (MIX PROPORTIONING)	ACI 301 (SPECIFICATIONS)

- 2. THE CONTRACTOR SHALL PROVIDE CERTIFIED DESIGN MIX DATA TO THE ENGINEER PRIOR TO THE PLACING OF ANY CONCRETE.
- 3. ALL CONCRETE SHALL BE READY MIXED, CONTROLLED CONCRETE BASED ON APPROVED DESIGN MIXES HAVING A MINIMUM 28-DAY COMPRESSIVE CYLINDER STRENGTH AS LISTED BELOW. SPECIAL DESIGN MIXES SHALL BE MADE FOR ALL PUMPED CONCRETE. FLY ASH MAY BE USED AS A PLASTICIZER TO REDUCE WATER CONTENT, BUT CANNOT BE USED TO REDUCE CEMENT CONTENT. AGGREGATE SHALL BE LIMITED TO A MAXIMUM OF 3/4".

LOCATION	28-DAY COMPRESSIVE STRENGTH (PSI)
MISC. FILL MUD SLABS	2,500
FOOTINGS FOUND. WALLS	3,500

SLAB ON GRADE FRAMED SLABS, ETC.

- 4. NO ADMIXTURES SHALL BE USED UNLESS SPECIFIED OR APPROVED BY THE ARCHITECT AND ENGINEER. APPROVED ADMIXTURES SHALL COMPLY WITH ACI SPECIFICATIONS. THE USE OF CALCIUM CHLORIDE IS STRICTLY FORBIDDEN.
- 5. PROVIDE ALL PUMPING, SHORING AND BRACING AS NECESSARY.
- 6. ALL FORMWORK SHALL BE DESIGNED, ERECTED, SUPPORTED, BRACED, AND MAINTAINED ACCORDING TO ACI 347
- 7. NO ALUMINUM METAL IN ANY FORM SHALL BE PERMITTED IN CONCRETE.
- 8. SLABS ON GRADE SHALL BE CAST IN ALTERNATE POURS IN A CHECKERBOARD PATTERN. THE MAXIMUM AREA OF EACH POUR SHALL NOT EXCEED 900 SQ. FT. WITH SIDE LENGTHS NOT EXCEEDING 30 FT. ALLOW A MINIMUM OF 3 DAYS BEFORE MAKING ADJACENT POURS.

REINFORCEMENT SPECIFICATIONS

- ALL REINFORCING BARS SHALL BE DEFORMED BARS CONFORMING TO ASTM 615, GRADE 60.
- 2. ALL WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185, GRADE 60 AND SHALL BE MADE OF COLD DRAWN WIRE CONFORMING TO ASTM A82.
- 3. ALL REINFORCING BARS AND TAGS SHALL BE FREE OF LOOSE FLAKY RUST, MUD, OIL OR OTHER COATINGS THAT WILL DESTROY, REDUCE OR HAMPER FULL BOND CAPACITY
- 4. ALL REINFORCEMENT SHALL BE SECURELY TIED IN PLACE AND ADEQUATELY SUPPORTED. ALL BARS MARKED CONT. (CONTINUOUS) ARE TO BE LAPPED OR HOOKED AT NON-CONTINUOUS ENDS. SPLICE CONT. TOP BARS AT CENTER BETWEEN SUPPORTS AND BOTTOM BARS SHALL BE SPLICED AT SUPPORTS.
- 5. ALL WELDED WIRE FABRIC FOR SLAB ON GRADE SHALL BE CONTINUOUS. LAP SPLICE 12" MINIMUM WHERE REQUIRED.
- 6. REBAR SHALL BE LAP SPLICED 40 BAR DIAMETERS IN CAST-IN-PLACE CONCRETE AND 48 BAR DIAMETERS IN MASONRY.

	REBAR	LAP SPLICE SCH	HEDULE		
IMPERIAL BAR	"SOFT" METRIC	NOMINAL	SPLICE LENGTH (IN.)		
SIZE	SIZE"	DIAMETER (IN.)	CONCRETE	MASONRY	
#3	#10	0.375	15	18	
#4	#13	0.500	20	24	
#5	#16	0.625	25	30	
#6	#19	0.750	30	36	
#7	#22	0.875	35	42	
#8	#25	1.000	40	48	
#9	#29	1.128	45	54	
#10	#32	1.270	51	61	
#11	#36	1.410	56	68	

Ike Kligerman Barkley

330 West Forty-Second Street New York, NY 10036 212 268 0128

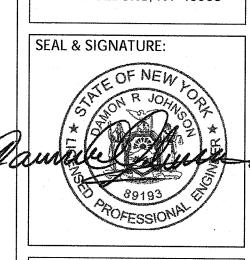
> HANINGTON ENGINEERING CONSULTANTS, LLC

501 FIFTH AVE, SUITE 909 NEW YORK, NY 10017 TEL 212 338-9801 DRAWINGS@HANINGTONENGINEERING.COM JOB #21-2970

No.	REMARKS	DATE
1	ISSUED FOR REVIEW	05.06.21
2	ISSUED FOR REVIEW	05.24.21
3	ISSUED FOR REVIEW	06.24.21
4	ISSUED FOR REVIEW	08.03.21
5	ISSUED FOR FILING	08.13.21

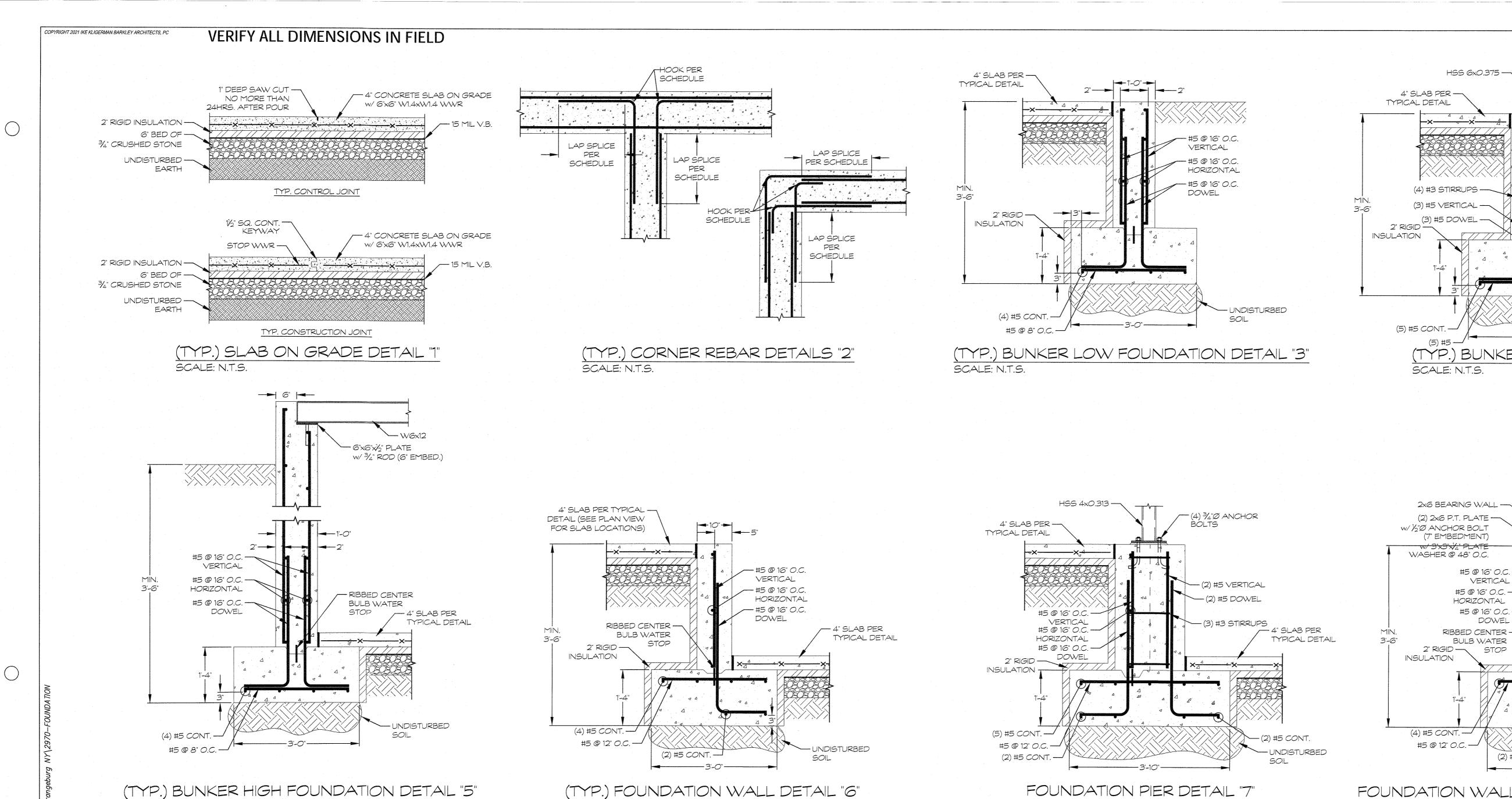
PROJECT:

RESIDENCE
59 TWEED BOULEVARD
ORANGEBURG, NY 10960



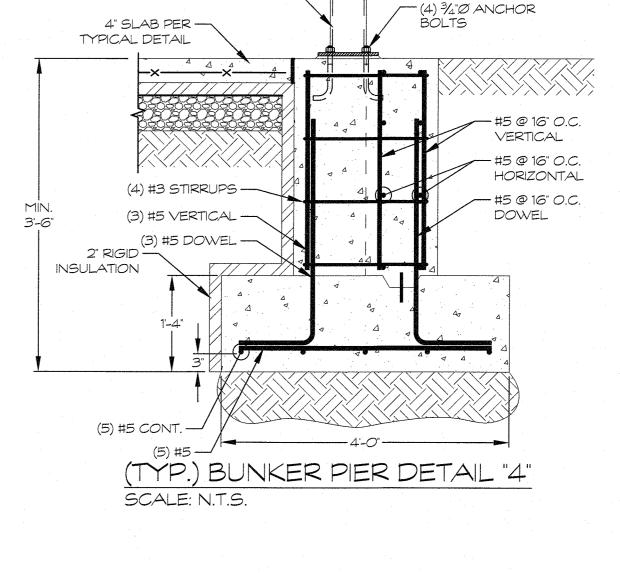
FOUNDATION
SPECIFICATIONS &
DETAILS

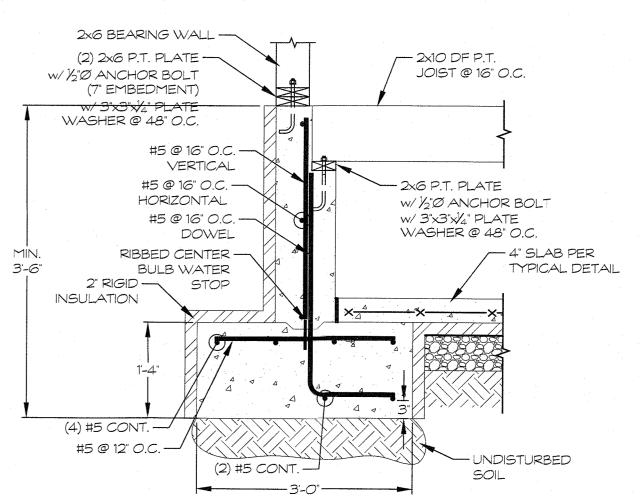
DATE:	01.08.2021
PROJECT No.:	20017
DRAWING BY:	JDZ
CHK BY:	DRI
DWG No.:	
FO-10	00.00



SCALE: N.T.S.

SCALE: N.T.S.





FOUNDATION WALL AT CRAWL SPACE DETAIL "8" SCALE: N.T.S.

Ike Kligerman Barkley

330 West Forty-Second Street New York, NY 10036 212 268 0128

HANINGTON
ENGINEERING
CONSULTANTS, LLC

501 FIFTH AVE, SUITE 909
NEW YORK, NY 10017
TEL 212 338-9801

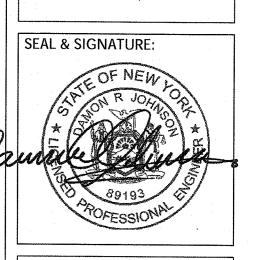
DRAWINGS@HANINGTONENGINEERING.COM

JOB #21-2970

No.	REMARKS	DATE
1	ISSUED FOR REVIEW	05.06.2
2	ISSUED FOR REVIEW	05.24.21
3	ISSUED FOR REVIEW	06.24.2
4	ISSUED FOR REVIEW	08.03.21
5	ISSUED FOR FILING	08.13.21

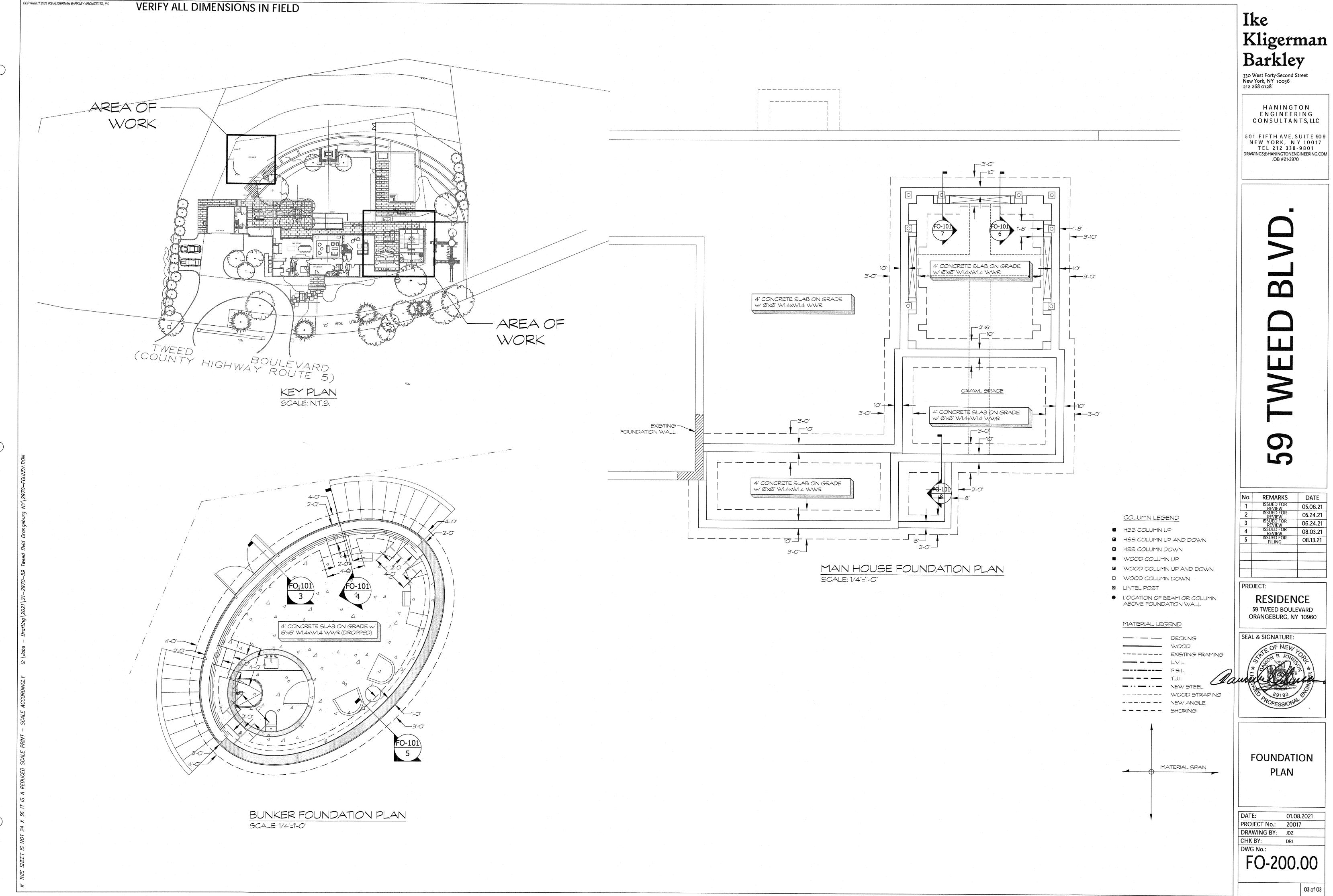
PROJECT:

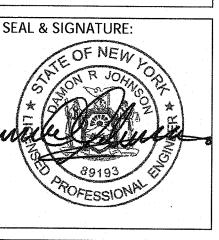
RESIDENCE 59 TWEED BOULEVARD ORANGEBURG, NY 10960



FOUNDATION
SPECIFICATIONS &
DETAILS

FO-10	01.00
DWG No.:	
CHK BY:	DRJ
DRAWING BY:	JDZ
PROJECT No.:	20017
DATE:	01.08.2021





VERIFY ALL DIMENSIONS IN FIELD

GENERAL STRUCTURAL SPECIFICATIONS:

COPYRIGHT 2021 IKE KLIGERMAN BARKLEY ARCHITECTS, PC

- 1. ALL WORK SHALL COMPLY WITH THE REQUIREMENTS OF THE BUILDING CODE 2015 OF NEW YORK STATE.
- 2. ALL HEC DRAWINGS ARE SCEMATIC ONLY. THE SCALES INDICATED ON THE PLANS ARE FOR REFERENCE ONLY.
- 3. THE STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH THE SPECIFICATIONS, ARCHITECTURAL, MECHANICAL, PLUMBING, AND ELECTRICAL DRAWINGS, AND ALL SUBCONTRACTOR SHOP DRAWINGS AND ARE APPROVED ONLY FOR THE WORK INDICATED ON THE PLANS.
- 4. THE CONTRACTOR SHALL PROVIDE ALL SHORING, BRACING, AND SIMILAR APPARATUS AS MAY BE REQUIRED TO STABILIZE THE STRUCTURE DURING THE COURSE OF ALL CONSTRUCTION.

STEEL CONSTRUCTION SPECIFICATIONS

- 1. ALL STRUCTURAL STEEL WORK SHALL CONFORM TO THE AISC "MANUAL OF STEEL CONSTRUCTION," LATEST EDITION.
- 2. ALL WELDING SHALL BE IN ACCORDANCE WITH THE "STRUCTURAL WELDING CODE," AWS D1.1, LATEST EDITION AND BE PERFORMED BY WELDERS CERTIFIED IN ACCORDANCE WITH AWS STANDARDS.
- 3. STEEL MATERIALS SHALL CONFORM TO THE FOLLOWING AND BE ACCOMPANIED BY CERTIFIED MILL REPORTS UNLESS OTHERWISE NOTED ON THE DRAWINGS:

W-SHAPES & WT-SHAPES	ASTM A992
C-SHAPES & MC-SHAPES	ASTM A36
HSS SHAPES	ASTM 500, GRADE C
ANGLES, PLATES, & MISC	ASTM A36
STEEL PIPE	•
HIGH STRENGTH BOLTS	ASTM F3125, GRADE A325
MACHINE BOLTS	ASTM A307, GRADE A
ANCHOR BOLTS/RODS	
WELDING ELECTRODES	ASTM E70XX

- 4. THE CONTRACTOR SHALL PROVIDE 1 REPRODUCIBLE AND 4 COPIES OF SHOP DRAWINGS. THE 4 COPIES WILL BE RETAINED BY THE ARCHITECT AND ENGINEER AND THE REPRODUCIBLE WILL BE RETURNED TO THE CONTRACTOR. PREFERRED ALTERNATIVE: PROVIDE ELECTRONIC COPIES (PDF) OF SHOP DRAWINGS FOR A / E REVIEW AND PROCESSING.
- 5. FRAMING CONNECTIONS NOT DETAILED, OR CONNECTIONS THAT ARE MODIFIED FROM THOSE DETAILED, SHALL BE DESIGNED BY THE SUPPLIER FOR THE END REACTION SHOWN ON THE PLAN. IF NO END REACTION IS PROVIDED, DESIGN FOR 1/2 THE BEAM MAXIMUM UNIFORM LOAD PER THE AISC MANUAL FOR STEEL CONSTRUCTION.
- 6. AVOID ONE-SIDED CONNECTIONS IF POSSIBLE. IF IT IS NECESSARY TO USE A ONE-SIDED CONNECTION, THIS CONNECTION SHALL BE DESIGNED IN ACCORDANCE WITH AISC MANUAL AND SHALL HAVE A MINIMUM OF TWO LINES OF BOLTS IN EACH LEG OF THE ONE-SIDED CONNECTION ANGLE.
- 7. SHOP CONNECTIONS SHALL BE WELDED OR HIGH STRENGTH BOLTED. UNLESS OTHERWISE NOTED, USE MINIMUM 3/6" FILLET WELDS AND 3/4"Ø BOLTS IN STANDARD HOLES.
- 8. FIELD CONNECTIONS SHALL BE WELDED OR HIGH STRENGTH BOLTED. NO FIELD WELDING OF HOT DIPPED GALVANIZED MEMBERS WILL BE ALLOWED, UNLESS OTHERWISE NOTED, USE MINIMUM 3/6" FILLET WELDS AND 3/4"Ø BOLTS IN STANDARD HOLES.
- 9. CONTRACTOR SHALL FURNISH ALL PLATES, CLIP ANGLES, CONNECTIONS. NAILER HOLES, ETC. REQUIRED FOR THE COMPLETION OF THE STRUCTURE EVEN IF EVERY SUCH ITEM IS NOT SHOWN ON DRAWINGS.
- 10. PROVIDE ONE SHOP COAT OF PRIMER ON ALL STEEL EXCEPT FOR ITEMS TO BE HOT DIPPED GALVANIZED OR SPRAY FIREPROOFED. DO NOT PAINT PORTIONS
- 11. ALL EXTERIOR ELEMENTS AND THOSE ELEMENTS NOTED TO BE GALVANIZED SHALL BE HOT DIPPED GALVANIZED IN ACCORDANCE WITH ASTM A123 AFTER SANDBLAST CLEANING. USE ASTM F3125, GR. A325 BOLTS HOT DIPPED GALVANIZED WITH GALVANIZED HARDENED WASHERS AND GALVANIZED HEAVY HEX NUTS FOR BOLTING OF GALVANIZED ITEMS.
- 12. BEAMS OR JOISTS WITH AN ACCEPTABLE AMOUNT OF CAMBER (1.4D) SHALL BE INSTALLED WITH THE CAMBER SIDE UP.
- 13. ALL HOLES AND CUTS SHALL BE SHOWN ON THE SHOP DRAWINGS AND MADE IN THE SHOP. CUTS OR BURNING OF HOLES IN STRUCTURAL STEEL IN THE FIELD WILL NOT BE PERMITTED.
- 14. NON-SHRINK NON-METALLIC GROUT WITH A MINIMUM 28 DAY STRENGTH OF 5,000 PSI SHALL BE USED UNDER BASE PLATES. THE USE OF LEVELING PLATES UNDER COLUMN BASE PLATES WILL NOT BE PERMITTED.
- 15. ALL COLUMNS ARE TO BE LATERALLY BRACED AT EACH FLOOR.
- 16. STEEL COLUMNS, BASE PLATES AND ALL STEEL BELOW GRADE SHALL HAVE A MINIMUM 3" CONCRETE COVER PROTECTION.
- 17. ALL EXISTING STEEL IS TO BE CLEANED OF RUST, SCALE, AND DEBRIS PRIOR TO WELDING OF NEW STEEL.
- 18. ALL CONTACT SURFACES WITHIN HIGH-STRENGTH BOLT CONNECTIONS AND WELDING AREAS SHALL BE FREE OF DIRT, OIL, OR ANY FOREIGN MATERIALS.
- 19. FIRE PROTECTION OF STRUCTURAL STEEL SHALL COMPLY WITH THE REQUIREMENTS OF THE BUILDING CODE.

WOOD FRAME CONSTRUCTION SPECIFICATIONS:

- 1. ALL LUMBER WORK AND MATERIALS SHALL CONFORM TO THE FOLLOWING CODES AND SPECIFICATIONS (LATEST EDITION):
 - A. AITC TIMBER CONSTRUCTION MANUAL
 - B. APA PLYWOOD DESIGN SPECIFICATION C. NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION (NDS)
- 2. ALL TOP-FLANGE AND FACE-MOUNTED HANGERS SHALL BE MANUFACTURED BY "SIMPSON STRONG TIE" UNLESS OTHERWISE APPROVED BY THE ENGINEER. HANGERS SHALL SELECTED BASED ON THE ACCOMPANYING SCHEDULES AND DETAILS, AND SHALL BE INSTALLED BASED ON THE MANUFACTURER'S SPECIFICATIONS.
- 3. ALL STRUCTURAL LUMBER THAT IS EXPOSED TO WEATHER, IN CONTACT WITH EARTH, OR WITHIN 18" OF EARTH SHALL BE PRESSURE TREATED IN ACCORDANCE WITH THE REQUIREMENTS OF THE AMERICAN WOOD PRESERVES' ASSOCIATION
- 4. ALL WOOD POST WHICH BEAR ON TOP OF CONCRETE SHALL UTILIZE POST BASES THAT DO NOT ALLOW FOR CONTACT BETWEEN WOOD AND CONCRETE POST BASES SHALL BE AS NOTED IN THE ACCOMPANYING SCHEDULES AND SHALL BE MANUFACTURED BY "SIMPSON STRONG TIE."
- 5. PROVIDE CROSS BRIDGING (METAL OR WOOD) OR SOLID BLOCKING AT 4 FT. ON CENTER MINIMUM FOR ALL JOISTS. WHEN BEARING WALLS RUN PERPENDICULAR TO JOISTS, INSTALL SOLID BLOCKING UNDERNEATH WALL.
- 6. TYPICAL END BEARING FOR ALL BEAMS AND JOISTS SHALL BE MINIMUM 4".
- 7. PLYWOOD SUB-FLOORS TO BE GLUED AND SCREWED TO JOISTS.
- 8. ALL STUD BEARING WALLS TO BE PROVIDED WITH TWO CONTINUOUS TOP PLATES AND ONE CONTINUOUS BOTTOM PLATE.
- 9. PLYWOOD SHEATHING FOR ROOFS, FLOORS, AND WALLS SHALL BE C-D EXPOSURE 1 APA RATED PLYWOOD SHEATHING ("CDX").

SOLID SAWN LUMBER SPECIFICATIONS:

1. ALL SOLID SAWN LUMBER SHALL BE DOUGLAS FIR (NORTH), NUMBER 2 OR BETTER WITH THE FOLLOWING MINIMUM PROPERTIES (SINGLE MEMBER):

BENDING HORIZONTAL SHEAR COMP. PERP. TO GRAIN COMP. PARALLEL TO GRAIN MODULUS OF FLASTICITY	F _{c0} = F _{c0} =	180 psi 625 psi 1,400 psi
MODULUS OF ELASTICITY	E =	1,600 ksi

- 2. ALL LUMBER SHALL BE KILN DRIED TO A MAXIMUM MOISTURE CONTENT OF 19%.
- 3. SEVERELY DISTORTED (TWISTED, BOWED, CUPPED, CHECKED, ETC.) LUMBER SHALL NOT BE USED.

STRUCTURAL COMPOSITE LUMBER (LVL, LSL, AND PSL) SPECIFICATIONS:

1. ALL LAMINATED VENEER LUMBER (LVL) SHALL BE TRUSS JOIST "MICROLLAM" AS MANUFACTURED BY "WEYERHAEUSER" WITH THE FOLLOWING MINIMUM

BENDING	E	_	2,600 p
HORIZONTAL SHEAR			285 p
COMP. PERP. TO GRAIN			750 p
COMP. PARALLEL TO GRAIN			2,510 p
MODULUS OF ELASTICITY			2,000 k

2. ALL PARALLEL STRAND LUMBER (PSL) SHALL BE TRUSS JOIST "PARALLAM" AS MANUFACTURED BY "WEYERHAEUSER" WITH THE FOLLOWING MINIMUM PROPERTIES:

BENDING	F_b	=	2,900 ps
HORIZONTAL SHEAR	F_{v}	=	290 ps
COMP. PERP. TO GRAIN	Fan	=	625 ps
COMP. PARALLEL TO GRAIN	F	=	2,900 ps
MODULUS OF ELASTICITY	E	=	2,000 ks

2. ALL LAMINATED STRAND LUMBER (LSL) SHALL BE TRUSS JOIST "TIMBERSTRAND" AS MANUFACTURED BY "WEYERHAEUSER" WITH THE FOLLOWING MINIMUM

BENDING	F_b	=	1,900 p
HORIZONTAL SHEAR	Fv	=	150 p
COMP. PERP. TO GRAIN	Fan	=	435 ps
COMP. PARALLEL TO GRAIN			1,400 p
MODULUS OF ELASTICITY			1,300 k

COMPOSITE WOOD "I"-JOIST SPECIFICATIONS:

- 1. ALL PLYWOOD WEB, I-JOIST LUMBER SHALL BE TRUSS JOIST "TJI" AS MANUFACTURED BY "WEYERHAEUSER."
- 2. THE CONTRACTOR SHALL INSTALL ALL I-JOISTS IN STRICT ACCORDANCE WITH THE MANUFACTURER'S SPECIFICATIONS. THE WEBS MAY BE DRILLED OR CUT IN THE FIELD ACCORDING TO THE MANUFACTURERS SPECIFICATIONS AFTER OBTAINING APPROVAL FROM THE ENGINEER, DO NOT CUT OR NOTCH FLANGES.
- 3. I-JOISTS ARE TO BE STACKED ON MINIMUM 6"X6" SUPPORT BLOCKS WITH A MAXIMUM SPACING OF 10' O.C. WHEN STACKING MULTIPLE UNITS, 2"x3" "STICKERS" SHOULD BE USED BETWEEN UNITS. I-JOISTS SHOULD BE STORED AND HANDLED IN THE VERTICAL ORIENTATION.
- 4. RIM BOARDS SHALL BE TJ RIM BOARD OR TIMBERSTRAND LSL AS MANUFACTURED BY "WEYERHAEUSER," RIM BOARDS SHALL BE 1/8" MINIMUM THICKNESS AND SHALL MATCH THE I-JOIST DEPTH.
- 5. PLYWOOD SUBFLOORS TO BE GLUED AND SCREWED TO "TJI" JOISTS.

DESIGN CRITERIA (USING ASCE 7-16):

1. GENERAL CRITERIA (CHAPTER 1)

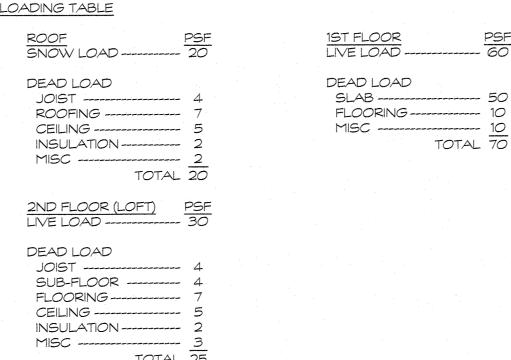
- RISK CATEGORY	 (SECTION 1.5)
SNOW CRITERIA (CHAPTER 7)	

- GROUND SNOW LOAD, pa	30	(SECTION 7.2)
- EXPOSURE FACTOR, Ce		(TABLE 7-2)
- THERMAL FACTOR, Ct	1.0	(TABLE 7-3)
- IMPORTANCE FACTOR, Is	1.0	(TABLE 1.5-2)
- FLAT ROOF SNOW LOAD, P	21	(EQUATION 7.3-1)

3.	SE	ISMIC CRITERIA (CHAPTER 11)		
	-	MPORTANCE FACTOR, _e	D 29.9 %g 6.2 %g	(TABLE 1.5-2) (SECTION 11.4.2) (SECTION 11.4.1) (SECTION 11.4.1)
	_	SEISMIC DESIGN CATEGORY	В	(SECTION 11,6)

4. WIND CRITERIA (CHAPTER 26 TO CHAPTER 31)

	BASIC WIND SPEED, V	114 MPH	(FIGURE 26.5-1)
_	WIND DIRECTIONALITY FACTOR, Kg	0.85	(SECTION 26.6)
_	EXPOSURE CATEGORY	В	(SECTION 26.7)
_	TOPOGRAPHIC FACTOR, Kzt	1.0	(SECTION 26.8)
_	GUST EFFECT FACTOR	0.85	(SECTION 26.9)
_	ENCLOSURE CLASSIFICATION	ENCLOSED	(SECTION 26.10)
_	INTERNAL PRESSURE COEFFICIENT	+/- 0.18	(SECTION 26.11)



	PLYWO	OD SHEATHING S	CHEDULE	
USE	THICKNESS	SPAN RATING	EDGE NAILING	FIELD NAILING
FLAT ROOF	3/4"	48"	8d @ 6" O.C.	8d @ 12" O.C.
SLOPED ROOF	5/8"	40"	8d @ 6" O.C.	8d @ 12" O.C.
FLOOR	3/4" T&G	24"	8d @ 4" O.C.	8d @ 12" O.C.
WALL	½" MIN.	24"	8d @ 4" O.C.	8d @ 12" O.C.

- I. PLYWOOD FOR ROOFS, FLOORS, & WALLS SHALL BE C-D EXPOSURE 1 APA RATED PLYWOOD SHEATHING ("CDX").
- 2. LAY UP PLYWOOD WITH FACE GRAIN PERPENDICULAR TO SUPPORTS WITH STAGGERED JOINTS. 3. ALL NAILS TO BE COMMON NAILS. WHEN USED IN FLOOR AND ROOF SHEATHING,
- NAILS ARE TO BE RING SHANKED. 4. ALL FLOOR SHEATHING TO BE GLUED AND SCREWED.
- 5. ALL EDGES OF WALL SHEATHING SHALL BE BLOCKED AND SCREWED.

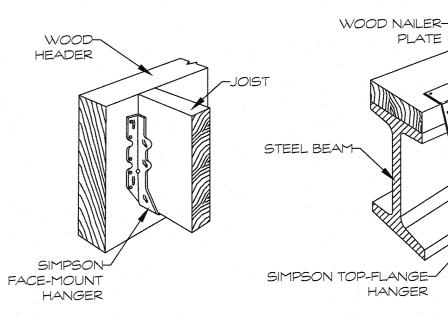
	WOOD LINTEL SCHEDULE								
DESIGNATION	SPAN	HEADER SIZE	POST SIZE (EA. END)						
	0'-0" TO 4'-0"	(2) 2"×8"	1 JACK / 1 KING STUD						
L2	4'-0" TO 6'-0"	(2) 2"x10"	1 JACK / 1 KING STUD						
L3	6'-0" TO 8'-0"	(2) 13/4" × 91/2" LVL	1 JACK / 1 KING STUD						
L4	8'-0" TO 10'-0"	(3) 13/4" × 91/2" LVL	1 JACK / 1 KING STUD						
L5	10'-0" TO 12'-0"	(3) 1¾" × 11%" LVL	1 JACK / 1 KING STUD						
L6	12'-0" TO 15'-0"	(3) 1¾" × 14" LVL	1 JACK / 2 KING STUD						
L7	15'-0" TO 18'-0"	(3) 13/4" × 16" LVL	2 JACK / 2 KING STUD						

WOOD HEADER NOTES UNLESS OTHERWISE NOTES ON PLAN, HEADER SIZES ARE TYPICAL THROUGHOUT BUILDING. HEADER SIZES ARE FOR INTERNAL WOOD CONSTRUCTION (DOOR, ARCH, OPENINGS, ETC.). THEY ARE NOTE INTENDED TO SUPPORT STONE OR BLOCK WORK,

HEADER SIMPSON-

TOP-FLANGE

HANGER





TOP-FLANGE HANGER TO NAILER

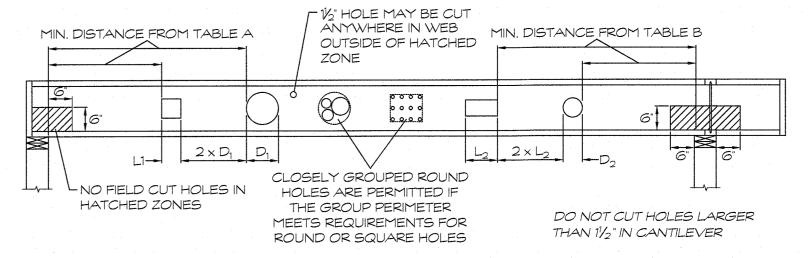


TABLE A - END SUPPORT (MINIMUM DISTANCE FROM EDGE OF HOLE TO INSIDE FACE OF NEAREST END SUPPORT)

DEDTI	ROUND HOLE SIZE								SQL	IARE C	OR REC	TANG	ULAR	HOLE	SIZE				
DEPTH	ال	2"	3"	4"	5"	61/2"	7"	87/8"	11"	13"	2"	3"	4"	5	61/2"	7"	87/8"	11"	13"
11%"	560	1-6"	2'-6"	3'-0"	4'-0"	5'-6"	6'-0"	8'-0"			2'-6"	3'-6"	4-6	5-6	7-0"	7-6"	8 <u>'</u> -Ö		
	B - INTE			-															
	B - INTE DISTANC			E OF H	HOLE -	TO INS	IDE FA	ACE OF			NTERM			CANTIL OR REC				SIZE	
				E OF H	HOLE -		IDE FA	ACE OF			NTERM							SIZE 11"	13"

1. HOLES MAY BE LOCATED VERTICALLY ANYWHERE WITHIN THE WEB. LEAVE 1/8" WEB (MINIMUM) AT THE TOP AND BOTTOM OF 2. KNOCKOUTS ARE LOCATED IN WEB AT APPROXIMATELY 12" ON-CENTER; THEY DO NOT AFFECT HOLE PLACEMENT.

3. RECTANGULAR HOLES BASED ON MEASUREMENT OF LONGEST SIDE. 4. FOR OTHER HOLE CONFIGURATIONS, OBTAIN WRITTEN APPROVAL FROM ENGINEER

		SIMPSON TOP-	FLANGE HANGER	S		
			HANGER TO HEADER		HANGER TO NAILER	
JOIST DESCRIPTION	HANGER DESCRIPTION	JOIST	HEADER		NAILER (DF)	TOP FLANGE
	DESORIF HOIN		TOP	FACE		NAILING
2x10	LB210AZ	(2) 0.148 × 1/2	(6) 0.10	52 × 31/2	2x	(6) 0.148 × 1/2
117/8" TJI 560	ITS3.56/11.88		(4) 0.148 × 3	(2) 0.148 × 3	2x	(6) 0.148 × 1/2

330 West Forty-Second Street New York, NY 10036 212 268 0128

> HANINGTON ENGINEERING CONSULTANTS, LLC

501 FIFTH AVE, SUITE 909 NEW YORK, NY 10017 TEL 212 338-9801 DRAWINGS@HANINGTONENGINEERING.COM JOB #21-2970

REMARKS 05.06.21 05.24.21 06.24.21 08.03.21 08.13.21

PROJECT:

RESIDENCE 59 TWEED BOULEVARD ORANGEBURG, NY 10960

SEAL & SIGNATURE:

STRUCTURAL **SPECIFICATIONS**

01.08.2021 PROJECT No.: 20017 DRAWING BY: JDZ CHK BY:

DWG No.:

BEAM SIZE

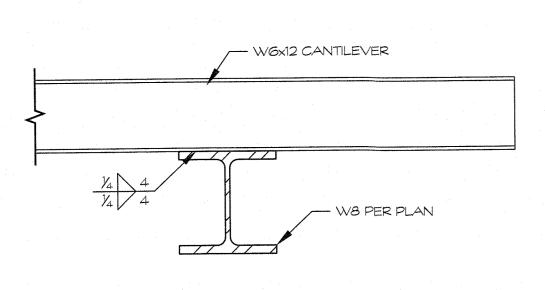
W8, W10

W12, W14

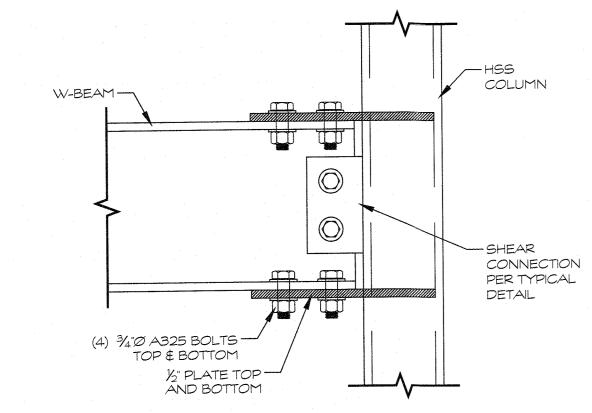
	W16, W18	4	
	L4"x4"x3/6" BOTH SIDES BEAM	G	IRDER
	3"		
	3"		- 3/4"Ø A325 BOLTS W/ WASHERS "X" EACH SIDE
"X" 3/4"(A325 BOLTS		

"X" BOLTS

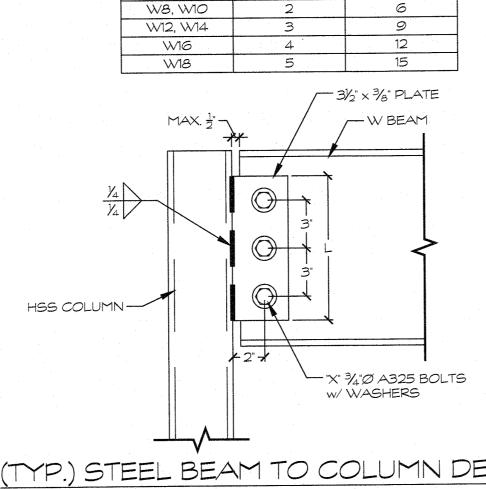
(TYP.) STEEL BEAM TO BEAM DETAIL "1" SCALE: N.T.S.



W6x12 CANTILEVER DETAIL "4" SCALE: N.T.S.



(TYP.) MOMENT CONNECTION SIDE VIEW "7" SCALE: N.T.S.

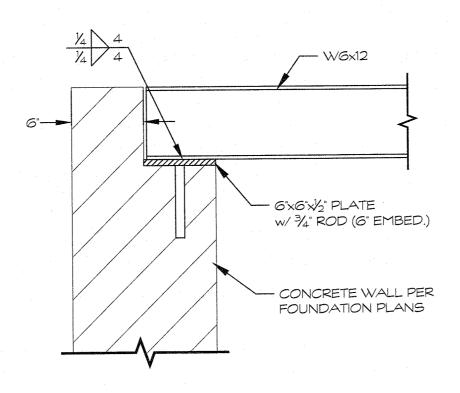


"X" BOLTS

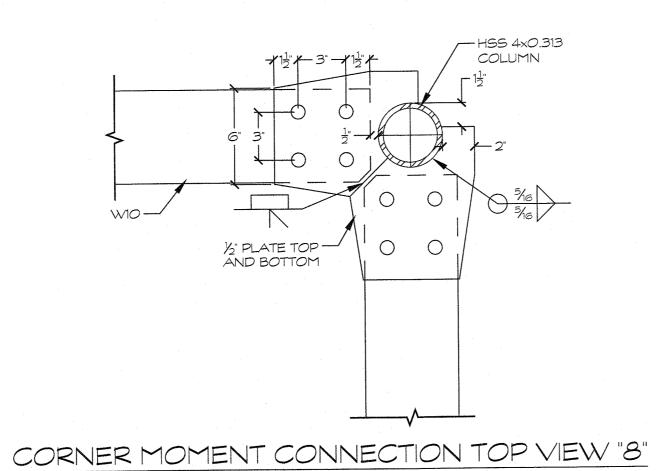
BEAM SIZE

LENGTH, L

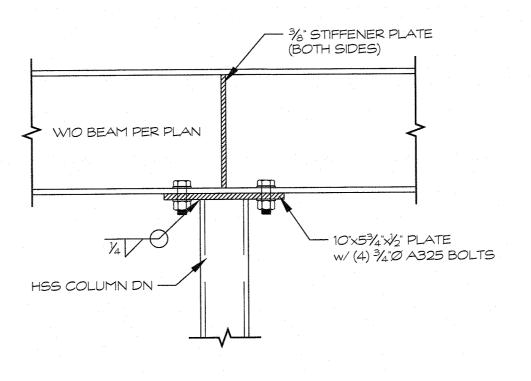
(TYP.) STEEL BEAM TO COLUMN DETAIL "2" SCALE: N.T.S.



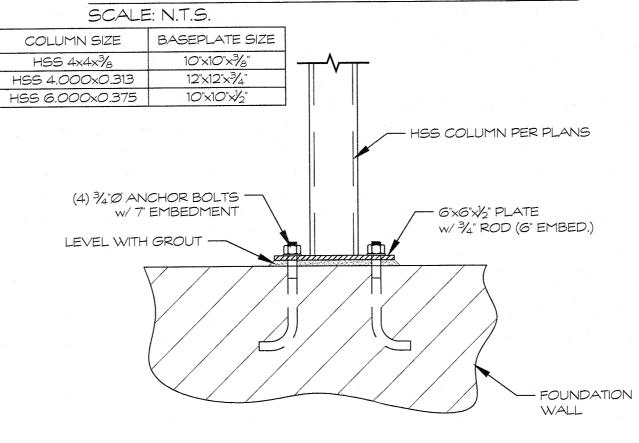
W6x12 BEARING DETAIL "5" SCALE: N.T.S.



SCALE: N.T.S.



W10 CONTINUOUS BEAM DETAIL "3"



HSS COLUMN BASEPLATE DETAIL "6" SCALE: N.T.S.

Ike Kligerman Barkley

330 West Forty-Second Street New York, NY 10036 212 268 0128

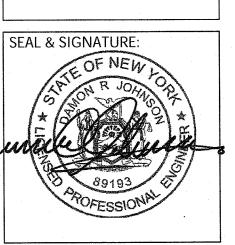
HANINGTON ENGINEERING CONSULTANTS, LLC

501 FIFTH AVE, SUITE 909 NEW YORK, NY 10017 T E L 212 338-9801 DRAWINGS@HANINGTONENGINEERING.COM JOB #21-2970

No.	REMARKS	DATE
1	ISSUED FOR REVIEW	05.06.21
2	ISSUED FOR REVIEW	05.24.21
3	ISSUED FOR REVIEW	06.24.21
4	ISSUED FOR REVIEW	08.03.21
5	ISSUED FOR FILING	08.13.21

PROJECT:

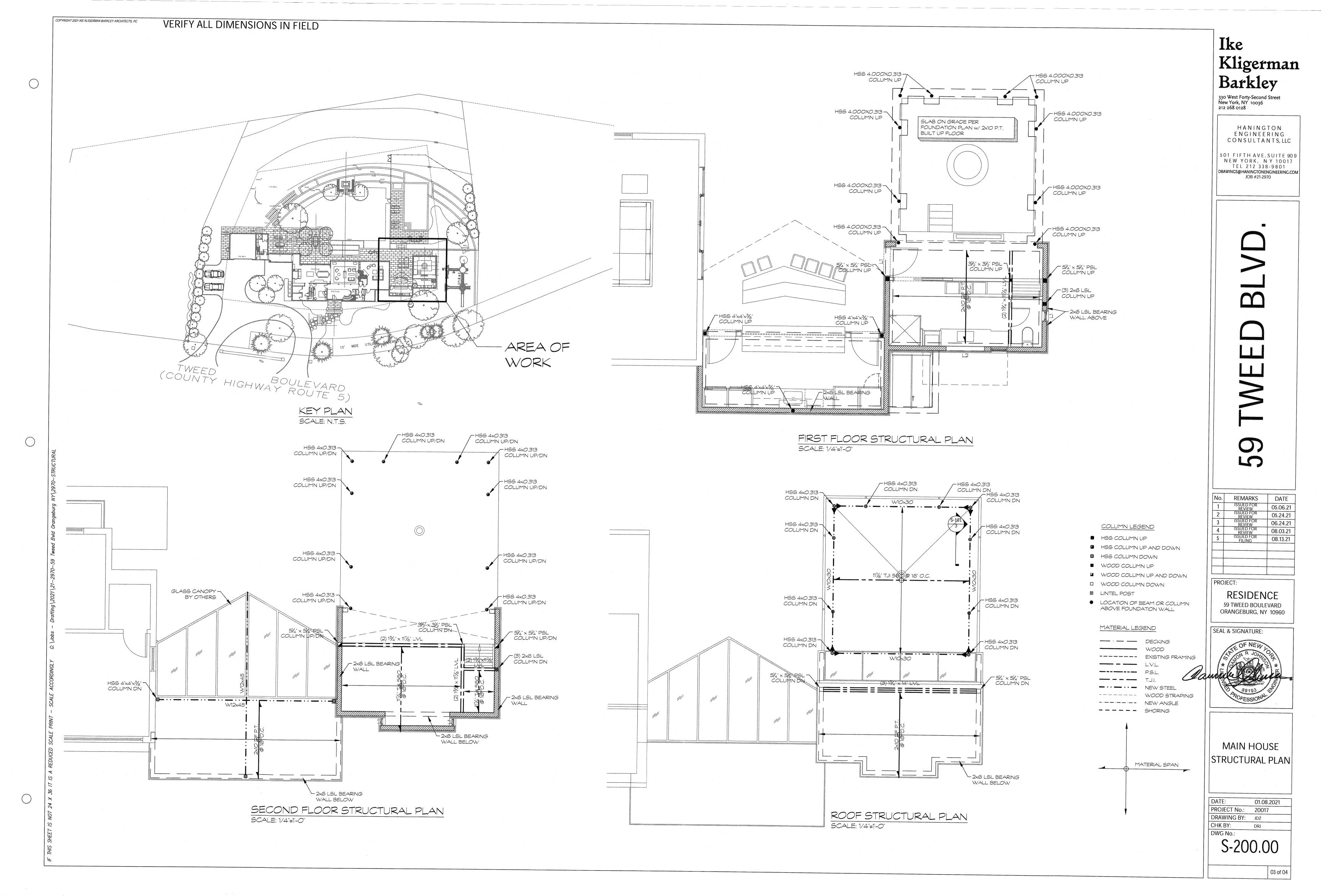
RESIDENCE 59 TWEED BOULEVARD ORANGEBURG, NY 10960

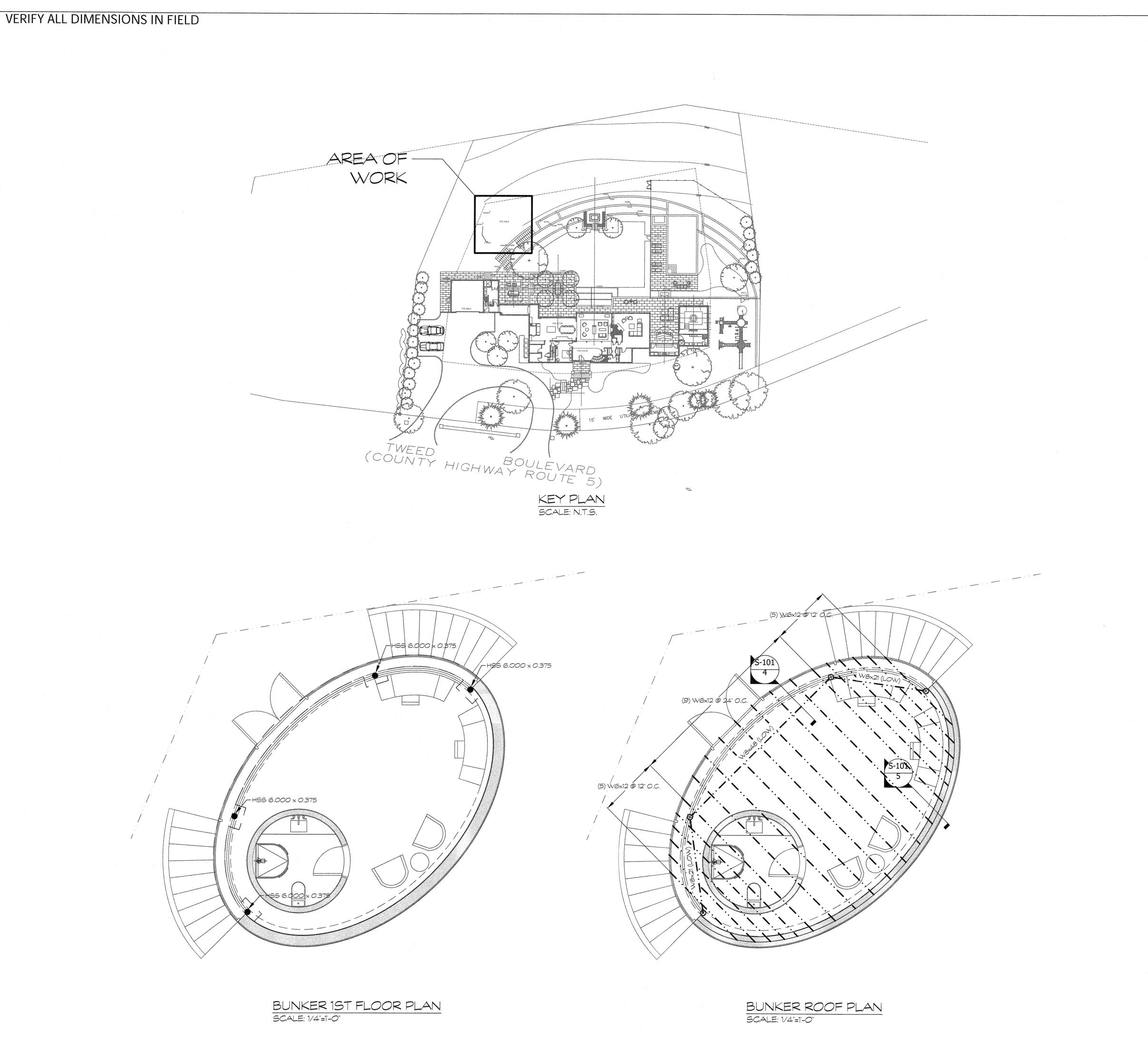


STRUCTURAL DETAILS

01.08.2021 PROJECT No.: 20017 DRAWING BY: JDZ CHK BY: DWG No.:

S-101.00





COPYRIGHT 2021 IKE KLIGERMAN BARKLEY ARCHITECTS, PC

- HSS COLUMN UP

- HSS COLUMN DOWN ■ WOOD COLUMN UP
- WOOD COLUMN UP AND DOWN
- U WOOD COLUMN DOWN
- LINTEL POST

LOCATION OF BEAM OR COLUMN ABOVE FOUNDATION WALL

MATERIAL LEGEND

- DECKING WOOD ---- EXISTING FRAMING

NEW STEEL

---- WOOD STRAPING ----- NEW ANGLE ---- SHORING

MATERIAL SPAN

COLUMN LEGEND

Kligerman Barkley HSS COLUMN UP AND DOWN

330 West Forty-Second Street New York, NY 10036 212 268 0128

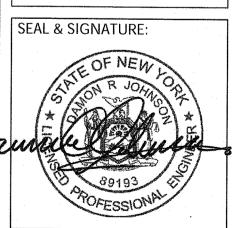
Ike

HANINGTON ENGINEERING CONSULTANTS, LLC

501 FIFTH AVE, SUITE 909 NEW YORK, NY 10017 TEL 212 338-9801 DRAWINGS@HANINGTONENGINEERING.COM JOB #21-2970

REMARKS	DATE
ISSUED FOR REVIEW	05.06.21
ISSUED FOR REVIEW	05.24.21
ISSUED FOR REVIEW	06.24.21
ISSUED FOR REVIEW	08.03.21
ISSUED FOR FILING	08.13.21

RESIDENCE 59 TWEED BOULEVARD ORANGEBURG, NY 10960



OFFICE PAVILION STRUCTURAL PLAN

DATE:	01.08.2021
PROJECT No.:	20017
DRAWING BY:	JDZ
CHK BY:	DRJ
DWG No.:	

S-201.00

PROJECT ARCHITECT: CWB ARCHITECTS

STRUCTURAL ENGINEER: DOMINICK R.PILLA ASSOCIATES PC

PROJECT ADDRESS: 59 TWEED BLVD, NYACK, NY 10960

ISSUED: FOR BUILDING PERMIT

STRUCTURAL PLANS

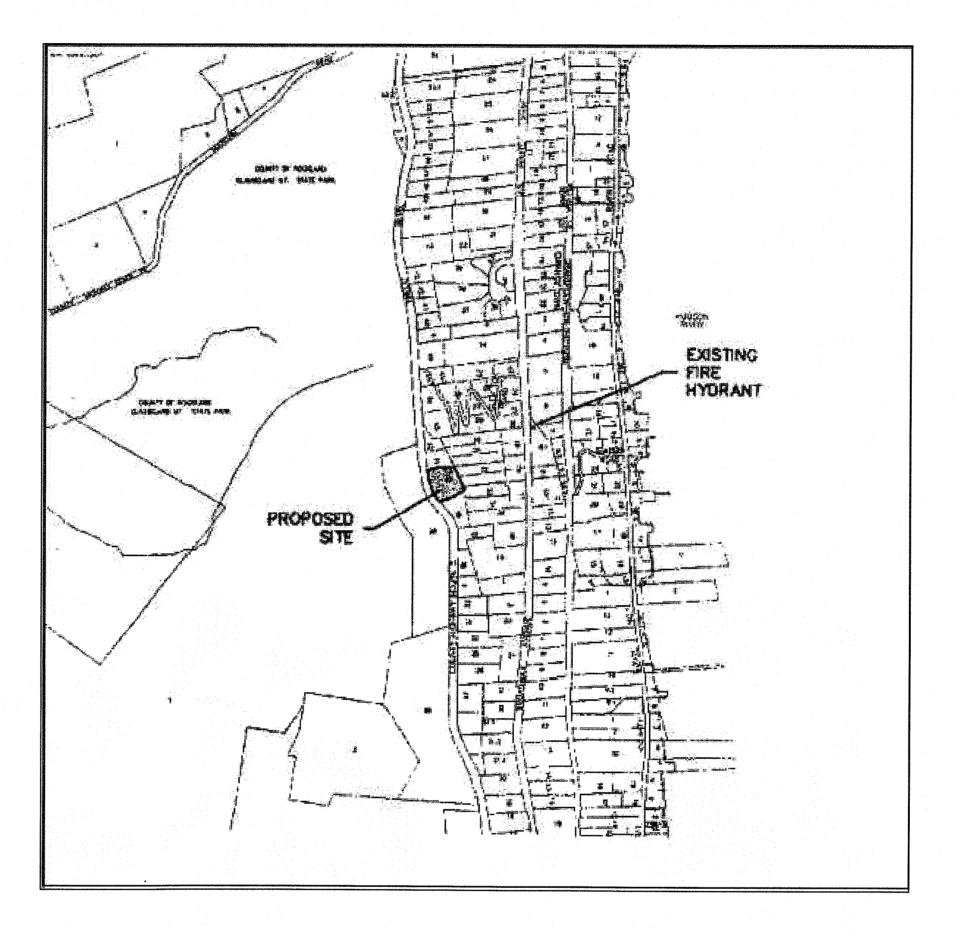
DATE: 08/17/2021

	INDEX OF DRAWINGS						
PAGE#	DWG#	REVISION #	DRAWING TITLE				
1	S-001	TITLE SHEET					
2 S-002 00		GENERAL NOTES					
3	3 S-300 00		RETAINING WALL PLAN				
4	S-400	00	RETAINING WALL SECTIONS				
5	S-410	00	RETAINING WALL DETAILS				

SYMBOL LEG	END
SECTION # XX SHT # SHEET #	SECTION SYMBOL
DETAIL # SHEET #	DETAIL REFERENCE SYMBOL
ELEVATION # SHEET #	ELEVATION REFERENCE SYMBOL
TITLE FOR PLAN, SECTION OR DETAIL # SHT # IF SHOWN POINT TO SHEET WHERE SECTION OR DETAIL WAS TAKEN	PLAN, SECTION, OR DETAIL TITLE SYMBOL
**************************************	ELEVATION SYMBOL
4 >	BEAM MOMENT CONNECTION
\$ \$	POST UP/POST DOWN
LT-# GB-# F-#.# MW-# CB-#	PRECAST LINTEL CONCRETE GRADE BEAM FOOTING NUMBER MASONRY WALL NUMBER CONCRETE BEAM CONCRETE PIER
CW-#	CONCRETE WALL NUMBER

BM BEAM BS BOTH SIDES BU BUILT UP MEMBER BW BEARING WALL C COMPRESSION FORCE IN KIPS CANT. CANTILEVER CL CENTER LINE CG CENTER OF GRAVITY COL COLUMN CONT CONTINUOUS COMP LAP COMPETE PENETRATION WELD DB DEVELOPMENT LENGTH OF REINFORCEMENT BAR DEL DELTA OR CHANGE IN ELEVATION EY EACH WAY F FINISHED SUBFACE BB GRADE BEAM H HORIZONTAL REINFORCEMENT LUBB LONG LEGS BACK-TO-BACK LIGHTWEIGHT CONCRETE M BENDING MOMENT IN FOOT-KIPS MC MOMENT CONNECTION SHOWN ON DRAWING MINIMUM N) NEW CONSTRUCTION BEARING BOLTS THREADS INCLUDED IN SHEAR PLAN NOT TO SCALE DC ON CENTER DC PLATE PP PARTIAL PENETRATION WELD PROPERTY LINE SAD SEE ARCHITECTURAL DRAWINGS/DETAILS SILS SILS SLAB ON DECK TYPE SILS SILB SILL SEED TO SEED TO SEED SEED SEED SEED SEED SEED SEED SEE	AESS	ARCHITECTURALLY EXPOSED STRUCTURAL STEEL
BS BU BUILT UP MEMBER BW BEARING WALL C COMPRESSION FORCE IN KIPS CANT. CANTILEVER CL CENTER LINE CG CENTER OF GRAVITY COL COLUMN CONT CONTINUOUS COMP LAP COMPESSION REINF LAP SLICE CP COMPLETE PENETRATION WELD DB DEVELOPMENT LENGTH OF REINFORCEMENT BAR DEL DELTA OR CHANGE IN ELEVATION EF EACH FACE EL ELEVATION EW EACH WAY F FINISHED SURFACE BB GRADE BEAM H HORIZONTAL REINFORCEMENT H HORIZONTAL REINFORCEMENT H HORIZONTAL FORCE IN KIPS 11,12 NEW CODE FORMED STEEL JOISTS AP FULL TENSION CAPACITY LAP SPLICE LD TENSION DEVELOPMENT LENGTH FOR REINFORCEMENT LIBB LONG LEGS BACK-TO-BACK LIGHTWEIGHT CONCRETE M BENDING MOMENT IN FOOT-KIPS MC MOMENT CONNECTION SHOWN ON DRAWING MIN MINIMUM N) NEW CONSTRUCTION NEW CONSTRUCT	В	
BU BUILT UP MEMBER BW BEARING WALL C COMPRESSION FORCE IN KIPS CANT. CANTILE VER CL CENTER LINE CG CENTER OF GRAVITY COL COLUMN CONT CONTINUOUS COMP LAP COMPRESSION REINF LAP SLICE CP COMPLETE PENETRATION WELD DB DEVELOPMENT LENGTH OF REINFORCEMENT BAR DEL DELTA OR CHANGE IN ELEVATION E) EXISTING CONSTRUCTION EF EACH FACE EL ELEVATION EW EACH WAY F FINISHED SURFACE GB GRADE BEAM H HORIZONTAL REINFORCEMENT H HORIZONTAL REINFORCEMENT H HORIZONTAL FORCE IN KIPS ILJ2 NEW CODE FORMED STEEL JOISTS LAP FULL TENSION DEVELOPMENT LENGTH FOR REINFORCEMENT LIDE LDC COMPRESSION SPLICE LENGTH FOR REINFORCEMENT LIBB LONG LEGS BACK-TO-BACK LW LIGHTWEIGHT CONCRETE M BENDING MOMENT IN FOOT-KIPS MC MOMENT CONNECTION SHOWN ON DRAWING MIN MINIMUM N) NEW CONSTRUCTION N BEARING BOLTS THREADS INCLUDED IN SHEAR PLAN NTS NOT TO SCALE OC ON CENTER CO PILE CAP PLATE CO PILE CAP PLATE P PARTIAL PENETRATION WELD PROPERTY LINE SIM SIMILAR SIM SIMILAR SIPW SOLDIER PILE LAGGING WALL SW SHEAR WALL TENSION FORCE IN KIPS THICKNESS TO PREINFORCEMENT TO BE CONFERNED TO FOR SIELLE TO PREINFORCEMENT TO SCALE TO PREINFORCEMENT TO SECALE TO PREINFORCEMENT TO SECALE TO PREINFORCEMENT TO SECALE TO SEE ARCHITECTURAL DRAWINGS/DETAILS SIM SIMILAR SIPW SOLDIER PILE LAGGING WALL SW SHEAR WALL TENSION FORCE IN KIPS THICKNESS TO PO F STEEL TOP FREINFORCEMENT TO BE CONFERMED TO TO SECALE TO PO F STEEL TOP OF STEEL TYP TYPICAL UNO,UON UNLESS OTHERWISE NOTED MM MOMENT VERTICAL BEAM END REACTION IN KIPS	BM	
BW BEARING WALL C COMPRESSION FORCE IN KIPS CANT. CANTILEVER CL CENTER LINE CG CENTER OF GRAVITY COL COLUMN CONT CONTINUOUS COMP LAP COMPRESSION REINF LAP SLICE CP COMPLETE PENETRATION WELD CDB DEVELOPMENT LENGTH OF REINFORCEMENT BAR CDEL DELTA OR CHANGE IN ELEVATION EP EACH FACE EL ELEVATION EW EACH WAY F FINISHED SURFACE GB GRADE BEAM H HORIZONTAL REINFORCEMENT H HORIZONTAL FORCE IN KIPS ILJ2 NEW CODE FORMED STEEL JOISTS LAP FULL TENSION CAPACITY LAP SPLICE LD TENSION DEVELOPMENT LENGTH FOR REINFORCEMENT LIBB LONG LEGS BACK-TO-BACK LW LIGHTWEIGHT CONCRETE M BENDING MOMENT IN FOOT-KIPS MC MOMENT CONNECTION SHOWN ON DRAWING MIN MINIMUM N) NEW CONSTRUCTION N BEARING BOLTS THREADS INCLUDED IN SHEAR PLAN. NOT TO SCALE OC ON CENTER CC PILE CAP PLATE PP PARTIAL PENETRATION WELD L PROPERTY LINE SACH SHOW SHEAR SHEAR WALL TENSION FORCE IN KIPS THICKNESS TOP REINFORCEMENT THICKNESS TOP OF STEEL TYP TYPICAL INNO,UON UNLESS OTHERWISE NOTED M MOMENT V VERTICAL BEAM END REACTION IN KIPS	BS	
COMPRESSION FORCE IN KIPS CANT. CANTILEVER CL CENTER LINE CG CENTER OF GRAVITY COL COLUMN CONT CONTINUOUS COMP LAP COMPRESSION REINF LAP SLICE CP COMPLETE PENETRATION WELD DB DEVELOPMENT LENGTH OF REINFORCEMENT BAR DEL DELTA OR CHANGE IN ELEVATION EF EACH FACE EL ELEVATION EF EACH WAY FINISHED SURFACE GB GRADE BEAM HORIZONTAL REINFORCEMENT H HORIZONTAL REINFORCEMENT H HORIZONTAL FORCE IN KIPS MILJ2 NEW CODE FORMED STEEL JOISTS LAP FULL TENSION CAPACITY LAP SPLICE COMPRESSION SPLICE LENGTH FOR REINFORCEMENT LIBB LONG LEGS BACK-TO-BACK LIGHTWEIGHT CONCRETE M BENDING MOMENT IN FOOT-KIPS MC MOMENT CONNECTION SHOWN ON DRAWING MIN MINIMUM N) NEW CONSTRUCTION N BEARING BOLTS THREADS INCLUDED IN SHEAR PLAN NTS NOT TO SCALE OC ON CENTER DC PILE CAP PL PARTIAL PENETRATION WELD PROPERTY LINE SAD SEE ARCHITECTURAL DRAWINGS/DETAILS SILIS SLAB ON DECK TYPE SIM SIMILAR SPW SOLDIER PILE LAGGING WALL SW SHEAR WALL TENSION FORCE IN KIPS THICKNESS TOP REINFORCEMENT TOP REINFORCEMENT TOP GENEROCEMENT TOP GENEROREMENT TOP GENEROREMENT TOP GENEROREMENT TOP OF CONCRETE TOP OF OF OFTE OFTE TOP OF STEEL TYP TYPICAL UNOLUN UNLESS OTHERWISE NOTED M MOMENT V VERTICAL BEAM END REACTION IN KIPS VIF VERTICAL BEAM END REACTION IN KIPS		
CANT. CANTILEVER CL CENTER LINE CG CENTER OF GRAVITY COL COLUMN CONT CONTINUOUS COMPLETE OF GRAVITY COLUMN CONT COMPRESSION REINF LAP SLICE CP COMPRESSION REINF LAP SLICE CP COMPLETE PENETRATION WELD DB DEVELOPMENT LENGTH OF REINFORCEMENT BAR DEL DELTA OR CHANGE IN ELEVATION E) EXISTING CONSTRUCTION EF EACH FACE EL ELEVATION EW EACH WAY FINISHED SURFACE GB GRADE BEAM H HORIZONTAL FEINFORCEMENT H HORIZONTAL FORCE IN KIPS HIJL TENSION CAPACITY LAP SPLICE LD TENSION DEVELOPMENT LENGTH FOR REINFORCEMENT HORIZONTAL FORCE IN SPLICE LD TENSION SPLICE LENGTH FOR REINFORCEMENT LIBB LONG LEGS BACK-TO-BACK LIGHTWEIGHT CONCRETE M BENDING MOMENT IN FOOT-KIPS MC MOMENT CONNECTION SHOWN ON DRAWING MIN MINIMUM N) NEW CONSTRUCTION N BEARING BOLTS THREADS INCLUDED IN SHEAR PLAN NTS NOT TO SCALE OC ON CENTER CC PILE CAP PL PLATE PP PARTIAL PENETRATION WELD PL ATE DC ON CENTER CC PILE CAP PL PLATE PP PARTIAL PENETRATION WELD PL PROPERTY LINE SAD SEE ARCHITECTURAL DRAWINGS/DETAILS SI,S2 SLAB ON DECK TYPE SC SLIP CRITICAL BOLT SIMM SIMILAR SIM		
CL CENTER LINE CG CENTER OF GRAVITY COL COLUMN CONT CONTINUOUS COMP LAP COMPRESSION REINF LAP SLICE CP COMPLETE PENETRATION WELD DB DEVELOPMENT LENGTH OF REINFORCEMENT BAR DELL DELTA OR CHANGE IN ELEVATION EF EACH FACE EL ELEVATION EW EACH WAY FINISHED SURFACE GB GRADE BEAM H HORIZONTAL REINFORCEMENT H HORIZONTAL FORCE IN KIPS HIJZ NEW CODE FORMED STEEL JOISTS LAP FULL TENSION CAPACITY LAP SPLICE LD TENSION DEVELOPMENT LENGTH FOR REINFORCEMENT LUBB LONG LEGS BACK-TO-BACK LW LIGHTWEIGHT CONCRETE M BENDING MOMENT IN FOOT-KIPS MC MOMENT CONNECTION SHOWN ON DRAWING MIN MINIMUM N) NEW CONSTRUCTION N BEARING BOLTS THREADS INCLUDED IN SHEAR PLAN NOT TO SCALE OC ON CENTER PL PLATE PP PARTIAL PENETRATION WELD PROPERTY LINE SAD SEE ARCHITECTURAL DRAWINGS/DETAILS SILSS SILSS SLAB ON DECK TYPE SIC SLIP CRITICAL BOLT SIM SIMILAR SPW SOLDIER PILE LAGGING WALL SW SHEAR WALL T TENSION FORCE IN KIPS THICKNESS T TOP REINFORCEMENT TOP OF CONCRETE TOP OF FOOTING TOP OF STEEL TYP TYPICAL JINO,UON UNLESS OTHERWISE NOTED M MOMENT V VERTICAL BEAM END REACTION IN KIPS VERTIVAL BEAM END REACTION IN KIPS VERTIVAL BEAM END REACTION IN KIPS VERTIVAL BEAM END REACTION IN KIPS VERTICAL BEAM END REACTION IN KIPS		
CG CENTER OF GRAVITY COLL COLUMN CONT CONTINUOUS COMP LAP COMPRESSION REINF LAP SLICE CP COMPLETE PENETRATION WELD DB DEVELOPMENT LENGTH OF REINFORCEMENT BAR DEL DELTA OR CHANGE IN ELEVATION E) EXISTING CONSTRUCTION EF EACH FACE EL ELEVATION EW EACH WAY F FINISHED SURFACE GB GRADE BEAM H HORIZONTAL FORCE IN KIPS II,12 NEW CODE FORMED STEEL JOISTS LAP FULL TENSION CAPACITY LAP SPLICE DE TENSION DEVELOPMENT LENGTH FOR REINFORCEMENT LIBB LONG LEGS BACK-TO-BACK LIGHTWEIGHT CONCRETE M BENDING MOMENT IN FOOT-KIPS MC MOMENT CONNECTION SHOWN ON DRAWING MIN MINIMUM N) NEW CONSTRUCTION N BEARING BOLTS THREADS INCLUDED IN SHEAR PLAN NOT TO SCALE OC ON CENTER PL PARTIAL PENETRATION WELD PROPERTY LINE SIAD SEE ARCHITECTURAL DRAWINGS/DETAILS SIAS2 SLAB ON DECK TYPE SIAD SIAD SEE ARCHITECTURAL DRAWINGS/DETAILS SIAS2 SLAB ON DECK TYPE SIAD SEE ARCHITECTURAL DRAWINGS/DETAILS SIAS2 SLAB ON DECK TYPE SIAD SEE ARCHITECTURAL DRAWINGS/DETAILS SIAS2 SLAB ON DECK TYPE SIAD SEE ARCHITECTURAL DRAWINGS/DETAILS SIAS2 SLAB ON DECK TYPE SIAD SEE ARCHITECTURAL DRAWINGS/DETAILS SIAS2 SLAB ON DECK TYPE SIAD SEE ARCHITECTURAL DRAWINGS/DETAILS SIAS2 SLAB ON DECK TYPE SIAD SEE ARCHITECTURAL DRAWINGS/DETAILS SIAS2 SLAB ON DECK TYPE SIAD SEE ARCHITECTURAL DRAWINGS/DETAILS SIAS2 SLAB ON DECK TYPE SIAD SEE ARCHITECTURAL DRAWINGS/DETAILS SIAS2 SLAB ON DECK TYPE SIAD SEE ARCHITECTURAL DRAWINGS/DETAILS SIAS2 SLAB ON DECK TYPE SIAD SEE ARCHITECTURAL DRAWINGS/DETAILS SIAS2 SLAB ON DECK TYPE SIAD SEE ARCHITECTURAL DRAWINGS/DETAILS SIAS2 SLAB ON DECK TYPE SIAD SEE ARCHITECTURAL DRAWINGS/DETAILS SIAS2 SLAB ON DECK TYPE SIAD SEE ARCHITECTURAL DRAWINGS/DETAILS SIAS2 SLAB ON DECK TYPE SIAD SEE ARCHITECTURAL DRAWINGS/DETAILS SIAS2 SLAB ON DECK TYPE SIAD SEE ARCHITECTURAL DRAWINGS/DETAILS SIAS2 SLAB ON DECK TYPE SIAD SEE ARCHITECTURAL DRAWINGS/DETAILS SIAS2 SLAB ON DECK TYPE SIAD SEE ARCHITECTURAL DRAWINGS/DETAILS SIAS2 SLAB ON DECK TYPE SIAD SEE ARCHITECTURAL DRAWINGS/DETAILS SIAD SEE ARCHITECTURAL DRAWINGS/DETAILS SIAD SEE ARCHITECTURAL DRAWINGS/DETAILS SIAD SE		
COL COLUMN CONT CONTINUOUS COMPLAP COMPRESSION REINF LAP SLICE CP COMPRESSION SERVACE EL ELEVATION EW EACH WAY F FINISHED SURFACE GB GRADE BEAM H HORIZONTAL REINFORCEMENT H HORIZONTAL FORCE IN KIPS II, 12 NEW CODE FORMED STEEL JOISTS LAP FULL TENSION CAPACITY LAP SPLICE LD TENSION DEVELOPMENT LENGTH FOR REINFORCING I COMPRESSION SPLICE LENGTH FOR REINFORCEMENT LIBB LONG LEGS BACK-TO-BACK LIGHTWEIGHT CONCRETE M BENDING MOMENT IN FOOT-KIPS MC MOMENT CONNECTION SHOWN ON DRAWING MIN MINIMUM N) NEW CONSTRUCTION N BEARING BOLTS THREADS INCLUDED IN SHEAR PLAN NTS NOT TO SCALE CO ON CENTER CP PLATE CP PLATE CP PLATE CP PARTIAL PENETRATION WELD CO ON CENTER CP PLATE CP PARTIAL PENETRATION WELD CO ON CENTER CP PLATE CP PARTIAL PENETRATION WELD CO ON CENTER CP PILE CAP CP PLATE CP PARTIAL PENETRATION WELD CO ON CENTER CP PILE CAP CP PLATE CP PROPERTY LINE CS SLIP CRITICAL BOLT SIM SIMILAR CS SLIP CRITICAL BOLT SIM SIMILAR CS SLIP CRITICAL BOLT SIM SIMILAR CS SLIP CRITICAL BOLT CS TOP OF CONCRETE CO OF OF OF OF OTONING COS TOP OF FOOTING COS TOP OF FOOTING COS TOP OF STEEL CS SLIP CRITICAL BOLM EACTION IN KIPS CYP TYPICAL UNDUSTRICTION OF EACTION IN KIPS CYP TYPICAL UNDUSTRICTION COMPRESSION TENTER CS STRUCTOR CS TOTOR		
COMP LAP COMPRESSION REINF LAP SLICE COMPLETE PENETRATION WELD DB DEVELOPMENT LENGTH OF REINFORCEMENT BAR DEL DELTA OR CHANGE IN ELEVATION E) EXISTING CONSTRUCTION EF EACH FACE EL ELEVATION EW EACH WAY F FINISHED SURFACE GB GRADE BEAM H HORIZONTAL REINFORCEMENT H HORIZONTAL REINFORCEMENT H HORIZONTAL FORCE IN KIPS II, J2 NEW CODE FORMED STEEL JOISTS LAP FULL TENSION CAPACITY LAP SPLICE LD TENSION DEVELOPMENT LENGTH FOR REINFORCEMENT LOC COMPRESSION SPLICE LENGTH FOR REINFORCEMENT LOC LOB LOG LEGS BACK-TO-BACK LIGHTWEIGHT CONCRETE M BENDING MOMENT IN FOOT-KIPS MC MOMENT CONNECTION SHOWN ON DRAWING MINIMUM N) NEW CONSTRUCTION N BEARING BOLTS THREADS INCLUDED IN SHEAR PLAN NTS NOT TO SCALE OC ON CENTER DC DC ON CENTER DC P P PARTIAL PENETRATION WELD PROPERTY LINE SAD SEE ARCHITECTURAL DRAWINGS/DETAILS SILS2 SLAB ON DECK TYPE SIC SLIP CRITICAL BOLT SIMILAR SPW SOLDIER PILE LAGGING WALL SIMILAR SPW SOLDIER PILE SPW SOLDIER PILE SPW SOLDIER PILE SPW SOLDIER PILE SPW SOLDIER	COL	
COMPLETE PENETRATION WELD DEVELOPMENT LENGTH OF REINFORCEMENT BAR DEVELOPMENT BEVATION EF EXISTING CONSTRUCTION EF EACH FACE EL ELEVATION EW EACH WAY F FINISHED SURFACE BB GRADE BEAM H HORIZONTAL REINFORCEMENT H HORIZONTAL FORCE IN KIPS HIJZ NEW CODE FORMED STEEL JOISTS APP FULL TENSION CAPACITY LAP SPLICE LD TENSION DEVELOPMENT LENGTH FOR REINFORCEMENT LOB LONG LEGS BACK-TO-BACK LUGHTWEIGHT CONCRETE M BENDING MOMENT IN FOOT-KIPS MC MOMENT CONNECTION SHOWN ON DRAWING MIN MINIMUM N) NEW CONSTRUCTION N BEARING BOLTS THREADS INCLUDED IN SHEAR PLAN NOT TO SCALE DC ON CENTER DC PILE CAP PLATE PP PARTIAL PENETRATION WELD PL PLATE PP PARTIAL PENETRATION WELD PL PROPERTY LINE SAD SEE ARCHITECTURAL DRAWINGS/DETAILS SIJS2 SLAB ON DECK TYPE SIM SIMILAR SPW SOLDIER PILE LAGGING WALL SW SHEAR WALL T TENSION FORCE IN KIPS TOP REINFORCEMENT TOO SCOLEMENT TOO SCOLEMENT TOO SECONTING TOO STEEL TYP TYPICAL JUNCJUON UNLESS OTHERWISE NOTED M MOMENT V VERTICAL BEAM END REACTION IN KIPS VERTIY IN FIELD WP WORKPOINT	CONT	CONTINUOUS
DELL DELTA OR CHANGE IN ELEVATION E) EXISTING CONSTRUCTION EF EACH FACE EL ELEVATION EW EACH WAY F FINISHED SURFACE GB GRADE BEAM H HORIZONTAL REINFORCEMENT H HORIZONTAL FORCE IN KIPS II, J2 NEW CODE FORMED STEEL JOISTS AP FULL TENSION CAPACITY LAP SPLICE LD TENSION DEVELOPMENT LENGTH FOR REINFORCEMENT LLBB LONG LEGS BACK-TO-BACK LW LIGHTWEIGHT CONCRETE M BENDING MOMENT IN FOOT-KIPS MC MOMENT CONNECTION SHOWN ON DRAWING MINIMUM N) NEW CONSTRUCTION N BEARING BOLTS THREADS INCLUDED IN SHEAR PLAN NTS NOT TO SCALE ON CENTER PC PILE CAP PL PLATE PP PARTIAL PENETRATION WELD PROPERTY LINE SEAD SEE ARCHITECTURAL DRAWINGS/DETAILS SI,S2 SLAB ON DECK TYPE SIAD SEE ARCHITECTURAL DRAWINGS/DETAILS SIMILAR SPW SOLDIER PILE LAGGING WALL SHEAR WALL TENSION FORCE IN KIPS THICKNESS TOP REINFORCEMENT TOP REINFORCEMENT TOP GEONFIRMED TOP OF FOOTING TOS TOP OF STEEL TYP TYPICAL JNO,UON UNLESS OTHERWISE NOTED M MOMENT V VERTICAL BEAM END REACTION IN KIPS VERTIV Y I FIELD WP WORKPOINT		
DELL DELTA OR CHANGE IN ELEVATION E) EXISTING CONSTRUCTION EF EACH FACE EL ELEVATION EW EACH WAY F FINISHED SURFACE GB GRADE BEAM H HORIZONTAL REINFORCEMENT H HORIZONTAL REINFORCEMENT H HORIZONTAL FORCE IN KIPS II, J2 NEW CODE FORMED STEEL JOISTS LAP FULL TENSION CAPACITY LAP SPLICE LD TENSION DEVELOPMENT LENGTH FOR REINFORCEMENT LDC COMPRESSION SPLICE LENGTH FOR REINFORCEMENT LDD GLEGS BACK-TO-BACK LW LIGHTWEIGHT CONCRETE M BENDING MOMENT IN FOOT-KIPS MC MOMENT CONNECTION SHOWN ON DRAWING MIN MINIMUM N) NEW CONSTRUCTION N BEARING BOLTS THREADS INCLUDED IN SHEAR PLAN. NOT TO SCALE DC ON CENTER PO PLATE PP PARTIAL PENETRATION WELD PLATE PP PARTIAL PENETRATION WELD PLATE PP PARTIAL PENETRATION WELD SIMS SEE ARCHITECTURAL DRAWINGS/DETAILS SINS2 SLAB ON DECK TYPE SC SLIP CRITICAL BOLT SIM SIMILAR SIPW SOLDIER PILE LAGGING WALL STENSION FORCE IN KIPS THICKNESS THICKNESS TOP OF CONCRETE TOP REINFORCEMENT TOP REINFORCEMENT TOP REINFORCEMENT TOP OF FOOTING TOS TOP OF FOOTING TOS TOP OF FOOTING TOS TOP OF STEEL TYP TYPICAL JUNC, UON UNLESS OTHERWISE NOTED M MOMENT V VERTICAL BEAM END REACTION IN KIPS VERTIY IN FIELD WP WORKPOINT	CP	
E) EXISTING CONSTRUCTION EF EACH FACE EL ELEVATION EW EACH WAY F FINISHED SURFACE GB GRADE BEAM GH HORIZONTAL REINFORCEMENT H HORIZONTAL FORCE IN KIPS II,J2 NEW CODE FORMED STEEL JOISTS LAP FULL TENSION CAPACITY LAP SPLICE LD TENSION DEVELOPMENT LENGTH FOR REINFORCEMENT LDC COMPRESSION SPLICE LENGTH FOR REINFORCEMENT LLBB LONG LEGS BACK-TO-BACK LIGHTWEIGHT CONCRETE M BENDING MOMENT IN FOOT-KIPS MC MOMENT CONNECTION SHOWN ON DRAWING MIN MINIMUM N) NEW CONSTRUCTION N BEARING BOLTS THREADS INCLUDED IN SHEAR PLAN NOT TO SCALE OC ON CENTER PL PLATE PP PARTIAL PENETRATION WELD PROPERTY LINE SAD SEE ARCHITECTURAL DRAWINGS/DETAILS SI,S2 SLAB ON DECK TYPE SIM SIMILAR SPW SOLDIER PILE LAGGING WALL SIMILAR SPW SOLDIER PILE LAGGING WALL SHEAR WALL TENSION FORCE IN KIPS THICKNESS TOP REINFORCEMENT TOB CONFIRMED TOO FO FOOTING TOO FO FOOTING TOO FO FOOTING TOO FO FOOTING TOO FOO		
EF EACH FACE EL ELEVATION EW EACH WAY F FINISHED SURFACE GB GRADE BEAM H HORIZONTAL REINFORCEMENT H HORIZONTAL FORCE IN KIPS II,J2 NEW CODE FORMED STEEL JOISTS LAP FULL TENSION CAPACITY LAP SPLICE LD TENSION DEVELOPMENT LENGTH FOR REINFORCEMENT LBB LONG LEGS BACK-TO-BACK LW LIGHTWEIGHT CONCRETE M BENDING MOMENT IN FOOT-KIPS MC MOMENT CONNECTION SHOWN ON DRAWING MIN MINIMUM N) NEW CONSTRUCTION N BEARING BOLTS THREADS INCLUDED IN SHEAR PLAN NTS NOT TO SCALE DC ON CENTER DC PILE CAP PLATE PP PARTIAL PENETRATION WELD PROPERTY LINE SAD SEE ARCHITECTURAL DRAWINGS/DETAILS SILS2 SLAB ON DECK TYPE SIM SIMILAR SPW SOLDIER PILE LAGGING WALL SW SHEAR WALL T TENSION FORCE IN KIPS THICKNESS T TOP REINFORCEMENT TOD SE CONFIRMED TOO TOP OF FOOTING TOO STEEL TYP TYPICAL JNO,UON UNLESS OTHERWISE NOTED MM MOMENT VERTICAL BEAM END REACTION IN KIPS VIFF VERIFY IN FIELD WP WORKPOINT		
EL ELEVATION EW EACH WAY F FINISHED SURFACE GB GRADE BEAM H HORIZONTAL REINFORCEMENT H HORIZONTAL FORCE IN KIPS II,12 NEW CODE FORMED STEEL JOISTS LAP FULL TENSION CAPACITY LAP SPLICE LD TENSION DEVELOPMENT LENGTH FOR REINFORCING I LDC COMPRESSION SPLICE LENGTH FOR REINFORCEMENT LUB LONG LEGS BACK-TO-BACK LW LIGHTWEIGHT CONCRETE M BENDING MOMENT IN FOOT-KIPS MC MOMENT CONNECTION SHOWN ON DRAWING MIN MINIMUM N) NEW CONSTRUCTION N BEARING BOLTS THREADS INCLUDED IN SHEAR PLAN NOT TO SCALE DC ON CENTER DC PILE CAP PL PLATE PP PARTIAL PENETRATION WELD PL PROPERTY LINE SAD SEE ARCHITECTURAL DRAWINGS/DETAILS SI,S2 SLAB ON DECK TYPE SIM SIMILAR SPW SOLDIER PILE LAGGING WALL SW SHEAR WALL TENSION FORCE IN KIPS THICKNESS TOP REINFORCEMENT TOP BE CONFIRMED TOP OF FOOTING TOS TOP OF STEEL TYP TYPICAL JNO,UON UNLESS OTHERWISE NOTED M MOMENT VERTICAL BEAM END REACTION IN KIPS VIF VERIFY IN FIELD WP WORKPOINT		
EW EACH WAY F FINISHED SURFACE GB GRADE BEAM H HORIZONTAL REINFORCEMENT H HORIZONTAL FORCE IN KIPS II,J2 NEW CODE FORMED STEEL JOISTS LAP FULL TENSION CAPACITY LAP SPLICE LD TENSION DEVELOPMENT LENGTH FOR REINFORCEMENT LD TENSION DEVELOPMENT LENGTH FOR REINFORCEMENT LD TENSION DEVELOPMENT LENGTH FOR REINFORCEMENT LBB LONG LEGS BACK-TO-BACK LW LIGHTWEIGHT CONCRETE M BENDING MOMENT IN FOOT-KIPS MC MOMENT CONNECTION SHOWN ON DRAWING MIN MINIMUM N) NEW CONSTRUCTION N BEARING BOLTS THREADS INCLUDED IN SHEAR PLAN NTS NOT TO SCALE DC ON CENTER PC PILE CAP PL PLATE PP PARTIAL PENETRATION WELD PL PLATE PP PARTIAL PENETRATION WELD PL PROPERTY LINE SAD SEE ARCHITECTURAL DRAWINGS/DETAILS SILS2 SLAB ON DECK TYPE SIM SIMILAR SPW SOLDIER PILE LAGGING WALL SW SHEAR WALL TENSION FORCE IN KIPS THICKNESS TOP REINFORCEMENT TOBE CONFIRMED TOO TOP OF CONCRETE TOP OF FOOTING TOS TOP OF STEEL TYP TYPICAL JNO,UON UNLESS OTHERWISE NOTED M MOMENT VERTICAL BEAM END REACTION IN KIPS VIF VERIFY IN FIELD WP WORKPOINT		
GB GRADE BEAM H HORIZONTAL REINFORCEMENT H HORIZONTAL FORCE IN KIPS II,J2 NEW CODE FORMED STEEL JOISTS LAP FULL TENSION CAPACITY LAP SPLICE LD TENSION DEVELOPMENT LENGTH FOR REINFORCING IN LDC COMPRESSION SPLICE LENGTH FOR REINFORCEMENT LDB LONG LEGS BACK-TO-BACK LW LIGHTWEIGHT CONCRETE M BENDING MOMENT IN FOOT-KIPS MC MOMENT CONNECTION SHOWN ON DRAWING MIN MINIMUM N) NEW CONSTRUCTION N BEARING BOLTS THREADS INCLUDED IN SHEAR PLAN NOT TO SCALE OC ON CENTER DC ON CENTER DC PL PLATE PP PARTIAL PENETRATION WELD PROPERTY LINE SAD SEE ARCHITECTURAL DRAWINGS/DETAILS SI,S2 SLAB ON DECK TYPE SC SLIP CRITICAL BOLT SIM SIMILAR SPW SOLDIER PILE LAGGING WALL ST TENSION FORCE IN KIPS THICKNESS TO PREINFORCEMENT TO BE CONFIRMED TOO TO PO F CONCRETE TOP OF FOOTING TOS TOP OF STEEL TYP TYPICAL JNO,UON UNLESS OTHERWISE NOTED MM MOMENT VERTIFY IN FIELD WP WORKPOINT	EW	
H HORIZONTAL REINFORCEMENT H HORIZONTAL FORCE IN KIPS NEW CODE FORMED STEEL JOISTS LAP FULL TENSION CAPACITY LAP SPLICE LD TENSION DEVELOPMENT LENGTH FOR REINFORCEMENT LDC COMPRESSION SPLICE LENGTH FOR REINFORCEMENT LLBB LONG LEGS BACK-TO-BACK LW LIGHTWEIGHT CONCRETE M BENDING MOMENT IN FOOT-KIPS MC MOMENT CONNECTION SHOWN ON DRAWING MIN MINIMUM N) NEW CONSTRUCTION N BEARING BOLTS THREADS INCLUDED IN SHEAR PLAN NTS NOT TO SCALE DC ON CENTER DC ON CENTER DL PLATE PP PARTIAL PENETRATION WELD PL PROPERTY LINE SAD SEE ARCHITECTURAL DRAWINGS/DETAILS SI,52 SLAB ON DECK TYPE SIM SIMILAR SPW SOLDIER PILE LAGGING WALL SIMILAR SPW SOLDIER PILE LAGGING WALL SIMILAR SPW SOLDIER PILE LAGGING WALL ST TENSION FORCEMENT TOP REINFORCEMENT TOP REINFORCEMENT TOP REINFORCEMENT TOP OF FOOTING TOP OF FOOTING TOP OF FOOTING TOP OF STEEL TYP TYPICAL JNO,UON UNLESS OTHERWISE NOTED MM MOMENT VERTIFY IN FIELD WP WORKPOINT	F	FINISHED SURFACE
H HORIZONTAL FORCE IN KIPS 11,12 NEW CODE FORMED STEEL JOISTS LAP FULL TENSION CAPACITY LAP SPLICE LD TENSION DEVELOPMENT LENGTH FOR REINFORCING I LDC COMPRESSION SPLICE LENGTH FOR REINFORCEMENT LLBB LONG LEGS BACK-TO-BACK LUW LIGHTWEIGHT CONCRETE MENDING MOMENT IN FOOT-KIPS MC MOMENT CONNECTION SHOWN ON DRAWING MIN MINIMUM N) NEW CONSTRUCTION N BEARING BOLTS THREADS INCLUDED IN SHEAR PLAN NOT TO SCALE DC ON CENTER DC PILE CAP PL PLATE PP PARTIAL PENETRATION WELD PP PARTIAL PENETRATION WELD PP PARTIAL PENETRATION WELD SI,S2 SLAB ON DECK TYPE SIC SLIP CRITICAL BOLT SIM SIMILAR SPW SOLDIER PILE LAGGING WALL SW SHEAR WALL TENSION FORCE IN KIPS THICKNESS TO TOP REINFORCEMENT TOBE CONFIRMED TOD GE CONFIRMED TOD GE CONFIRMED TOD FOOTING TOS STEEL TYP TYPICAL JNO,UON UNLESS OTHERWISE NOTED MM MOMENT VERTIFY IN FIELD WP WORKPOINT	GB	
NEW CODE FORMED STEEL JOISTS LAP FULL TENSION CAPACITY LAP SPLICE TENSION DEVELOPMENT LENGTH FOR REINFORCING I COMPRESSION SPLICE LENGTH FOR REINFORCEMENT LEBB LONG LEGS BACK-TO-BACK LW LIGHTWEIGHT CONCRETE M BENDING MOMENT IN FOOT-KIPS MC MOMENT CONNECTION SHOWN ON DRAWING MIN MINIMUM N) NEW CONSTRUCTION N BEARING BOLTS THREADS INCLUDED IN SHEAR PLAN NTS NOT TO SCALE OC ON CENTER PC PILE CAP PL PLATE PP PARTIAL PENETRATION WELD PL PROPERTY LINE SAD SEE ARCHITECTURAL DRAWINGS/DETAILS SI,S2 SLAB ON DECK TYPE SIG SLIP CRITICAL BOLT SIM SIMILAR SPW SOLDIER PILE LAGGING WALL SW SHEAR WALL TENSION FORCE IN KIPS THICKNESS TOP REINFORCEMENT TO BE CONFIRMED TOP OF CONCRETE TOP OF FOOTING TOS TOP OF STEEL TYP TYPICAL JNO,UON UNLESS OTHERWISE NOTED M MOMENT V VERTICAL BEAM END REACTION IN KIPS VIF VERIFY IN FIELD WP WORKPOINT	H	
FULL TENSION CAPACITY LAP SPLICE TENSION DEVELOPMENT LENGTH FOR REINFORCING IN COMPRESSION SPLICE LENGTH FOR REINFORCEMENT LUBB LONG LEGS BACK-TO-BACK LW LIGHTWEIGHT CONCRETE M BENDING MOMENT IN FOOT-KIPS MC MOMENT CONNECTION SHOWN ON DRAWING MINIM MINIMUM N) NEW CONSTRUCTION N BEARING BOLTS THREADS INCLUDED IN SHEAR PLAN. NOT TO SCALE OC ON CENTER PL PLATE PARTIAL PENETRATION WELD PROPERTY LINE SAD SEE ARCHITECTURAL DRAWINGS/DETAILS SI,S2 SLAB ON DECK TYPE SC SLIP CRITICAL BOLT SIMI SIMILAR SPW SOLDIER PILE LAGGING WALL SW SHEAR WALL TENSION FORCE IN KIPS TO TO PEINFORCEMENT TO BE CONFIRMED TOO TO FOOTING TOO TOP OF FOOTING TOS TOP OF STEEL TYP TYPICAL JNO,UON UNLESS OTHERWISE NOTED M MOMENT VERTIFY IN FIELD WO WORKPOINT		
TENSION DEVELOPMENT LENGTH FOR REINFORCING IN COMPRESSION SPLICE LENGTH FOR REINFORCEMENT LENB LONG LEGS BACK-TO-BACK LW LIGHTWEIGHT CONCRETE MEDITING MOMENT IN FOOT-KIPS MC MOMENT CONNECTION SHOWN ON DRAWING MIN MINIMUM NOW NEW CONSTRUCTION SHOWN ON DRAWING MIN NOT TO SCALE OC ON CENTER PLATE PLATE PLATE PROPERTY LINE SAD SEE ARCHITECTURAL DRAWINGS/DETAILS SI,S2 SLAB ON DECK TYPE SC SLIP CRITICAL BOLT SIMILAR SPW SOLDIER PILE LAGGING WALL ST THICKNESS TOP REINFORCEMENT TO BE CONFIRMED TO POF FOOTING TO STEEL TYPP TYPICAL UND,UON UNLESS OTHERWISE NOTED WORKPOINT		
CDC COMPRESSION SPLICE LENGTH FOR REINFORCEMENT LOBB LONG LEGS BACK-TO-BACK LIGHTWEIGHT CONCRETE MEDING MOMENT CONNECTION SHOWN ON DRAWING MINIMUM MINIMUM NO NEW CONSTRUCTION SHOWN ON DRAWING SHOWN ON TO SCALE ON CENTER PLATE ON CENTER PLATE ON CENTER PLATE PROPERTY LINE SAD SEE ARCHITECTURAL DRAWINGS/DETAILS SIAS SLAB ON DECK TYPE SC SLIP CRITICAL BOLT SIMILAR SIMILAR SOLDIER PILE LAGGING WALL SHEAR WALL TENSION FORCE IN KIPS THICKNESS TOP REINFORCEMENT TO BE CONFIRMED TO BE CONFIRMED TO FOOTONG TOP OF FOOTING TOP OF STEEL TYPE TYPICAL DNO,UON UNLESS OTHERWISE NOTED MINIMUM MOMENT VERTICAL BEAM END REACTION IN KIPS VERIFY IN FIELD WP WORKPOINT		
LUBB LONG LEGS BACK-TO-BACK LW LIGHTWEIGHT CONCRETE M BENDING MOMENT IN FOOT-KIPS MC MOMENT CONNECTION SHOWN ON DRAWING MIN MINIMUM N) NEW CONSTRUCTION N BEARING BOLTS THREADS INCLUDED IN SHEAR PLAN NTS NOT TO SCALE OC ON CENTER PC PILE CAP PL PLATE PP PARTIAL PENETRATION WELD PL PROPERTY LINE SAD SEE ARCHITECTURAL DRAWINGS/DETAILS SI,S2 SLAB ON DECK TYPE SC SLIP CRITICAL BOLT SIM SIMILAR SPW SOLDIER PILE LAGGING WALL SW SHEAR WALL TENSION FORCE IN KIPS THICKNESS TOP REINFORCEMENT TO BE CONFIRMED TOO TOP OF CONCRETE TOP OF FOOTING TOS TOP OF STEEL TYP TYPICAL JNO,UON UNLESS OTHERWISE NOTED M MOMENT VERTICAL BEAM END REACTION IN KIPS VIF VERIFY IN FIELD WP WORKPOINT		
M BENDING MOMENT IN FOOT-KIPS MC MOMENT CONNECTION SHOWN ON DRAWING MIN MINIMUM N) NEW CONSTRUCTION N BEARING BOLTS THREADS INCLUDED IN SHEAR PLAN NOT TO SCALE OC ON CENTER DC PILE CAP PLATE PP PARTIAL PENETRATION WELD PL PROPERTY LINE SAD SEE ARCHITECTURAL DRAWINGS/DETAILS SI,\$2 SLAB ON DECK TYPE SC SLIP CRITICAL BOLT SIM SIMILAR SPW SOLDIER PILE LAGGING WALL SW SHEAR WALL TENSION FORCE IN KIPS THICKNESS TOP REINFORCEMENT TOBE CONFIRMED TOC TOP OF CONCRETE TOF TOP OF FOOTING TOS TOP OF STEEL TYP TYPICAL JNO,UON UNLESS OTHER WISE NOTED M MOMENT VERTICAL BEAM END REACTION IN KIPS VIF VERIFY IN FIELD WP WORKPOINT	LLBB	
MC MOMENT CONNECTION SHOWN ON DRAWING MIN MINIMUM N) NEW CONSTRUCTION N BEARING BOLTS THREADS INCLUDED IN SHEAR PLAN NTS NOT TO SCALE OC ON CENTER PC PILE CAP PLATE PP PARTIAL PENETRATION WELD PL PROPERTY LINE SAD SEE ARCHITECTURAL DRAWINGS/DETAILS SI,S2 SLAB ON DECK TYPE SC SLIP CRITICAL BOLT SIMILAR SPW SOLDIER PILE LAGGING WALL SW SHEAR WALL TENSION FORCE IN KIPS THICKNESS TOP REINFORCEMENT TOBE CONFIRMED TOC TOP OF CONCRETE TOF TOP OF STEEL TYP TYPICAL JNO,UON UNLESS OTHERWISE NOTED M MOMENT VERTICAL BEAM END REACTION IN KIPS VIF VERIFY IN FIELD WO WORKPOINT	LW	LIGHTWEIGHT CONCRETE
MIN MINIMUM N) NEW CONSTRUCTION N BEARING BOLTS THREADS INCLUDED IN SHEAR PLAN NTS NOT TO SCALE OC ON CENTER PL PL PL PL PL PH PARTIAL PENETRATION WELD PL PROPERTY LINE SAD SEE ARCHITECTURAL DRAWINGS/DETAILS SI,S2 SLAB ON DECK TYPE SC SLIP CRITICAL BOLT SIM SIMILAR SPW SOLDIER PILE LAGGING WALL SW SHEAR WALL TENSION FORCE IN KIPS THICKNESS TOP REINFORCEMENT TOB E CONFIRMED TOO OF CONCRETE TOP OF FOOTING TOP OF STEEL TYP TYPICAL JNO,UON UNLESS OTHERWISE NOTED M MOMENT VERTICAL BEAM END REACTION IN KIPS VIF VERIFY IN FIELD WP WORKPOINT	M	
N) NEW CONSTRUCTION N BEARING BOLTS THREADS INCLUDED IN SHEAR PLAN NTS NOT TO SCALE OC ON CENTER PC PILE CAP PL PLATE PP PARTIAL PENETRATION WELD PL PROPERTY LINE SAD SEE ARCHITECTURAL DRAWINGS/DETAILS SI,S2 SLAB ON DECK TYPE SC SLIP CRITICAL BOLT SIM SIMILAR SPW SOLDIER PILE LAGGING WALL SW SHEAR WALL TENSION FORCE IN KIPS THICKNESS TOP REINFORCEMENT TOBE CONFIRMED TOC TOP OF CONCRETE TOF TOP OF FOOTING TOS TOP OF STEEL TYP TYPICAL JNO,UON UNLESS OTHERWISE NOTED M MOMENT V VERTICAL BEAM END REACTION IN KIPS VIF VERIFY IN FIELD WP WORKPOINT		
BEARING BOLTS THREADS INCLUDED IN SHEAR PLAN NTS NOT TO SCALE OC ON CENTER PC PL PLATE PP PARTIAL PENETRATION WELD PL PROPERTY LINE SAD SEE ARCHITECTURAL DRAWINGS/DETAILS SI,S2 SLAB ON DECK TYPE SC SLIP CRITICAL BOLT SIM SIMILAR SPW SOLDIER PILE LAGGING WALL SW SHEAR WALL TENSION FORCE IN KIPS THICKNESS TOP REINFORCEMENT TOP REINFORCEMENT TOP GO TOP OF CONCRETE TOF TOP OF FOOTING TOS TOP OF STEEL TYP TYPICAL JNO,UON UNLESS OTHERWISE NOTED M MOMENT VERTICAL BEAM END REACTION IN KIPS VIF VERIFY IN FIELD WP WORKPOINT		
NTS NOT TO SCALE OC ON CENTER PC PILE CAP PL PLATE PP PARTIAL PENETRATION WELD PL PROPERTY LINE SAD SEE ARCHITECTURAL DRAWINGS/DETAILS S1,S2 SLAB ON DECK TYPE SC SLIP CRITICAL BOLT SIM SIMILAR SPW SOLDIER PILE LAGGING WALL SW SHEAR WALL TENSION FORCE IN KIPS THICKNESS TOP REINFORCEMENT TOP REINFORCEMENT TOP OF CONCRETE TOP OF FOOTING TOS TOP OF STEEL TYP TYPICAL JNO,UON UNLESS OTHERWISE NOTED M MOMENT V VERTICAL BEAM END REACTION IN KIPS VIF VERIFY IN FIELD WP WORKPOINT		
OC ON CENTER PC PILE CAP PL PLATE PP PARTIAL PENETRATION WELD PL PROPERTY LINE SAD SEE ARCHITECTURAL DRAWINGS/DETAILS SI,S2 SLAB ON DECK TYPE SC SLIP CRITICAL BOLT SIM SIMILAR SPW SOLDIER PILE LAGGING WALL SW SHEAR WALL TENSION FORCE IN KIPS THICKNESS TOP REINFORCEMENT TOP REINFORCEMENT TOO TOP OF CONCRETE TOO TOP OF FOOTING TOS TOP OF STEEL TYP TYPICAL JNO,UON UNLESS OTHERWISE NOTED M MOMENT VERTICAL BEAM END REACTION IN KIPS VIF VERIFY IN FIELD WP WORKPOINT		
PC PILE CAP PL PLATE PP PARTIAL PENETRATION WELD PL PROPERTY LINE SAD SEE ARCHITECTURAL DRAWINGS/DETAILS SI,S2 SLAB ON DECK TYPE SC SLIP CRITICAL BOLT SIM SIMILAR SPW SOLDIER PILE LAGGING WALL SW SHEAR WALL TENSION FORCE IN KIPS THICKNESS TO TOP REINFORCEMENT TOBE CONFIRMED TOC TOP OF CONCRETE TOF TOP OF FOOTING TOS TOP OF STEEL TYP TYPICAL JNO,UON UNLESS OTHERWISE NOTED M MOMENT VERTICAL BEAM END REACTION IN KIPS VIF VERIFY IN FIELD WP WORKPOINT		
PP PARTIAL PENETRATION WELD PL PROPERTY LINE SAD SEE ARCHITECTURAL DRAWINGS/DETAILS S1,S2 SLAB ON DECK TYPE SC SLIP CRITICAL BOLT SIM SIMILAR SPW SOLDIER PILE LAGGING WALL SW SHEAR WALL TENSION FORCE IN KIPS THICKNESS TO TOP REINFORCEMENT TOBE CONFIRMED TOO TOP OF CONCRETE TOP OF FOOTING TOS TOP OF STEEL TYP TYPICAL JNO,UON UNLESS OTHERWISE NOTED M MOMENT V VERTICAL BEAM END REACTION IN KIPS VIF VERIFY IN FIELD WP WORKPOINT	PC	
PL PROPERTY LINE SAD SEE ARCHITECTURAL DRAWINGS/DETAILS S1,S2 SLAB ON DECK TYPE SC SLIP CRITICAL BOLT SIM SIMILAR SPW SOLDIER PILE LAGGING WALL SW SHEAR WALL TENSION FORCE IN KIPS THICKNESS TOP REINFORCEMENT TOBE CONFIRMED TOO OF CONCRETE TOP OF FOOTING TOS TOP OF STEEL TYP TYPICAL JNO,UON UNLESS OTHERWISE NOTED M MOMENT VERTICAL BEAM END REACTION IN KIPS VIF VERIFY IN FIELD WP WORKPOINT	PL	
SAD SEE ARCHITECTURAL DRAWINGS/DETAILS S1,S2 SLAB ON DECK TYPE SC SLIP CRITICAL BOLT SIM SIMILAR SPW SOLDIER PILE LAGGING WALL SW SHEAR WALL TENSION FORCE IN KIPS THICKNESS TOP REINFORCEMENT TOBE CONFIRMED TOO TOP OF CONCRETE TOP OF FOOTING TOS TOP OF STEEL TYP TYPICAL JNO,UON UNLESS OTHERWISE NOTED M MOMENT VERTICAL BEAM END REACTION IN KIPS VIF VERIFY IN FIELD WP WORKPOINT	PP	
SI,S2 SLAB ON DECK TYPE SC SLIP CRITICAL BOLT SIM SIMILAR SPW SOLDIER PILE LAGGING WALL SW SHEAR WALL TENSION FORCE IN KIPS THICKNESS TOP REINFORCEMENT TOP REINFORCEMENT TOP OF CONCRETE TOP OF FOOTING TOP OF STEEL TYP TYPICAL JNO,UON UNLESS OTHERWISE NOTED M MOMENT VERTICAL BEAM END REACTION IN KIPS VIF VERIFY IN FIELD WP WORKPOINT		
SC SLIP CRITICAL BOLT SIM SIMILAR SPW SOLDIER PILE LAGGING WALL SW SHEAR WALL TENSION FORCE IN KIPS THICKNESS TOP REINFORCEMENT TOBE CONFIRMED TOC TOP OF CONCRETE TOF TOP OF FOOTING TOS TOP OF STEEL TYP TYPICAL JNO,UON UNLESS OTHERWISE NOTED M MOMENT V VERTICAL BEAM END REACTION IN KIPS VIF VERIFY IN FIELD WP WORKPOINT		
SIM SIMILAR SPW SOLDIER PILE LAGGING WALL SW SHEAR WALL TENSION FORCE IN KIPS THICKNESS TOP REINFORCEMENT TBC TO BE CONFIRMED TOC TOP OF CONCRETE TOF TOP OF FOOTING TOS TOP OF STEEL TYP TYPICAL JNO,UON UNLESS OTHERWISE NOTED M MOMENT V VERTICAL BEAM END REACTION IN KIPS VIF VERIFY IN FIELD WP WORKPOINT		
SPW SOLDIER PILE LAGGING WALL SW SHEAR WALL TENSION FORCE IN KIPS THICKNESS TOP REINFORCEMENT TOBE CONFIRMED TOC TOP OF CONCRETE TOF TOP OF FOOTING TOS TOP OF STEEL TYP TYPICAL JNO,UON UNLESS OTHERWISE NOTED M MOMENT V VERTICAL BEAM END REACTION IN KIPS VIF VERIFY IN FIELD WP WORKPOINT		
SW SHEAR WALL TENSION FORCE IN KIPS THICKNESS TOP REINFORCEMENT TOP CONFIRMED TOC TOP OF CONCRETE TOF TOP OF FOOTING TOS TOP OF STEEL TYP TYPICAL JNO,UON UNLESS OTHERWISE NOTED M MOMENT V VERTICAL BEAM END REACTION IN KIPS VIF VERIFY IN FIELD WP WORKPOINT		
THICKNESS TOP REINFORCEMENT TOP REINFORCEMENT TOBE CONFIRMED TOC TOP OF CONCRETE TOF TOP OF FOOTING TOS TOP OF STEEL TYP TYPICAL JNO,UON UNLESS OTHERWISE NOTED M MOMENT V VERTICAL BEAM END REACTION IN KIPS VIF VERIFY IN FIELD WP WORKPOINT		SHEAR WALL
TOP REINFORCEMENT TOBE CONFIRMED TOC TOP OF CONCRETE TOF TOP OF FOOTING TOS TOP OF STEEL TYP TYPICAL JNO,UON UNLESS OTHERWISE NOTED M MOMENT V VERTICAL BEAM END REACTION IN KIPS VIF VERIFY IN FIELD WP WORKPOINT		
TO BE CONFIRMED TOC TOP OF CONCRETE TOF TOP OF FOOTING TOS TOP OF STEEL TYP TYPICAL JNO,UON UNLESS OTHERWISE NOTED M MOMENT V VERTICAL BEAM END REACTION IN KIPS VIF VERIFY IN FIELD WP WORKPOINT		
TOC TOP OF CONCRETE TOF TOP OF FOOTING TOS TOP OF STEEL TYP TYPICAL UNO,UON UNLESS OTHERWISE NOTED M MOMENT V VERTICAL BEAM END REACTION IN KIPS VIF VERIFY IN FIELD WP WORKPOINT		
TOP OF FOOTING TOS TOP OF STEEL TYP TYPICAL UNO,UON UNLESS OTHERWISE NOTED M MOMENT V VERTICAL BEAM END REACTION IN KIPS VIF VERIFY IN FIELD WP WORKPOINT		
TOS TOP OF STEEL TYP TYPICAL JNO,UON UNLESS OTHERWISE NOTED M MOMENT VERTICAL BEAM END REACTION IN KIPS VIF VERIFY IN FIELD WP WORKPOINT		
TYP TYPICAL JNO,UON UNLESS OTHERWISE NOTED M MOMENT VERTICAL BEAM END REACTION IN KIPS VIF VERIFY IN FIELD WP WORKPOINT	TOS	
M MOMENT VERTICAL BEAM END REACTION IN KIPS VIF VERIFY IN FIELD WP WORKPOINT	ΓΥΡ	
VERTICAL BEAM END REACTION IN KIPS VIF VERIFY IN FIELD WP WORKPOINT	JNO,UON	UNLESS OTHERWISE NOTED
VIF VERIFY IN FIELD WP WORKPOINT	M	
WP WORKPOINT		
WEEDED WIND LADING		
	** ** *	" ADDED WIND LADING

LEC	GEND
See had to be well of the thinks	NEW CONCRETE RETAINING WALL
	NEW FOOTING
	PROPERTY LINE



\bigcap	VICINITY MAP

	SPECIAL INSPECTIONS	
, , , , , , , , , , , , , , , , , , , ,	STRUCTURAL STEEL - WELDING	BC 1704.3.1
	STRUCTURAL STEEL - DETAILS	BC 1704.3.2
	STRUCTURAL STEEL - HIGH STRENGTH BOLTS	BC 1704.3.3
	STRUCTURAL COLD-FORMED STEEL	BC 1704.3.4
•	CONCRETE - CAST-IN-PLACE	BC 1704.4
	CONCRETE - PRECAST	BC 1704.4
	CONCRETE - PRESTRESSED	BC 1704.4
	MASONRY	BC 1704.5
*	WOOD - INSTALLATION OF HIGH-LOAD DIAPHRAGMS	BC 1704.6.1
	WOOD-INSTALLATION OF METAL-PLATE-CONNECTED TRUSSES	BC 1704.6.2
	WOOD - INSTALLATION OF PREFABRICATED I-JOISTS	BC 1704.6.3
	SUBGRADE INSPECTION	BC 1704.7.1
	SUBRAGE CONDITIONS - FILL PLACEMENT & IN-PLACE DENSITY	BC 1704.7.2 BC 1704.7.3
	SUBSURFACE INVESTIGATIONS (BORINGS/TEST PITS) TR4	BC 1704.7.4
	DEEP FOUNDATION ELEMENTS TR5	BC 1704.8
	HELICAL PILES (BB# 2014-020) TR5H	BC 1704.8.5
	VERTICAL MASONRY FOUNDATION ELEMENTS	BC 1704.9
	WALL PANELS, CURTAIN WALLS, AND VENEERS	BC 1704.10
	STRUCTURAL STABILITY - EXISTING BUILDINGS	BC 1704.20.1
	EXCAVATIONS-SHEETING, SHORING, AND BRACING	BC 1704.20.2
	UNDERPINNING	BC 1704.20.3 BC1814
	RAISING AND MOVING OF A BUILDING	BC 1704.20.5
. •	POST-INSTALLED ANCHORS (BB# 2014-018, 2014-019)	BC 1704.32
	SEISMIC ISOLATION SYSTEMS	BC 1707.8
	CONCRETE DESIGN MIX TR3	BC1905.3 BC 1913.5
	CONCRETE SAMPLING AND TESTING TR2	BC 1905.6 BC1913.10
	PROGRESS INSPECTIONS	
	FOOTING AND FOUNDATION	BC 110.3.1
	STRUCTURAL WOOD FRAME	BC 110.3.2.
	FINAL	BC 110.5

•	REINFORCING STEEL
	STEEL DECKING AND STUD LAYOUT
	STRUCTURAL STEEL
	STEEL STAIRS
	CONCRETE STAIRS
	STRUCTURAL COLD FORM STEEL
	PRECAST CONCRETE
•	CONCRETE DESIGN MIX
	ANCHOR ROD LAYOUT
	COMPOSITE DRAWINGS OF ALL SLAB PENETRATIONS
	CONCRETE MASONRY
	CURTAIN WALLS

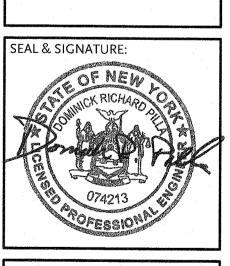
Ike Kligerman Barkley

330 West Forty-Second Stree New York, NY 10036

59 TWEED BLVD.

ببجريسند		
No.	REMARKS	DATE
7		
2		
3		
PRO	IECT:	

RESIDENCI



TITLE SHEET

DATE:	08/17/2021	
PROJECT No. :	21-077	
RAWING BY :	AM/JB	
CHK BY :	GF	
DWG No.:	-	
S-001	.00	

GENERAL NOTES

- UNLESS OTHERWISE NOTED OR SHOWN ON THE STRUCTURAL DRAWINGS, THE FOLLOWING REQUIREMENTS, TOGETHER WITH THE PROJECT PLANS, SPECIFICATIONS AND GEOTECHNICAL REPORT APPLY TO THE STRUCTURES IN THIS CONTRACT.
- 1. CONSTRUCTION IS TO COMPLY WITH THE REQUIREMENTS OF THE GOVERNING BUILDING CODE AND ALL OTHER APPLICABLE FEDERAL, STATE, AND LOCAL
- 2. THE STRUCTURAL DOCUMENTS SHALL BE USED IN CONJUNCTION WITH AND COORDINATED WITH THE ARCHITECTURAL, CIVIL AND MEP CONTRACT DOCUMENTS AS WELL AS ANY OTHER TRADES. IF A CONFLICT EXISTS, CONTRACTOR SHALL NOTIFY THE ARCHITECT/ENGINEER AND OBTAIN CLARIFICATION PRIOR TO BIDDING AND PROCEEDING WITH WORK

CODES, STANDARDS, REGULATIONS AND LAWS.

- 3. THE GENERAL CONTRACTOR SHALL COORDINATE ALL CONTRACT DOCUMENTS WITH FIELD CONDITIONS, DIMENSIONS, ELEVATIONS AND PROJECT SHOP DRAWINGS PRIOR TO CONSTRUCTION. DO NOT SCALE DRAWINGS; USE ONLY PRINTED DIMENSIONS. REPORT ANY DISCREPANCIES IN WRITING TO THE ARCHITECT/ENGINEER PRIOR TO PROCEEDING WITH WORK. DO NOT CHANGE SIZE OR LOCATION OF STRUCTURAL MEMBERS WITHOUT WRITTEN INSTRUCTIONS FROM THE STRUCTURAL ENGINEER OF RECORD.
- 4. THE DESIGN AT THE EXISTING PART OF THE BUILDING, WHICH WILL REMAIN, IS BASED ON INCOMPLETE INFORMATION ABOUT THE EXISTING STRUCTURE. THE SIZE AND DEPTH OF EXISTING FOUNDATION. AS THE WORK PROGRESS, THE CONTRACTOR SHALL PROVIDE THE ENGINEER WITH FIELD INFORMATION ABOUT THE EXISTING FOUNDATION AND OTHER STRUCTURAL MEMBERS AND FOLLOW ANY CHANGES IN DESIGN THAT WILL BE REQUIRED BY THE ENGINEER DUE TO UNANTICIPATED FIELD CONDITIONS.
- 5. OPENINGS SHOWN ON STRUCTURAL DRAWINGS ARE ONLY PICTORIAL. SEE THE ARCHITECTURAL AND M.E.P. DRAWINGS FOR THE SIZE AND LOCATION OF OPENINGS IN THE STRUCTURE.
- 6. CONTRACTORS WHO DISCOVER DISCREPANCIES, OMISSIONS OR VARIATIONS IN THE CONTRACT DOCUMENTS DURING BIDDING SHALL IMMEDIATELY NOTIFY THE ARCHITECT. THE ARCHITECT WILL RESOLVE THE CONDITION AND ISSUE A WRITTEN CLARIFICATION.
- 7. THE CONTRACTOR SHALL PROTECT ADJACENT PROPERTY, HIS OWN WORK AND THE PUBLIC FROM HARM. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR CONSTRUCTION MEANS AND METHODS, AND JOBSITE SAFETY INCLUDING ALL OSHA REQUIREMENTS.
- 8. SEE PROJECT SPECIFICATIONS FOR TESTING. SEE THE STRUCTURAL SPECIAL INSPECTION NOTES FOR INSPECTION REQUIREMENTS.
- 9. DETAILS LABELED "TYPICAL" APPLY TO ALL SITUATIONS THAT ARE THE SAME OR SIMILAR TO THOSE SPECIFICALLY REFERENCED, WHETHER OR NOT THEY ARE KEYED IN AT EACH LOCATION. QUESTIONS REGARDING THE APPLICABILITY OF TYPICAL DETAILS SHALL BE RESOLVED BY THE ARCHITECT.
- 10. THE STRUCTURE IS DESIGNED TO BE STRUCTURALLY SOUND WHEN COMPLETED. PRIOR TO COMPLETION, THE CONTRACTOR IS RESPONSIBLE FOR STABILITY AND TEMPORARY BRACING, INCLUDING, BUT NOT LIMITED TO, MASONRY WALLS. THE CONTRACTOR SHALL VERIFY THAT CONSTRUCTION LOADS DO NOT EXCEED THE CAPACITY OF THE STRUCTURE AT THE TIME THE LOAD IS APPLIED. WHENEVER THE CONTRACTOR IS UNSURE OF THESE REQUIREMENTS, THE CONTRACTOR SHALL RETAIN A NEW YORK STATE LICENSED ENGINEER TO DESIGN AND INSPECT THE TEMPORARY BRACING AND STABILITY OF THE STRUCTURE.

CODES AND SPECIFICATIONS

THE DESIGN SHOWN ON THESE DRAWINGS IS BASED ON THE FOLLOWING CODES, SPECIFICATIONS AND STANDARDS:

- . "NEW YORK CITY BUILDING CODE," 2014 EDITION. ASCE 7-16: "MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES," AWS D1.1: "STRUCTURAL WELDING CODE," 2014. "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE," ACI 318-2014. "SPECIFICATIONS FOR STRUCTURAL CONCRETE," ACI 301-1999. "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES," ACI 530-2013.
- "SPECIFICATIONS FOR STRUCTURAL STEEL BUILDINGS," AISC 360-16, "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES," AISC "SEISMIC PROVISIONS FOR STRUCTURAL STEEL BUILDINGS," AISC-341-16, "STANDARD FOR COMPOSITE STEEL FLOOR DECK-SLAB," SDI, 2011.
- 2. "NATIONAL DESIGN SPECIFICATION (NDS) FOR WOOD CONSTRUCTION," AWC NDS
- ASCE 37-14: "DESIGN LOADS ON STRUCTURES DURING CONSTRUCTION."
- 3. "BUILDING CODE OF NEW YORK STATE," 2020.

"STANDARD FOR STEEL ROOF DECK," SDI 2010.

SHOP DRAWINGS AND OTHER SUBMITTALS

- 1. SEE TITLE SHEET FOR REQUIRED SUBMITTALS.
- 2. INCOMPLETE SUBMITTALS WILL BE RETURNED WITHOUT REVIEW.
- 3. SUBMIT SPECIFIC COMPONENTS, SUCH AS COLUMNS, FOOTINGS, ETC., IN A SINGLE PACKAGE. SUBMIT SIMILAR FLOORS TOGETHER.
- 4. ON FIRST SUBMITTAL, CLEARLY FLAG AND CLOUD ALL DIFFERENCES FROM THE CONTRACT DOCUMENTS. ON RE-SUBMITTALS, FLAG AND CLOUD ALL CHANGES AND ADDITIONS TO PREVIOUS SUBMITTAL. ONLY CLOUDED ITEMS WILL BE
- 5. SUBMITTALS FOR SPECIAL STRUCTURAL, LOAD-CARRYING ITEMS THAT ARE REQUIRED BY CODES OR STANDARDS TO RESIST FORCES MUST BE PREPARED BY, OR UNDER THE DIRECT SUPERVISION OF, A DELEGATED ENGINEER. EXAMPLES INCLUDE STRUCTURAL STEEL CONNECTIONS, STRUCTURAL LIGHT GAGE STEEL FRAMING, AND EXTERIOR ENCLOSURE SYSTEMS.
- 6. A DELEGATED ENGINEER IS DEFINED AS A NEW YORK STATE LICENSED ENGINEER WHO SPECIALIZES IN AND UNDERTAKES THE DESIGN OF STRUCTURAL COMPONENTS OR STRUCTURAL SYSTEMS INCLUDED IN A SPECIFIC SUBMITTAL PREPARED FOR THIS PROJECT AND IS AN EMPLOYEE OR OFFICER OF, OR CONSULTANT TO, THE CONTRACTOR OR FABRICATOR RESPONSIBLE FOR THE SUBMITTAL. THE DELEGATED ENGINEER SHALL SIGN, SEAL AND DATE THE SUBMITTAL, INCLUDING CALCULATIONS AND DRAWINGS. SEE SPECIFICATIONS FOR MORE SPECIFIC CRITERIA.
- . THE TRADE CONTRACTOR IS RESPONSIBLE FOR CONFIRMING AND CORRELATING DIMENSIONS AT THE JOB SITES, FOR TOLERANCES, CLEARANCES, QUANTITIES, FABRICATION PROCESSES AND TECHNIQUES OF CONSTRUCTION, COORDINATION OF THE WORK WITH OTHER TRADES AND FULL COMPLIANCE WITH THE CONTRACT DOCUMENTS.
- 8. THE GENERAL CONTRACTOR/CONSTRUCTION MANAGER SHALL REVIEW AND APPROVE SUBMITTALS AND SHALL SIGN AND DATE EACH DRAWING PRIOR TO SUBMITTING TO THE ARCHITECT. THIS APPROVAL IS TO CONFIRM THAT THE SUBMITTAL IS COMPLETE, COMPLIES WITH THE SUBMITTAL REQUIREMENTS AND IS COORDINATED WITH FIELD DIMENSIONS, OTHER TRADES, ERECTION SEQUENCING AND CONSTRUCTABILITY.

- 9. THE STRUCTURAL ENGINEER REVIEWS SUBMITTALS TO CONFIRM THAT THE SUBMITTAL IS IN GENERAL CONFORMANCE WITH THE DESIGN CONCEPT PRESENTED IN THE CONTRACT DOCUMENTS. QUANTITIES AND DIMENSIONS ARE NOT CHECKED. NOTATIONS ON SUBMITTALS DO NOT AUTHORIZE CHANGES TO THE CONTRACT SUM. CHECKING OF THE SUBMITTAL BY THE STRUCTURAL ENGINEER SHALL NOT RELIEVE THE CONTRACTOR OF RESPONSIBILITY FOR DEVIATIONS FROM THE CONTRACT DOCUMENTS AND FROM ERRORS OR OMISSIONS IN THE SUBMITTAL.
- 10. IN ADDITION TO THE ABOVE, THE STRUCTURAL ENGINEER'S REVIEW OF DELEGATED ENGINEER SUBMITTALS IS LIMITED TO VERIFYING THAT THE SPECIFIED STRUCTURAL SUBMITTAL HAS BEEN FURNISHED, SIGNED AND SEALED BY THE DELEGATED ENGINEER AND THAT THE DELEGATED ENGINEER HAS UNDERSTOOD THE DESIGN INTENT AND USED THE SPECIFIED STRUCTURAL CRITERIA. NO DETAILED CHECK OF CALCULATIONS WILL BE MADE. THE DELEGATED ENGINEER IS SOLELY RESPONSIBLE FOR HIS/HER DESIGN, INCLUDING BUT NOT LIMITED TO THE ACCURACY OF HIS/HER CALCULATIONS AND COMPLIANCE WITH THE APPLICABLE CODES AND STANDARDS.
- 11. CAD FILES OF STRUCTURAL DRAWINGS MAY BE USED AS AN AID IN PREPARING SHOP DRAWINGS ONLY UPON THE CONTRACTOR SIGNING AN AGREEMENT. WHEN CAD FILES OR COPIES OF THE STRUCTURAL DRAWINGS ARE MADE AVAILABLE, IT IS UNDER THE FOLLOWING CONDITIONS:
- a. ALL INFORMATION CONTAINED IN THE CAD FILES OR COPIES OF THE STRUCTURAL DRAWINGS ARE INSTRUMENTS OF SERVICE OF THE ARCHITECT/ENGINEER AND SHALL NOT BE USED FOR OTHER PROJECTS ADDITIONS TO THE PROJECT OR THE COMPLETION OF THE PROJECT BY OTHERS. CAD FILES AND COPIES OF THE STRUCTURAL DRAWINGS REMAIN THE PROPERTY OF DOMINICK R. PILLA ASSOCIATES AND IN NO CASE SHALL THEIR TRANSFER BE CONSIDERED A SALE.
- b. CAD FILES OR COPIES OF THE STRUCTURAL DRAWINGS ARE NOT CONTRACT DOCUMENTS. IN THE EVENT OF A CONFLICT, THE STRUCTURAL DRAWINGS SHALL GOVERN.
- c. THE USE OF CAD FILES OR COPIES OF THE STRUCTURAL DRAWINGS SHALL NOT IN ANY WAY RELIEVE THE CONTRACTOR'S RESPONSIBILITY FOR PROPER CHECKING AND COORDINATION OF DIMENSIONS, DETAILS, SIZES AND QUANTITIES OF MATERIALS AS REQUIRED FOR THE PREPARATION OF COMPLETE AND ACCURATE SHOP DRAWINGS.
- d. THE CONTRACTOR SHALL REVISE ALL REFERENCES TO CONTRACT DOCUMENT SHEET NUMBERS AND SECTION MARKS AND SHALL REMOVE INFORMATION THAT IS NOT REQUIRED FOR THEIR WORK FROM THE CAD FILES OR COPIES OF THE STRUCTURAL DRAWINGS, INCLUDING THE TITLE BLOCK; AND
- e. DIMENSIONS IN THE CAD FILES MAY NOT BE PRECISE AND, IN SOME CASES, HAVE BEEN INTENTIONALLY ALTERED FOR PRESENTATION PURPOSES. DO NOT SCALE DIMENSIONS ELECTRONICALLY OR OTHERWISE.

SHALLOW FOUNDATIONS

- 1. FOUNDATION DESIGN IS BASED ON THE "GEOTECHNICAL ENGINEERING REPORT," PREPARED BY ROBERT DYKSTRA OF GTA ENGINEERING SERVICES OF NEW YORK, P.C. AND DATED 7/16/2021 SEE THAT REPORT FOR ADDITIONAL REQUIREMENTS.
- 2. FOUNDATIONS PLACED ON UNDISTURBED SOIL AT ELEVATIONS INDICATED ARE DESIGNED FOR AN ALLOWABLE NET SOIL BEARING PRESSURE OF 2,000 PSF.

AND

- FOUNDATIONS PLACED ON COMPACTED STRUCTURAL FILL HAVE BEEN DESIGNED FOR AN ALLOWABLE NET SOIL BEARING PRESSURE OF 2,000 PSF.
- 3. THE CONTRACTOR SHALL NOTIFY THE ENGINEER WHERE BOTTOM OF FOOTING ELEVATION IS CHANGED AND OBTAIN REVISED DESIGN OF THE FOUNDATION AND RETAINING WALLS AS REQUIRED.
- 4. ALL FILL REQUIRED BELOW ANY PORTION OF THE STRUCTURE SHALL BE COMPACTED IN 9" LIFTS TO AT LEAST 98% OF THE MAXIMUM DRY DENSITY PER ASTM D-1557. REMOVE UNSUITABLE FILL AND REPLACE WITH CONTROLLED FILL AS REQUIRED FOR SOUND PLACEMENT OF FOUNDATIONS.
- 5. SOIL SUPPORTED FOOTING SHALL BE FOUNDED UPON UNDISTURBED NATURAL SUBGRADE (OR CONTROLLED COMPACTED FILL) WITH A MINIMUM BEARING CAPACITY AS NOTED AND AS FIELD VERIFIED AND APPROVED BY A REGISTERED SOIL ENGINEER. THE BOTTOM OF THE FOOTING ELEVATIONS AND BEARING CAPACITIES AS SHOWN ON THE DRAWINGS ARE ESTIMATED AND WILL REQUIRE VERIFICATION. FINAL, EXACT ELEVATIONS AND BEARING CAPACITIES SHALL BE FIELD DETERMINED.
- 6. WHEN NECESSARY, FOOTINGS STEPS SHALL BE CONSTRUCTED AT MAXIMUM
- SLOPE OF ONE VERTICAL TO TWO HORIZONTALS. 7. CENTER ALL FOOTINGS UNDER THEIR RESPECTIVE COLUMNS OR WALLS, U.O.N.

EXCAVATION, BACKFILL AND DEWATERING

- 1. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR ALL EXCAVATION PROCEDURES INCLUDING LAGGING, SHORING, AND PROTECTION OF ADJACENT PROPERTY, STRUCTURES, STREETS AND UTILITIES IN ACCORDANCE WITH THE REQUIREMENTS OF THE LOCAL BUILDING DEPARTMENT AND OSHA REGULATIONS. DO NOT EXCAVATE WITHIN ONE FOOT OF THE ANGLE OF REPOSE OF ANY SOIL BEARING FOUNDATION UNLESS THE FOUNDATION IS PROPERLY PROTECTED AGAINST SETTLEMENT.
- 2. DO NOT BACKFILL AGAINST WALLS UNTIL 7 DAYS AFTER THE WALLS ARE BRACED BY THE STRUCTURE OR ARE TEMPORARILY BRACED. DO NOT BACKFILL CANTILEVERED RETAINING WALLS UNTIL CONCRETE IS 7 DAYS OLD. DO NOT BACKFILL UNTIL AFTER COMPLETION AND INSPECTION OF ANY WATERPROOFING.
- 3. THE CONTRACTOR IS RESPONSIBLE FOR THE DISPOSAL OF ALL ACCUMULATED WATER IN A MANNER THAT DOES NOT INCONVENIENCE OR DAMAGE THE WORK.

SLABS ON GRADE

- 1. SLABS-ON-GRADE SHALL BE 5" THICK FIBER REINFORCED CONCRETE SLAB. (PROVIDE 3 LBS PER CU. FT. OF MACRO SYNTHETIC FIBER.) SHALL BE FINISHED IN ACCORDANCE WITH ACI STANDARD 302.1R FOR CLASS 2 FLOORS. TYPE II OR I/II CEMENT AND 3/4" COARSE AGGREGATE (SIZE NO. 57) SHALL BE USED.
- 2 SUBGRADE SOILS BELOW SLAB SHALL BE PROOF ROLLED AND CERTIFIED BY A SOILS ENGINEER AS ACCEPTABLE BEFORE PLACEMENT OF GRAVEL OR CONCRETE.
- 3. PROVIDE CONTROL JOINTS SPACING OF 36 TIMES THE SLAB THICKNESS MAXIMUM.
- 4. FOLLOW RECOMMENDATIONS OF ACI 302.1R.
- 5. SIDEWALKS AND WALKWAYS, LOCATE ISOLATION JOINTS AT 20 FT. O.C. MAXIMUM SCORE AND TOOL BETWEEN ISOLATION JOINTS IN EQUAL BAYS OF 5 FT. OR LESS.
- 6. SEE THE ARCHITECTURAL DRAWINGS FOR SLAB ON GRADE DEPRESSIONS AND OTHER REQUIREMENTS.

REINFORCED CONCRETE

- 1. COMPLY WITH ACI 301 AND 318.
- 2. ALL CAST-IN-PLACE CONCRETE SHALL BE CONTROLLED CONCRETE AND SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH (Fc) AT 28 DAYS AS FOLLOWS:
- FOOTINGS 4,000 PSI POURED WALLS 4,000 PSI SLABS-ON-GRADE 4,000 PSI
- SLAB ON METAL DECK 4,000 PSI (LT. WT. CONC.)
- 3. USE NORMAL WEIGHT CONCRETE FOR ALL STRUCTURAL MEMBERS. U.O.N.
- 4. CONCRETE REINFORCEMENT SHALL BE ASTM A615, GRADE 60 DEFORMED REINFORCING STEEL. LAP BOTTOM STEEL OVER SUPPORTS AND TOP STEEL AT MIDSPAN (U.O.N.). HOOK DISCONTINUOUS ENDS OF ALL TOP BARS AND ALL BARS IN WALLS, U.O.N.
- 5. USE EPOXY COATED REINFORCEMENT CONFORMING TO ASTM A775 FOR CONCRETE SUBJECT TO WATER AND CHLORITE PENETRATION.

A.LOADING DOCK SLABS AND WALLS.

- 6. WHERE SPECIFIED, PROVIDE PLAIN, COLD-DRAWN ELECTRONICALLY WELDED WIRE REINFORCEMENT (WWF) CONFORMING TO ASTM A185. SUPPLY IN FLAT SHEETS ONLY. LAP SPLICE SHALL BE ONE CROSS WIRE SPACING PLUS TWO INCHES.
- 7. FOLLOW ACI 117-10 "SPECIFICATION FOR TOLERANCES OF CONCRETE CONSTRUCTION AND MATERIALS" FOR REQUIRED TOLERANCES.
- 8. UTILITIES SHALL NOT PENETRATE BEAMS OR COLUMNS BUT MAY PASS THROUGH SLABS AND WALLS INDIVIDUALLY, UON. SEE TYPICAL DETAILS.
- 9. PROVIDE CONSTRUCTION JOINTS IN ACCORDANCE WITH ACI 318, SECTION 6.4. PROVIDE KEYWAYS AND ADEQUATE DOWELS. SUBMIT DRAWINGS SHOWING LOCATION OF CONSTRUCTION JOINTS AND DIRECTION OF POUR FOR REVIEW.
- 10. PROVIDE 3/4" CHAMFER FOR ALL EXPOSED CORNERS. SEE ARCHITECTURAL DRAWINGS FOR ADDITIONAL CONCRETE FINISH REQUIREMENTS.

CONCRETE FIELD TESTING:

- 1. TESTING: OWNER WILL ENGAGE A QUALIFIED TESTING AGENCY TO PERFORM FIELD TESTS AND PREPARE TEST REPORTS.
- 2. CONCRETE TESTS: TESTING OF COMPOSITE SAMPLES OF FRESH CONCRETE OBTAINED ACCORDING TO ASTM C172 AND SECTION BC 1905.6.5 OF NYC BUILDING CODE SHALL BE PERFORMED ACCORDING TO THE FOLLOWING REQUIREMENTS:
- a. TESTING FREQUENCY: OBTAIN ONE COMPOSITE SAMPLE FOR EACH DAY'S POUR OF EACH CONCRETE MIXTURE LESS THAN 25 CU. YD., PLUS ONE SET FOR EACH ADDITIONAL 50 CU. YD. OR FRACTION THEREOF.
- b. WHEN FREQUENCY OF TESTING WILL PROVIDE FEWER THAN FIVE COMPRESSIVE STRENGTH TESTS OF EACH CONCRETE MIXTURE, TESTING SHALL BE CONDUCTED FROM AT LEAST FIVE RANDOMLY SELECTED BATCHES OR FROM EACH BATCH IF FEWER THAN FIVE ARE USED.
- c. WATER CONTENT AND SLUMP: VERIFY WATER CONTENT IN ACCORDANCE WITH AASHTO t-318 "STANDARD METHOD OF TESTS FOR WATER CONTENT USING MICROWAVE OVEN DRYING." TEST SLUMP IN ACCORDANCE WITH ASTM C143; ONE TEST AT POINT OF PLACEMENT FOR EACH COMPOSITE SAMPLE, BUT NOT LESS THAN ONE TEST FOR EACH DAY'S POUR OF EACH CONCRETE MIXTURE. PERFORM ADDITIONAL TESTS WHEN CONCRETE CONSISTENCY APPEARS TO CHANGE.
- d. AIR CONTENT: ASTM C231, PRESSURE METHOD, FOR NORMAL-WEIGHT CONCRETE: ASTM C173, VOLUMETRIC METHOD, FOR LIGHTWEIGHT CONCRETE: ONE TEST FOR EACH COMPOSITE SAMPLE, BUT NOT LESS THAN ONE TEST FOR EACH DAY'S POUR OF EACH CONCRETE MIXTURE.
- e. CONCRETE TEMPERATURE: ASTM C1064; ONE TEST HOURLY WHEN AIR TEMPERATURE IS 40 DEG F AND BELOW AND WHEN 80 DEG F AND ABOVE, AND ONE TEST FOR EACH COMPOSITE SAMPLE.
- f. COMPRESSION TEST SPECIMENS: ASTM C31.
- i. CAST AND LABORATORY CURE ALL TEST CYLINDER SPECIMENS. ii. WHEN REQUIRED, CAST AND FIELD CURE TWO SETS OF TWO STANDARD CYLINDER SPECIMENS FOR EACH COMPOSITE SAMPLE.
- g. COMPRESSIVE-STRENGTH TESTS: ASTM C39 AND SECTION BC 1905.6.2 OF THE NYC BUILDING CODE; TEST FIRST SET OF TWO LABORATORY-CURED SPECIMENS AT 7 DAYS FOR INFORMATION, SECOND SET OF TWO LABORATORY-CURED SPECIMENS AT 28 DAYS FOR ACCEPTANCE AND THIRD SET OF TWO SPECIMENS AT 56 DAYS IF NECESSARY.
- i. TEST ONE SET OF FIELD-CURED SPECIMENS AT 7 DAYS AND ONE SET OF TWO SPECIMENS AT 28 DAYS.
- ii. A COMPRESSIVE-STRENGTH TEST SHALL BE THE AVERAGE COMPRESSIVE STRENGTH FROM A SET OF TWO SPECIMENS OBTAINED FROM SAME COMPOSITE SAMPLE AND TESTED AT AGE INDICATED.

STRUCTURAL STEEL

- 1. FABRICATION AND ERECTION OF STRUCTURAL STEEL SHALL CONFORM TO AISC "SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS", WITH COMMENTARY, AND ALL OSHA REQUIREMENTS.
- 2. STRUCTURAL STEEL MATERIALS SHALL CONFORM TO THE FOLLOWING MINIMUM REQUIREMENTS, UNLESS OTHERWISE NOTED ON THE CONTRACT DOCUMENTS:
 - ROLLED W SHAPES: ASTM A992, GRADE 50.
- ROLLED M, S, C, MC, AND L SHAPES: ASTM A36, FY=36 KSI. PLATES AND BARS: ASTM A36, FY=36 KSI, UON.
- PLATES FOR MOMENT CONNECTIONS: ASTM A572, GR. 50. STEEL PIPE: ASTM A53, TYPE E OR S, GRADE B, FY=35 KSI.
- HOLLOW STRUCTURAL SECTIONS:
- ROUND SECTIONS: ASTM A500, GRADE C, FY=46 KSI SQUARE AND RECTANGULAR SECTIONS: ASTM A500, GRADE C, FY=50 KSI.
- 3. ALL STRUCTURAL STEEL CONNECTIONS BOLTS SHALL BE ASTM A325 OR ASTM A490, TYPE 1, UNLESS OTHERWISE NOTED, AND SHALL COMPLY WITH "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS," INCLUDING COMMENTARY, ALL JOINT TYPE SHALL BE 'PT' (PRETENSIONED).
- 4. BOLT SIZE SHALL BE 3/4" DIAMETER MINIMUM, UNLESS OTHERWISE NOTED.
- 5. A MINIMUM OF TWO (2) 3/4" DIAMETER A325 BOLTS SHALL BE PROVIDED AT EACH CONNECTIONS.

- 6. SHOP DRAWINGS SHALL BE COORDINATED WITH STAIR DETAILS. IF HANGER RODS ARE USED, PROVIDE STIFFENER PLATE, 3/8" THICK MINIMUM, ALONGSIDE HANGER LOCATION.
- 7. SHEAR AND BRACING CONNECTIONS SHALL BE DESIGNED AND DETAILED BY THE FABRICATOR IN ACCORDANCE WITH AISC FOR THE FORCES AND/OR REACTIONS SHOWN, THE FABRICATOR SHALL SUBMIT CALCULATIONS DEMONSTRATING THAT THE SELECTED SHEAR AND BRACING CONNECTIONS WILL ACHIEVE THE FORCES AND/OR REACTIONS INDICATED, OR AS REQUIRED BY THE CODES.
- 8. WHERE DETAILS ARE NOT GIVEN, CONNECTIONS FOR BEAMS AND GIRDERS ARE TO BE DESIGNED FOR THE GIVEN FORCES AND REACTIONS.
- 9. ANCHOR RODS SHALL BE ASTM F1554 GRADE 55 WITH WELDABILITY SUPPLEMENTARY REQUIREMENT S1, HOOKED OR ANCHOR RODS SHALL BE A449. TYPE 1, THREADED WITH NUTS AND WASHERS EACH END.
- 10. WHERE CAMBER IS INDICATED, FABRICATE BEAMS SO THAT ANY NATURAL CAMBER IS UPWARD AFTER ERECTION.
- 11. PROVIDE HOLES IN ALL STEEL TO PREVENT ANY ACCUMULATION OF WATER. HOLES SHALL NOT EXCEED 1" DIAMETER.
- 12. CUT, DRILL, OR PUNCH HOLES PERPENDICULAR TO METAL SURFACES. REAM HOLES THAT MUST BE ENLARGED TO ADMIT BOLTS AS PERMITTED BY ARCHITECT. DO NOT ENLARGE UNFAIR HOLES BY BURNING OR USING DRIFT PINS.
- 13. DO NOT SPLICE STRUCTURAL STEEL MEMBERS EXCEPT WHERE INDICATED ON THE DRAWINGS.
- 14. UNLESS NOTED OTHERWISE, PROVIDE A 1/4" CAP PLATE CONTINUOUSLY WELDED AT THE ENDS OF EXTERIOR EXPOSED HOLLOW STRUCTURAL SHAPES.
- 15. SEE ARCHITECTURAL AND MECHANICAL DRAWINGS FOR MISCELLANEOUS STEEL NOT SHOWN ON THE STRUCTURAL DRAWINGS.
- 16. ALL STEEL MEMBERS SHALL BE SHOP PAINTED TNEMEC 10-99 PRIMER OR APPROVED EQUAL 2.0 MILS IN THICKNESS. ALL WELDS AND BARE SPOTS SHALL RECEIVE TOUCH-UP PAINT. ALL STEEL WITH EXTERIOR EXPOSURE SHALL RECEIVE A SHOP PAINTED TNEMEC 66 OR 161 PRIMER AND A FIELD APPLIED FINISH COAT AFTER PRIMER TOUCH-UP. FINISH COAT SHALL BE EPOXY BASED WITH THICKNESS OF 2.0 MILS.
- 17. REFER TO ARCHITECTURAL DRAWINGS AND PROJECT SPECIFICATIONS FOR PAINTING AND FIREPROOFING OF STRUCTURAL STEEL. DO NOT PAINT STEEL SURFACES IN CONTACT WITH CONCRETE OR FIREPROOFING.
- 18. STEEL USING COMPLETE JOINT PENETRATION GROOVE WELDS THAT FUSE THROUGH THE THICKNESS OF THE FLANGE OR WEB SHALL HAVE A MINIMUM CHARPY V-NOTCH IMPACT TESTING VALUE AS FOLLOWS:
- a. ASTM A6, HOT-ROLLED SHAPES WITH A FLANGE THICKNESS EXCEEDING (2) INCHES AND BUILT-UP HEAVY SHAPES WITH PLATES EXCEEDING (2) INCHES IN THICKNESS: 20 FT-LB @ 70 DEG. F.
- b. REGARDLESS OF THICKNESS, ALL TRUSSES, LATERAL SYSTEM MEMBERS (INCLUDING COLUMNS, WIND GIRDERS, BRACES, ETC.): 20 FT-LB @ 70 DEG. F
- c. STEEL EXPOSED TO TEMPERATURES IN SERVICE BELOW 50 DEG. F: 20 FT-LB @ (LAST + 70 DEG. F, 40 DEG. F MAX)
- d. REGARDLESS OF THICKNESS, ALL TRUSSES, LATERAL SYSTEM MEMBERS (INCLUDING COLUMNS, WIND GIRDERS, BRACES, ETC) EXPOSED TO TEMPERATURES IN SERVICE BELOW 50 DEG. F: 30 FT-LB @ (LOWEST ANTICIPATED SERVICE TEMPERATURE + 70 DEG. F, 40 DEG. F MAX)
- e. WELD METAL: 20 FT-LB @ MINUS 20 DEG. F AND 40 FT-LB @ 70 DEG. F
- f. WELD METAL EXPOSED TO TEMPERATURES IN SERVICE BELOW 50 DEG. F: 20 FT-LB @ MINUS 20 DEG. F AND 40 FT-LB @ (LAST + 20 DEG. F, 40 DEG. F MAX)
- g. TESTING IS TO BE IN ACCORDANCE WITH ASTM A6, SUPPLEMENTARY REQUIREMENT S30, CHARPY V-NOTCH IMPACT TEST FOR STRUCTURAL SHAPES ALTERNATE CORE LOCATION, AT ROLLED SHAPES AND ASTM A673 FOR PLATES AT ANY PERMITTED LOCATIONS.

WELDING

- 1. ALL SHOP AND FIELD WELDING SHALL CONFORM TO THE AWS D1.1. STRUCTURAL WELDING CODE.
- 2. WELDING ELECTRODES SHALL CONFORM TO E70XX.
- 3. WHERE NECESSARY, REMOVE GALVANIZING OR PRIMER PRIOR TO WELDING.
- 4. ALL WELDERS SHALL BE LICENSED AND CERTIFIED TO AWS STANDARDS OR THOSE REQUIRED BY APPLICABLE BUILDING CODES.
- 5. ALL WELDS SHALL BE VISUALLY INSPECTED. ALL GROOVE WELDS SHALL RECEIVE RADIOGRAPHIC OR ULTRASONIC TESTING. MAGNETIC PARTICLE TEST 20 PERCENT OF ALL FILLET WELDS.
- 6. WELDING SHALL PROGRESS IN A MANNER THAT BALANCES THE STRESSES IN THE MEMBER, IN ACCORDANCE WITH AWS.

METAL DECK AND SHEAR STUDS

1. DECK UNITS SHALL BE MANUFACTURED AND INSTALLED IN ACCORDANCE WITH CURRENT "STANDARDS FOR COMPOSITE STEEL FLOOR DECKS -SLABS," AND "STANDARDS FOR STEEL ROOF DECKS" PUBLISHED BY THE STEEL DECK INSTITUTE.

7. FOLLOW PREHEAT REQUIREMENTS FOR BASE METAL PER AWS GUIDELINES.

- 2. ALL FLOOR DECK SECTIONS SHALL BE 3" DEEP, 18 GAGE MINIMUM, GALVANIZED COMPOSITE METAL DECK WITH A MINIMUM I = 0.993 IN4/FT, UNLESS OTHERWISE NOTED.
- 3. ALL ROOF DECK SHALL BE 1 1/2" TYPE 'B' WIDE RIB ROOF DECK, 20 GAGE MINIMUM, GALVANIZED DECK WITH MINIMUM I = 0.22 IN4/FT, UNLESS OTHERWISE
- 4. ALL DECKS SHALL BE CONTINUOUS OVER A MINIMUM OF TWO SPANS.

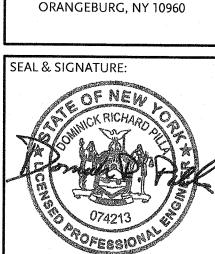
- 5. SHEAR CONNECTORS SHALL BE HEADED STUDS CONFORMING TO AWS D1.1 CHAPTER 7 (SECTION 7.2.6 AND TABLE 7.1) TYPE B, ASTM A108, Fu = 65 KSI, SIZE AND SPACING AS SPECIFIED ON THE DRAWINGS.
- 6. NO ELECTRICAL OR ANY OTHER ACCESS HOLES SHALL BE ALLOWED IN THE
- 7. PROVIDE CELLULAR, FLUTED AND FORM UNITS, OPENINGS, SLEEVES, ETC. AS INDICATED ON THE STRUCTURAL, ARCHITECTURAL AND MECHANICAL DRAWINGS AND SPECIFICATIONS.
- 8. PROVIDE METAL DECK REINFORCEMENT AT ALL UNFRAMED OPENINGS, SLEEVES AND COLUMN CUTOUTS AS SHOWN ON PLANS OR AS INDICATED IN THE SPECIFICATIONS.
- 9. ALL METAL DECK UNITS SHALL BE FASTENED TO THE STEEL FRAME BY 5/8" DIAMETER PUDDLE WELD WITH AN AVERAGE WELD SPACING OF AT LEAST 12" ON CENTER. SIDE LAPS ARE TO BE WELDED AT A MAXIMUM SPACING OF 36" ON
- 10. FURNISH, INSTALL AND WELD IN POSITION, CLOSURES AND OTHER MISCELLANEOUS ITEMS AS REQUIRED TO CLOSE OPENINGS BETWEEN FLOOR UNITS AND COLUMNS, BEAMS AND GIRDERS AND AREAS WHERE DECK CHANGES SPAN DIRECTION. CLOSURE SHALL NOT BE LESS THAN 16 GAGE SHEET METAL.
- 11. CONTRACTOR SHALL SUBMIT SHOP DRAWINGS SHOWING LAYOUT OF DECK PANELS INCLUDING DETAILS OF SPECIAL CONDITIONS.

SPECIAL STRUCTURAL INSPECTIONS

- 1. ALL WORKS SHALL BE SUBJECT TO SPECIAL INSPECTIONS IN ACCORDANCE WITH CHAPTER 17 OF NEW YORK STATE BUILDING CODE. SPECIAL INSPECTORS SHALL MEET THE QUALIFICATIONS OUTLINED IN THE RULES OF THE STATE OF NEW YORK SECTION 105.3.
- 2. SEE COVER SHEET FOR REQUIRED SPECIAL INSPECTIONS.

212 268 0128

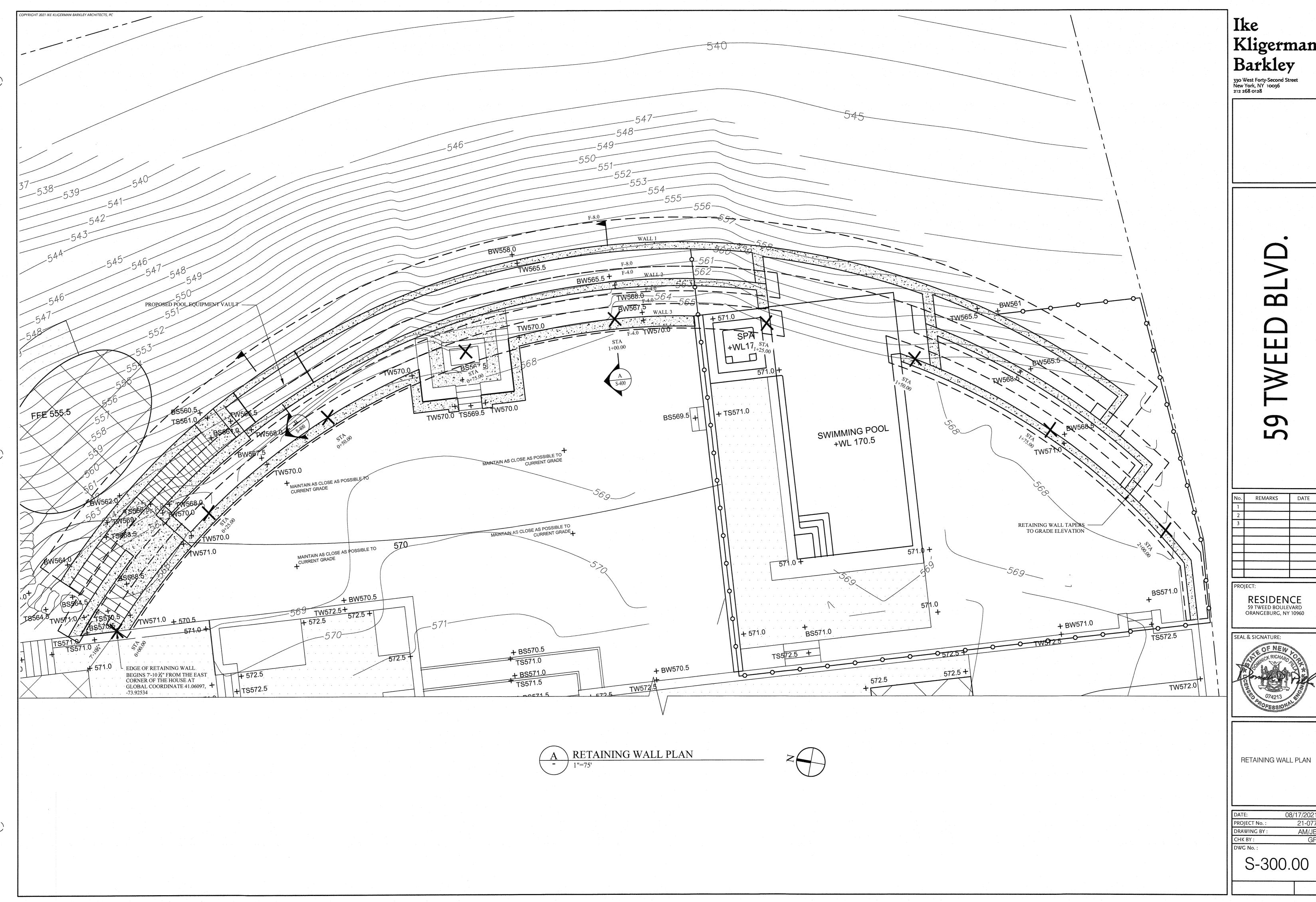
REMARKS DATE



59 TWEED BOULEVARD

GENERAL NOTES

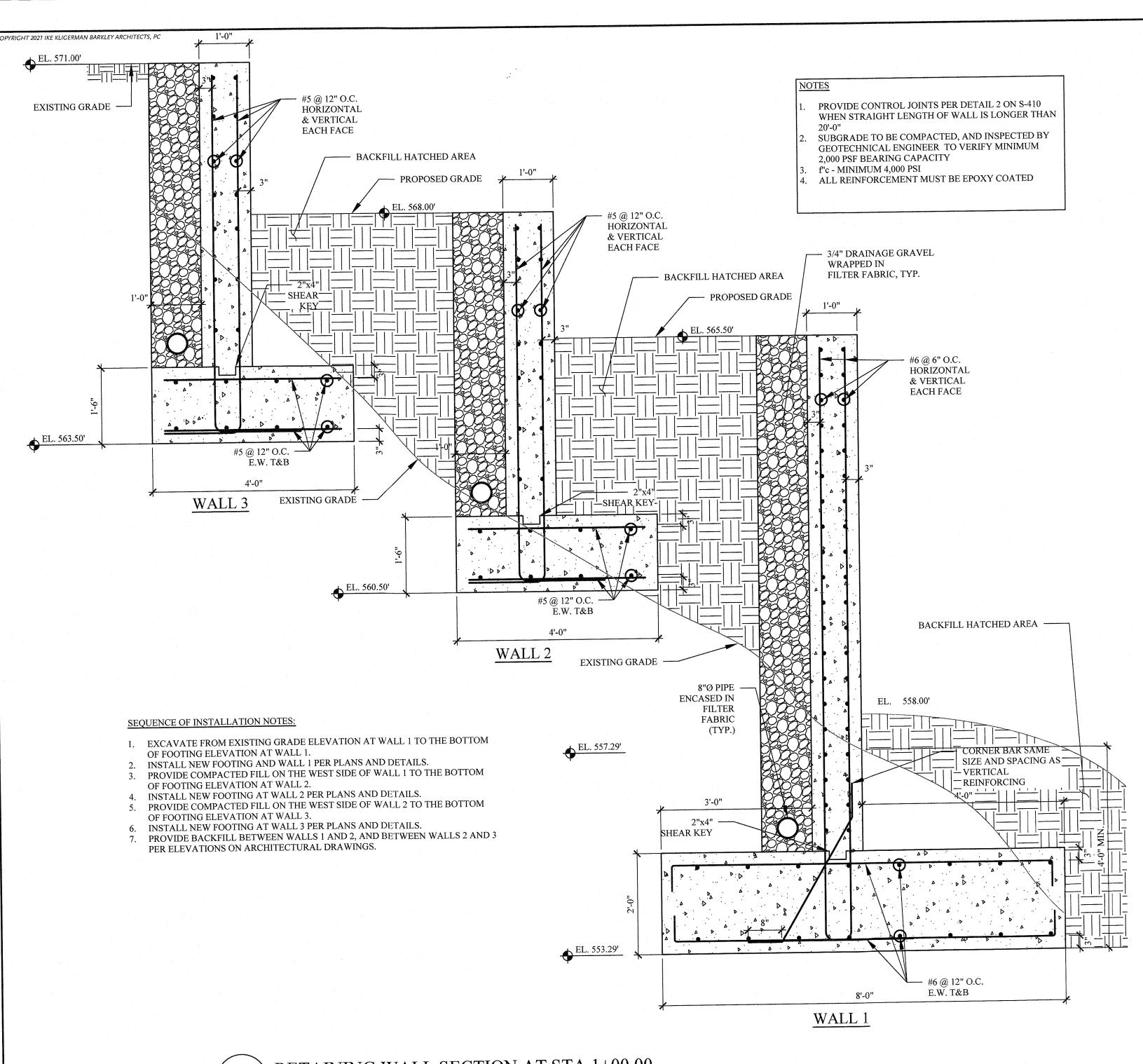
DATE: 08/17/202 PROIECT No. DRAWING BY CHK BY



Kligerman

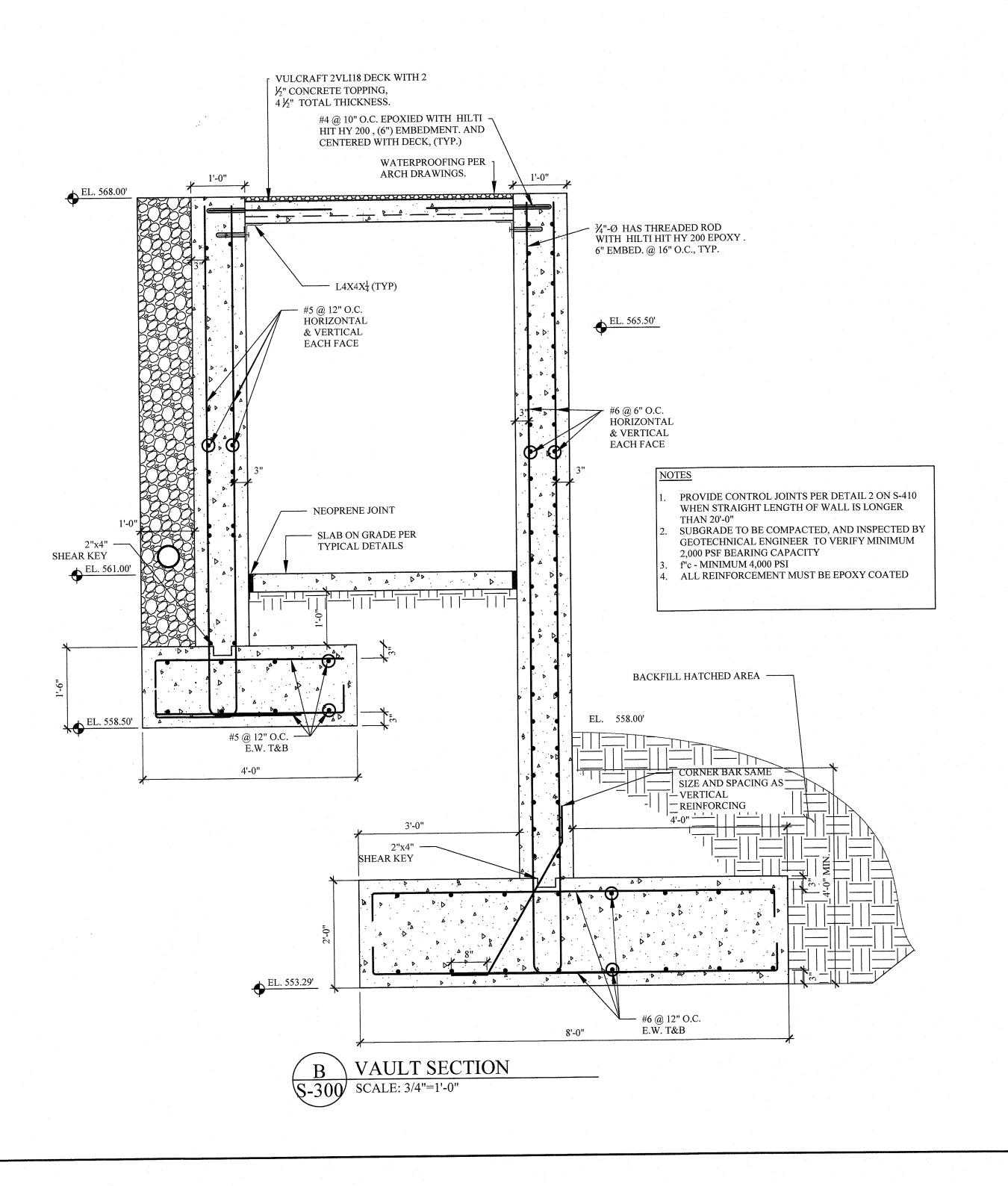


	•
DATE:	08/17/2021
PROJECT No. :	21-077
DRAWING BY :	AM/JB
CHK BY:	GF
DWG No. :	
0 00	



RETAINING WALL SECTION AT STA 1+00.00 S-300 SCALE: 3/4"=1'-0"

· · · · · · · · · · · · · · · · · · ·				RETA	INING WALL S	CHEDULE				
		FROM STA 0+00.00 TO STA 0+16.75	FROM STA 0+16.75 TO STA 0+25.00	FROM STA 0+25.00 TO STA 0+50.00	FROM STA 0+50.00 TO STA 0+75.00	FROM STA 0+75.00 TO STA 1+00.00	FROM STA 1+00.00 TO STA 1+25.00	FROM STA 1+25.00 TO STA 1+50.00	FROM STA 1+50.00 TO STA 1+75.00	FROM STA 1+75.00 TO STA 2+00.00
	T.O.W. ELEVATION (FT)	569.0	569.0	565.5	565.5	565.5	565.5	565.5	565.5	565.5
WALL 1	B.O.W. ELEVATION (FT)	564.0	562.0	560.5	558.0	558.0	558.0	558.0	561.0	561.0
	B.O.F. ELEVATION (FT)	562.0	560.0	558.5	556.0	556.0	556.0	556.0	559.0	559.0
	T.O.W. ELEVATION (FT)	571.0	570.0	568.0	568.0	568.0	568.0	NO WALL	568.5	568.5
WALL 2	B.O.W. ELEVATION (FT)	564.5	564.5	561.0	565.5	565.5	565.5	NO WALL	656.5	656.5
	B.O.F. ELEVATION (FT)	563.0	563.0	559.5	564.0	564.0	564.0	NO WALL	655.0	655.0
	T.O.W. ELEVATION (FT)	571.0	571.0	570.0	570.0	570.0	570.0	NO WALL	571.0	571.0
WALL 3	B.O.W. ELEVATION (FT)	570.5	570.5	567.5	567.5	567.5	567.5	NO WALL	568.5	568.5
	B.O.F. ELEVATION (FT)	560.0	569.0	566.0	566.0	566.0	566.0	NO WALL	567.0	567



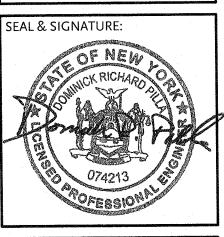
330 West Forty-Second Street New York, NY 10036 212 268 0128

1		
2		
3		
-		

PRO	JECT:	
	DECIDEN	С Г

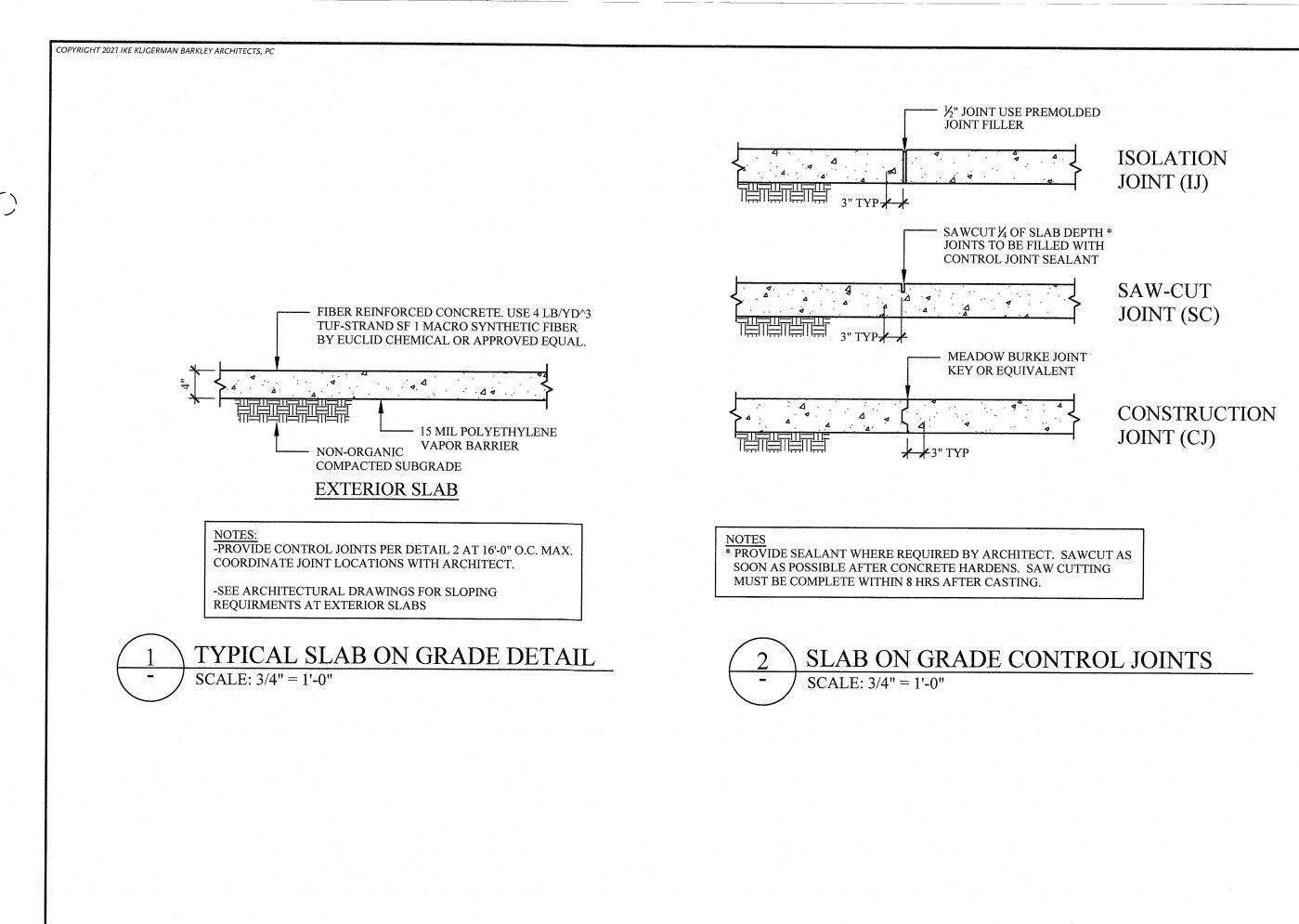
REMARKS DATE

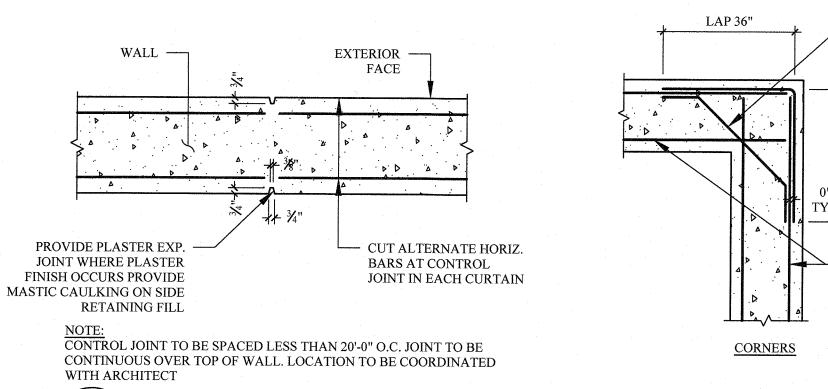
RESIDENCE
59 TWEED BOULEVARD
ORANGEBURG, NY 10960

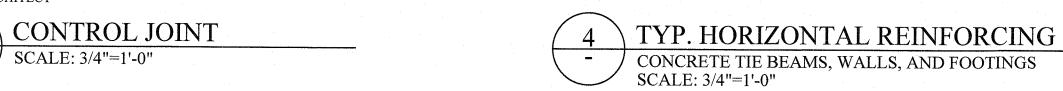


RETAINING WALL SECTIONS

08/17/2021 DATE: PROJECT No. AM/JE DRAWING BY CHK BY: DWG No.:







OUTSIDE CORNER — BARS SAME SIZE &

SPACING AS

HORIZONTAL

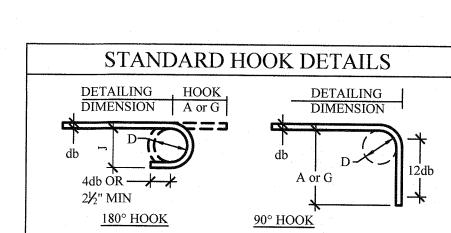
REINFORCING

TYPICAL HORIZONTAL

REINFORCING

2"x4" KEYWAY

INTERSECTIONS



LAP 36"

RECOMMENDED END HOOKS, ALL GRADES

		1000 1	1001/20	90° HOOKS
BAR	_	180° H	180° HOOKS	
SIZE	D	A or G	J	A or G
#3	2½"	5"	3"	6"
#4	3"	6"	4"	8"
#5	33/4"	7"	5"	10"
#6	4½"	8"	6"	1'-0"
#7	51/4"	10"	7"	1'-2"
#8	6"	11"	8"	1'-4"
#9	9½"	1'-3"	113/4"	1'-7"
#10	101/4"	1'-5"	1'-11/4"	1'-10"
#11	12"	1'-7"	1'-2¾"	2'-0"

MIN. LAP SPLICE I	LENGTH SCHEDULE	
fc=4,000 psi	fy=60,000 psi	_
THAN DB, CLEAR COVER NOT LESS	DEVELOPED OR SPLICED NOT LESS S THAN db, AND BEAM STIRRUPS OF D NOT LESS THAN CODE MINIMUM	3

Bar size	de	Id	Id (top bar)	1.3*ld	1.3*ld(top)
#4	0.500	19	25	25	32
#5	0.625	24	31	31	40
#6	0.750	28	37	37	48
#7	0.875	42	54	54	70
#8	1.000	47	62	62	80
#9	1.128	54	70	70	90
#10	1.270	60	78	78	102
#11	1.410	67	87	87	113

Bar size	de	Id	Id (top bar)	1.3*ld	1.3*ld(top)
#4	0.500	28	37	37	48
#5	0.625	36	46	46	60
#6	0.750	43	55	55	72
#7	0.875	62	81	81	105
#8	1.000	71	92	92	120
#9	1.128	80	104	104	136
#10	1.270	90	117	117	153
#11	1.410	100	130	130	170

Ike Kligerman Barkley

330 West Forty-Second Street New York, NY 10036 212 268 0128

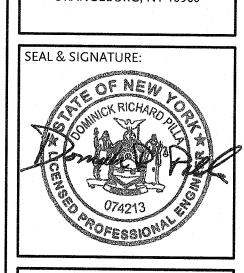
59 TWEED BLVD

ROJECT:

RESIDENCE

59 TWEED BOULEVARD
ORANGEBURG, NY 10960

REMARKS



RETAINING WALL DETAILS

DATE:	08/17/2021
PROJECT No.:	21-077
DRAWING BY:	AM/JB
CHK BY :	GF
DWG No.:	

S-410.00